STRUCTURAL GENERAL NOTES

A. GENERAL

1. The contractor shall verify all dimensions prior to starting construction. The architect shall be notified of any discrepancies or inconsistencies.
2. Dimensions shall take precedence over scale shown on drawings.
3. Notes and details on drawings shall take precedence over general notes and typical notes.
4. All work shall conform to the minimum standards of the following code. The 2012 edition of the International Building Code, and any other regulating agencies which have authority over any portion of the work, and those codes and standards listed in these notes and specifications.
5. See architectural drawings for the following:
   - Size and location of all door and window openings, except as noted.
   - Size and location of all interior and exterior nonbearing partitions.
   - Size and location of all concrete curbs, floor drains, slopes, depressed areas, changes in level, chamfers, grooves, inserts, etc.
   - Size and location of floor and roof openings except as shown
   - Floor and roof finishes
   - Stair framing and details (except as shown)
6. See mechanical, plumbing, and electrical drawings for the following:
   - Pipe runs, sleeves, hangers, trenches, wall and slab openings, etc. Except as shown or noted.
   - Electrical conduit runs, boxes, outlets in walls and slabs.
   - Concrete inserts for electrical, mechanical or plumbing fixtures.
   - Size and location of machine or equipment bases, anchor bolts for mounts.
7. The contract structural drawings and specifications represent the finished structure. They do not indicate the method of construction. The contractor shall provide all measures necessary to protect the structure during construction. Such measure shall include, but not be limited to, bracing, shoring for loads due to construction equipment, etc. Observation visits to the site by the structural engineer shall not include inspection of the above structural members.
8. Openings, pockets, etc. larger than 6 inches shall not be placed in slabs, decks, beams, joists, columns, walls, etc. unless specifically detailed on the structural drawings. Notify the structural engineer when drawings by others show openings, pockets, etc. not shown on the structural drawings, but which are located on structural members.
9. ASTM specifications noted shall be the latest revision.
10. Contractor shall investigate site during clearing and earthwork operations for filled excavations or buried structures such as cesspools, cisterns, foundations, etc. If any such structures are found, the structural engineer shall be notified immediately
11. Construction materials shall be spread out if placed on floors or roof. Load shall not exceed the design live load per square foot. Provide adequate shoring and/or bracing where structure has not attained design strength.
12. Design Loads:
    Roof:
    - 20 psf DEAD
    - 20 psf LIVE
    - 63 psf SNOW (Pg=90 psf)
    - 75 psf SNOW (Unheated)

    1 (revision)

    Floor:
    - 21 psf DEAD
    - 40 psf LIVE
    - 100 psf CORRIDOR
    - 125 psf STORAGE

    Wind:
    - Velocity 105 mph (3 sec. Gust)
    - Exposure "C"
    - Risk Category = II

    Seismic:
    1. Importance Factor: I = 1.0
    2. S_S = 0.689 S_1 = 0.224
    3. Site Class: "D"
    4. S_{DS} = 0.574 S_{D1} = 0.321
    5. Seismic Design Category "D"
    6. Seismic Force Resisting System: Timber roof & floor diaphragms with wood shear walls.
    7. Base Shear: V = 223.7 kips
    8. C_S = 0.088
    9. R = 6.5
    10. Analysis Procedure: Equivalent lateral force method.
    11. Risk Category: "II"

B. FOUNDATION

1. Footings are designed based on an allowable soil pressure of 3000 PSF per the project soils report. Footings and foundations have been designed in accordance with the soils report prepared by:

   Company: Atlas Technical Consultants, LLC
   Job Number: E213232g
   Date: 12/14/2021

2. Contractor shall provide for proper de-watering of excavations from surface water, ground water, seepage, etc.
3. Footings shall be placed according to depths shown on the drawings.
4. Footing back fill and utility trench back fill within building area shall be mechanically compacted in layers. Flooding will not be permitted.
5. All abandoned footings, utilities, etc. that interfere with new construction shall be removed.
6. The soil under perimeter beams and slabs shall be above optimum moisture as described in the referenced geotechnical report prior to concrete placement and shall be verified by the soils engineer.
7. Holdown anchor bolts shall meet the requirements of detail 1/SD-1.
8. All 1/2" Ø anchor bolts may be replaced with ICC approved 1/2"Ø Titen HD screws or 1/2"Ø all thread rod in 5/8" Ø hole with 4" embed using Simpson SET-XP epoxy at the spacing indicated below.

ANCHOR BOLT RETROFIT TABLE	
S1, S2, NON-SHEAR	SAME AS 1/2" Ø A.B.
S3 & S4	12" O.C.

C. CONCRETE

1. All phases of work pertaining to the concrete construction shall conform to the "Building Code Requirements for Reinforced Concrete" (ACI 318 latest approved edition) with modifications as noted in the drawings and specifications.
2. Reinforced concrete design is by the "Ultimate Strength Design Method", ACI 318-(latest edition)
3. Schedule of structural concrete 28-day strengths and types:

Location in structure	Strength PSI	Type
Slabs on Grade	4000	Hard rock
Footings	4000	Hard rock

   Design based on 2500 PSI, 28-day strength, special inspection is required only where indicated on foundation plans. See note "O" for P.T. tendon placement and additional information.
4. Concrete mix design shall be submitted to the engineer for approval with the following requirements:
   a. Compressive strength at age 28 days as specified above.
   b. Large aggregate-hardrock, ¾" maximum size conforming to ASTM C-33
   c. Cement-ASTM C-150, Type II Portland cement
   d. Maximum slump 5-inches, max water cement ratio: 0.50
   e. Minimum 6% entrained air for exterior foundation walls and concrete exposed to frost
5. No admixtures, except for entrained air, Hycrete or equivalent waterproofing admixture at the parking level slab, and as approved by the engineer.
6. Concrete mixing operations, etc. shall conform to ASTM C-94
7. Placement of concrete shall conform to ACI standard 514 and project specifications.
8. Clear coverage of concrete over outer reinforcing bars shall be as follows: Concrete poured directly against earth - 3 inches clear, structural slabs - 3/4 inches clear (top and bottom), formed concrete with earth back fill - 2 inches clear.
9. All reinforcing bars, anchor bolts and other concrete inserts shall be well secured in position prior to placing concrete.
10. Provide sleeves for plumbing and electrical openings in concrete before placing. Do not cut any reinforcing that may conflict. Coring in concrete is not permitted except as shown. Notify the structural engineer in advance of conditions not shown on the drawings.
11. Conduit or pipe size (O.D.) shall not exceed 30% of slab thickness and shall be placed between the top and bottom reinforcing, unless specifically detailed otherwise. Concentrations of conduits or pipes shall be avoided except where detailed openings are provided.
12. Modulus of elasticity of concrete, when tested in accordance with ASTM C-460, shall be at least the value given by the equations in section (19.2.2.1) 8.5.1 of ACI 318 for the specified 28-day strength.
13. Shrinkage of concrete, when tested in accordance with ASTM C-157, shall not exceed 0.00040 inches/inch.

D. REINFORCING STEEL

1. Reinforcing bars shall conform to the requirements of ASTM A-615 grade 60.
2. All reinforcing bar bends shall be made cold
3. Minimum lap of welded wire fabric shall be 6 inches or one full mesh and one half, which ever is greater.
4. All bars shall be marked so their identification can be made when the final in-place inspection is made.
5. Rebar splices are to be: Class "B"
6. Reinforcing splices shall be made only where indicated on the drawings.
7. Dowels between footings and walls or columns shall be the same grade, size and spacing or number as the vertical reinforcing, respectively.

E. WOOD

1. Framing Lumber:
   A. 2x4,and 2x6 studs: Douglas fir stud grade, U.N.O.
   B. 2x and 4x framing: Douglas fir larch No. 2 grade (except 2X4 and 2X6)
   C. 6x framing: Douglas fir larch No. 1 grade
2. Bolt holes shall be 1/16" maximum larger than the bolt size. Re-tighten all nuts prior to closing in.
3. Standard cut washers shall be used under all sill plate anchor bolts, U.N.O. at shear walls. See the Shear Wall Schedule on sheet S1.1 for anchor bolt spacing and washer requirements at shear walls.
4. All sills or plates resting on concrete or masonry shall be pressure treated Douglas Fir. Bolts shall be placed 9 inches from the end of a plate, or from a notch greater than ½ the width of the plate, and spaced at intervals noted.
5. Do not notch joists, rafters or beams except where shown in details. Obtain engineer's approval for any holes or notches not detailed. Holes through sills, plates, studs and double plates in interior, bearing and shear walls shall conform with detail 11/S1.2.
6. Connection hardware shall be by USP or Simpson Strong-Tie, or ICC approved equal.

DUAL SPECIFICATION TABLE	
CS16	RS150
ST6224	KST224
A35	MPA1
LUS24-2	JUS24-2
H1	RT15
H10	RT16A
LTP4	MP4F
LSSU	LSSH

DUAL SPECIFICATION TABLE	
HDU2	PHD2A
HDU4	PHD4A
HDU5	PHD5A
HDU8	PHD8A
HDU11	UPHD11
STHD10	STAD10
STHD14	STAD14
ST1884	

7. Fastening schedule per 2018 edition of the International Building Code, table No. 2304.9.1. Unless noted otherwise.
8. All nails, bolts, holdowns, straps or other steel fasteners in contact with pressure treated timber shall be hot-dipped galvanized, stainless steel or otherwise treated or isolated to prevent chemical attack. Contractor shall verify treatment method and confirm appropriate corrosion resistance is provided in accordance with hardware supplier recommendations.
9. Sawn lumber is to comply with DOC PS 20.
10. Wood structural panels to conform to the requirements for their type in DOC PS1, PS2, or ANSI/APA PRP210.
11. Non-bearing, non-shear interior walls to be anchored to floor and /or roof as indicated on detail 10/S1.1.

F. PREFABRICATED WOOD TRUSSES

1. Prefabricated wood roof trusses shall be as designed by the truss manufacturer. Bridging size and spacing by truss manufacturer unless noted otherwise. Contractor shall submit shop drawings, erection drawings and design calculations sealed by an engineer, registered in the state of Idaho, for review prior to manufacture. Calculations and shop drawings shall show any special details required at bearing points. All connectors shall be Simpson or equivalent with current ICC approval.
2. Truss manufacturer to design trusses for lateral load (LAT. = xxxx) in pounds, as shown on plans. Lateral loads are ASD level loads.
3. Additional trusses shall be supplied as required to support mechanical equipment.
4. All truss-to-truss and truss-to-beam connectors per truss manufacturer.
5. Wood truss design to be a deferred submittal item. Truss package to be submitted to the Building Official prior to installation.

G. GLUE LAMINATED BEAMS (GLB)

1. Glue laminated beams shall be 24F-V4 (cantilevers and continuous beams shall be 24F-V8) and have the following minimum properties: fb=2400 psi, Fv=265 psi, Fc (perpendicular)=650 psi, E=1,800,000 psi. All beams shall be fabricated using waterproof glue. Fabrication and handling per latest AITC and WCCA standards. Beams to bear grade stamp and AITC stamp and certificate. Moisture content shall be limited to 12% or less.

H. LAMINATED VENEER LUMBER (LVL)

1. Laminated veneer lumber to have: Fb=2600 psi, Fv=285 psi, E=1.9x10^6psi
2. Double & triple LVL beams shall be nailed together as follows:
   Provide (2) rows of 16d sinkers at 12" O.C. for beams < 11 7/8" deep
   Provide (3) rows of 16d sinkers at 12" O.C. for beams > 11 7/8" deep
3. Beams w/ (4) or more plies shall be bolted together as indicated in the manufacturer's written specifications.

I. WOOD STRUCTURAL PANELS

1. All wood structural panels shall be plywood or APA rated oriented strand board. Panels shall bear the stamp of an approved agency. Panels shall be of the span/index rating shown on the plans. Fastening shall be indicated on the plans.
2. All plywood shall be C-D interior sheathing with exterior glue. Plywood shall be 4-ply, minimum.

J. SHOP DRAWINGS

1. Shop drawings shall be submitted for all structural items in addition to items required by architectural specifications.
2. The contractor shall review all shop drawings prior to submittal. Items not in accordance with contract drawings shall be flagged for review.
3. Verify all dimensions with architect.
4. Any changes, substitutions, or deviations from original contract drawings shall be redlined or flagged by submitting parties, shall be considered approved after engineers review, unless noted otherwise.
5. The engineer has the right to approve or disapprove any changes to the original drawings at anytime before or after shop drawings review.
6. The shop drawings do not replace the original contract drawings. Items omitted or shown incorrectly and are not flagged by the structural engineer or architect are not to be considered changes to the original contract drawings.
7. The adequacy of engineering designs and layout performed by the others rests with the designing or submitting authority.
8. Reviewing is intended only as an aid to the contractor in obtaining correct shop drawings. Responsibility for corrections shall rest with the contractor.

K. SHEATHING

1 (revision)

1. Roof sheathing:
   23/32" wood structural panel: plywood or oriented strand board (O.S.B.) panel index = 48/24, unblocked, nail with 10d common nails at 6" O.C. at all boundaries and supported edges, 12" O.C. field. Minimum penetration 1" in supporting member (NER 272)
2. Floor sheathing:
   3/4" (min.) wood structural panel: plywood or oriented strand board (O.S.B.) T & G, panel index = 48/24, unblocked, nail with 10d common nails at 6" O.C. at all boundaries and supported edges, 12" O.C. field.
3. Shear wall sheathing:
   Sheathing for shear walls shall be as indicated on the shear wall plans and schedules. Sheathing at shear walls may be installed with panels horizontal or vertical. All shear wall panels shall have minimum wood structural panel span rating of 24/0 or "Wall-16."

L. STRUCTURAL STEEL

1. Hot-rolled structural steel shapes & plates shall be per ASTM A36 with the following exception. All W-Flange shapes shall be per ASTM A992.
2. Structural steel pipe shall be per ASTM A53 grade B, Tube steel per ASTM A500 Grade B.
3. Nuts & bolts in structural steel connections shall be per ASTM 325N, with hardened washers. Design is based upon bearing type connections with thread not excluded, therefore, no special inspection required, U.N.O. in note M below.
4. Anchor bolts shall be per ASTM A307 and F1554, U.N.O.
5. Welds shall be by E70XX, low hydrogen electrodes, all welding shall be performed in a shop approved by the building official.
6. Grout material for base plates shall be non-metallic, non-shrink, pre-packaged grout conforming to ASTM C 1107.

M. SPECIAL INSPECTION / QUALITY ASSURANCE PLAN

1. The seismic lateral load resisting system consists of timber floor and roof diaphragms with wood shear walls and premanufactured shear walls.
2. Special inspections
   A. Special inspections shall be required for the following items and those shown in the inspection tables:
      - All field welding
      - All high-strength bolting operations
      - All connections of cantilever steel columns to foundations
      - All post-installed anchorage to concrete (epoxy grout applications)
      - When required by the local building department: All timber elements of the lateral force resisting system components
   B. The owners shall employ special inspectors who shall provide additional inspections during construction in accordance with IBC section 17.
   C. All special inspections shall be performed by an independent certified inspector from an established testing agency, licensed and approved by the building department.
   D. The testing agency shall send copies of all structural testing and inspection reports directly to Vector Structural Engineering and all interested parties.
   E. All masonry construction shall require Level 1 special inspection.
3. Structural testing is not required.
4. All reports shall be distributed on a monthly basis to the engineer of record, owner, contractor, and to the building official.
5. No structural observation is required. However, the engineer of record reserves the right to make field observations during construction approximately once per week.

N. DEFERRED SUBMITTALS

1. All deferred submittals shall be submitted to the Engineer of Record for review and approval prior to being submitted to the Building Department. Provide (2) Copies of each deferred submittal for the Architect and Engineer of Record to review prior to forwarding them to the Building department. With notation indicating submittal conforms to the design of the building.
2. Deferred submittals must be submitted to the Building Department within 90 days from the receipt of the building permit.
3. All calculations and shop drawings submitted for review must be stamped (sealed) by a licensed and registered professional engineer within the state or jurisdiction of the project construction.
4. The engineer(s) responsible for the design of the deferred submittal shall stamp (seal) and wet-sign those documents and calculations for which they are responsible.
5. Contractor shall review, coordinate and markup drawings prior to submitting them to the Architect/Engineer of record for review prior to installation or fabrication.
6. The following items may be deferred submittals:
   a. Prefabricated wood trusses (See Prefabricated wood truss Section of GSN)
   b. Concrete mix designs
   c. Steel shop and erection drawings
   d. Equipment anchorage

1 (revision)

O. ICC APPROVAL SCHEDULE

ICC-ES ESR SCHEDULE		
A35	SIMPSON	2606
CS16	SIMPSON	2105
FHA18	SIMPSON	2105
GBC	SIMPSON	2605
H1	SIMPSON	2613
H10	SIMPSON	2613
HDU2	SIMPSON	2230
JUS24-2	USP	3445
KST224	USP	3445
LSSH	USP	3446
LSSU	SIMPSON	2551
LTP4	SIMPSON	2606
LUS24-2	SIMPSON	2549
MP4F	USP	3445

ICC-ES ESR SCHEDULE		
MPA1	USP	3445
PHD	USP	0200
RPS	SIMPSON	2608
RS150	USP	3445
RT15	USP	3445
RT16A	USP	3445
SDS	SIMPSON	2236
SET-XP	SIMPSON	2508
ST6224	SIMPSON	2105
STAD	USP	2787
SW18x9	SIMPSON	1267
TITEN HD	SIMPSON	2713
UPHD	USP	0200

STRUCTURAL SHEET INDEX

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ABBREVIATIONS	
A.B.	ANCHOR BOLT
ARCH'L	ARCHITECTURAL DRAWINGS
BLDG	BUILDING
BLK	BLOCK
BLK'G	BLOCKING
BM	BEAM
C.L.	CENTERLINE
CANT'L	CANTILEVERED
CLG	CEILING
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONT	CONTINUOUS
DBL	DOUBLE
DTL	DETAIL
EL	ELEVATION
EOR	ENGINEER OF RECORD
FND	FOUNDATION
FTG	FOOTING
GL	GLUE LAMINATED (BEAM)
H.D.	HOLD DOWN
HDR	HEADER
HORIZ.	HORIZONTAL
LSL	LAMINATED STAND LUMBER

ABBREVIATIONS	
LVL	LAMINATED VENEER LUMBER
MFR	MANUFACTURED
N.T.S.	NOT TO SCALE
O.C.	ON CENTER
O.S.B	ORIENTED STRAND BOARD
o/	OVER
OPT'L	OPTIONAL
PL	PLATE
PSL	PARALLEL STRAND LUMBER
REQ'D	REQUIRED
SHT	SHEET
SHTH'G	SHEATHING
SIM	SIMILAR
STL	STEEL
SW	STRONG-WALL
T&B	TOP & BOTTOM
T.O.F.	TOP OF FOOTING
T.O.W.	TOP OF WALL
TYP.	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
u/	UNDER
VERT.	VERTICAL
w/	WITH

RELEASE DATE:

100% CD

VECTOR ENGINEERS

CONSULTANT:

OWNER:

ELEMENT HOTEL

PROJECT:

ISSUE DATE | DESCRIPTION

#	REV. DATE	DESCRIPTION
1		

For Review
02/24/2025 10:13:22 AM

STRUCTURAL GENERAL NOTES

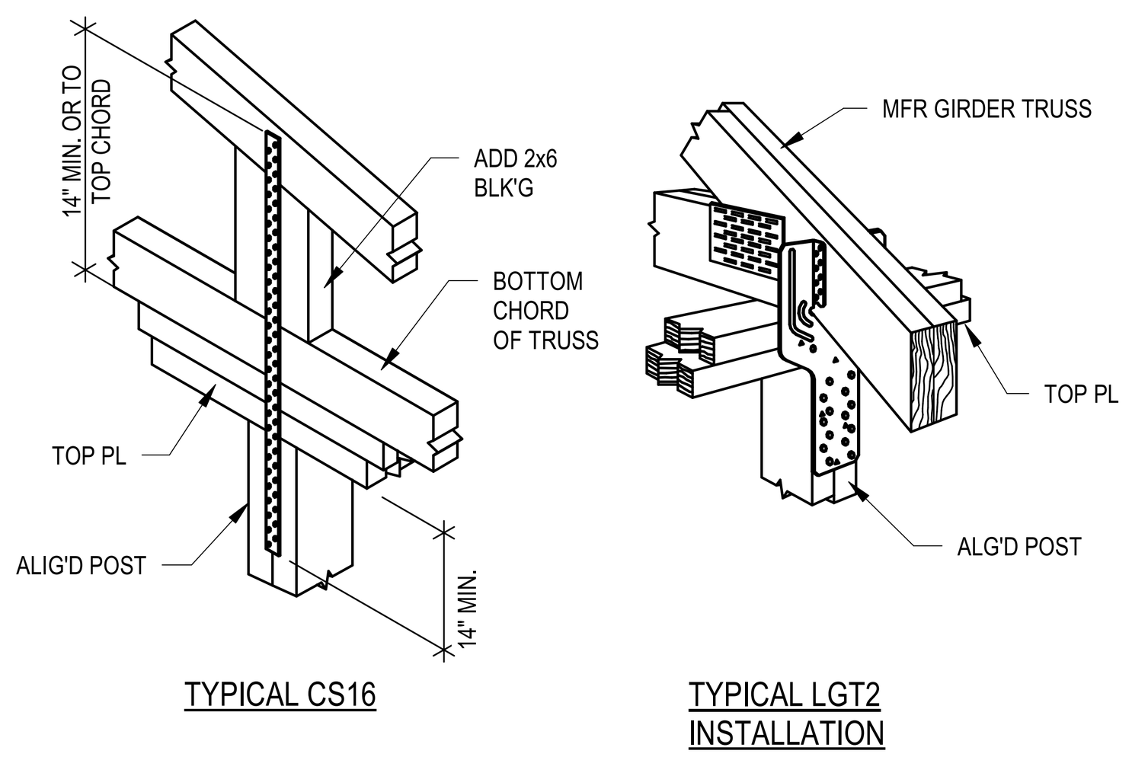
SHEET #
S1

STANDARD TRUSS TIE-DOWNS		
UPLIFT LOAD PER TRUSS MANUFACTURER	SIMPSON TIE-DOWN	REQ'D ALIGNED HOLDOWN & POST
< 365 LBS	H2.5 OR CS16	NOT REQ'D
< 400 LBS	H1 OR CS16	NOT REQ'D
< 845 LBS	H10 OR H7Z OR CS16	NOT REQ'D
< 1265 LBS	H16 OR CS16	HDU2 & (2) 2x4 POST
<1785 LBS	LGT2	HDU2 & (2) 2x4 POST
< 6585 LBS	HGT-2	(2) 2x4 POST w/ HDU4 @ BASE & (2) HDU2 @ TOP TO HGT-2. AT (1) PLY TRUSS, INSTALL 2X SHAPED FILLER ADJACENT TO TRUSS AT BEARING

NOTES:
1. TIE-DOWN CAPACITIES ARE BASED ON SPRUCE PINE FIR
2. TRUSS UPLIFT OF LESS THAN 200lbs: TIE-DOWN NOT REQ'D
3. SEE TYPICAL HOLDOWN ANCHORAGE DETAIL FOR HDU HOLDOWN INSTALLATION

STANDARD FLOOR-TO-FLOOR STRAP		
UPLIFT LOAD PER TRUSS MANUFACTURER	SIMPSON TIE-DOWN	REQ'D ALIGNED POST
< 1705 LBS	CS16	2x4 POST
< 3410 LBS	(2) CS16	(2) 2x4 POST

NOTES:
1. INSTALL CS16 STRAPS TO 2x STUDS ABOVE AND BELOW FLOOR FRAMING. NAIL EACH END w/ (11) 10d NAILS. (STRAP LENGTH = 48").
2. WHERE UPLIFT OCCURS ABOVE HDR OR BM, INSTALL STRAP PER SCHEDULE AT EACH TRIMMER OR POST
3. FLOOR TO FLOOR STRAPS REQ'D ALIGNED WITH ROOF TRUSS ABOVE



TYPICAL CS16

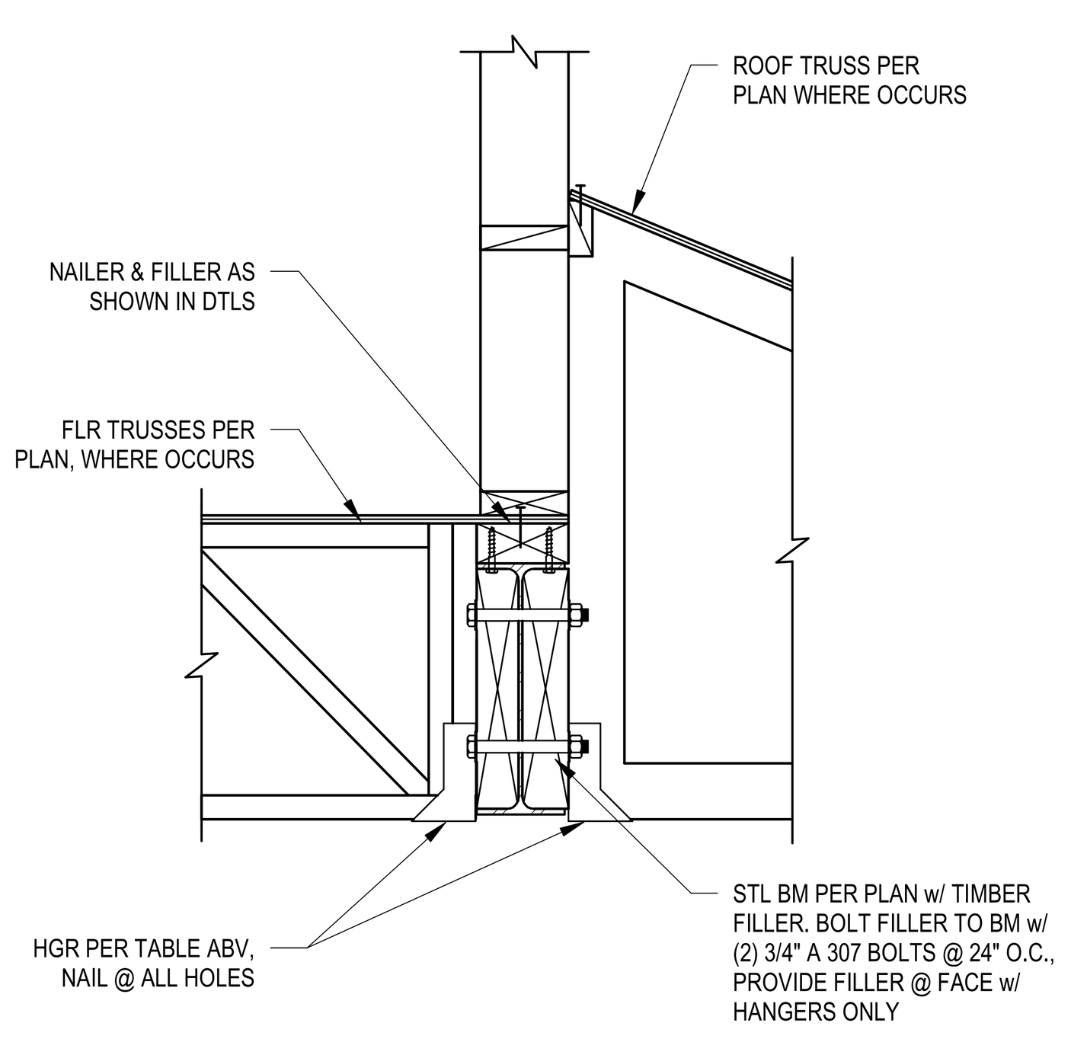
TYPICAL LGT2 INSTALLATION

TYPICAL TRUSS ANCHORAGE (14)

N.T.S.

MFR TRUSS TO BEAM HANGERS				
CARRYING MEMBER	CARRIED MBR WIDTH	HANGER TYPE	MAX. REACTION (FROM TRUSS CALCS.) (LBS)	NOTES
STEEL OR TIMBER	1-1/2"	LUS210	1275	FACE MOUNT
STEEL OR TIMBER	1-1/2"	HUS26	2565	FACE MOUNT
STEEL OR TIMBER	1-1/2"	HGUS26	3750	FACE MOUNT
STEEL OR TIMBER	1-1/2"	HGUS28	5720	FACE MOUNT
STEEL OR TIMBER	3"	LUS26-2	1000	FACE MOUNT
STEEL OR TIMBER	3"	HHUS26-2	2580	FACE MOUNT
STEEL OR TIMBER	3"	HGU26-2	3940	FACE MOUNT
STEEL OR TIMBER	3"	HGUS28-2	6805	FACE MOUNT
STEEL OR TIMBER	3"	HGUS210-2	8650	FACE MOUNT
STEEL OR TIMBER	3-1/2"	LUS46	1000	FACE MOUNT
STEEL OR TIMBER	3-1/2"	HHUS46	2580	FACE MOUNT
STEEL OR TIMBER	3-1/2"	HGUS46	3940	FACE MOUNT
STEEL OR TIMBER	3-1/2"	HGUS48	6805	FACE MOUNT
STEEL OR TIMBER	6"	HGUS26-4	3940	FACE MOUNT
STEEL OR TIMBER	6"	HGUS210-4	8780	FACE MOUNT
STEEL OR TIMBER	6"	HGUS212-4	9155	FACE MOUNT

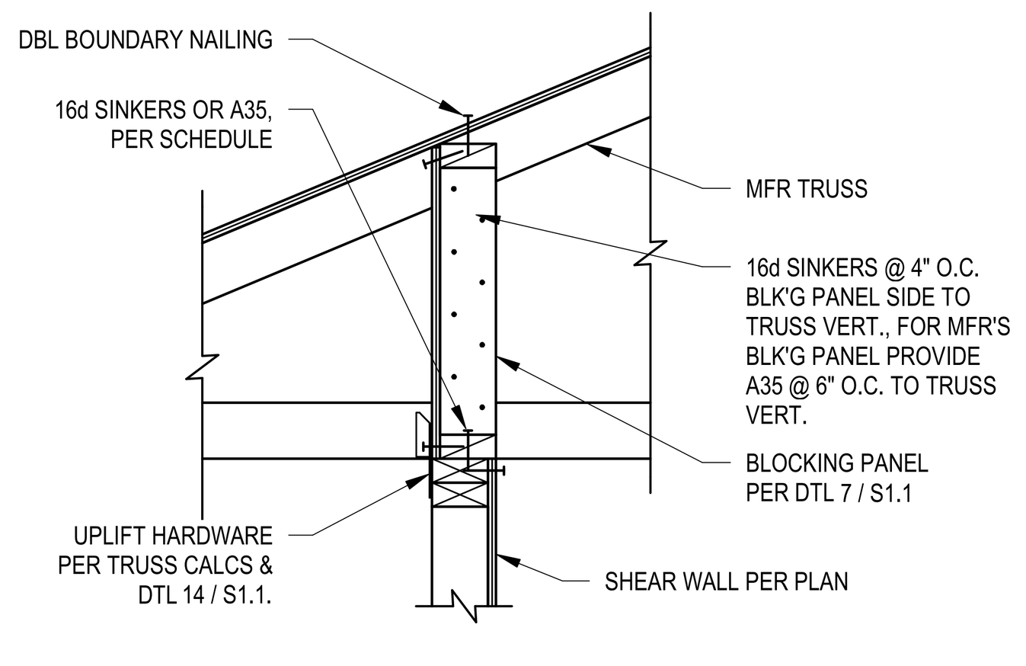
NOTES:
1. FOR STEEL BEAMS CARRYING FLOOR TRUSSES, PROVIDE TIMBER FILLER PER DTL BELOW.
2. ALTERNATE HANGERS MAY BE USED AT THE CONTRACTOR'S OPTION. SUBMIT TO ENGINEER OF RECORD FOR APPROVAL.
3. HANGERS APPLICABLE FOR TIMBER BEAMS.



TYPICAL STEEL BEAM

TYPICAL TRUSS HANGERS (15)

N.T.S.



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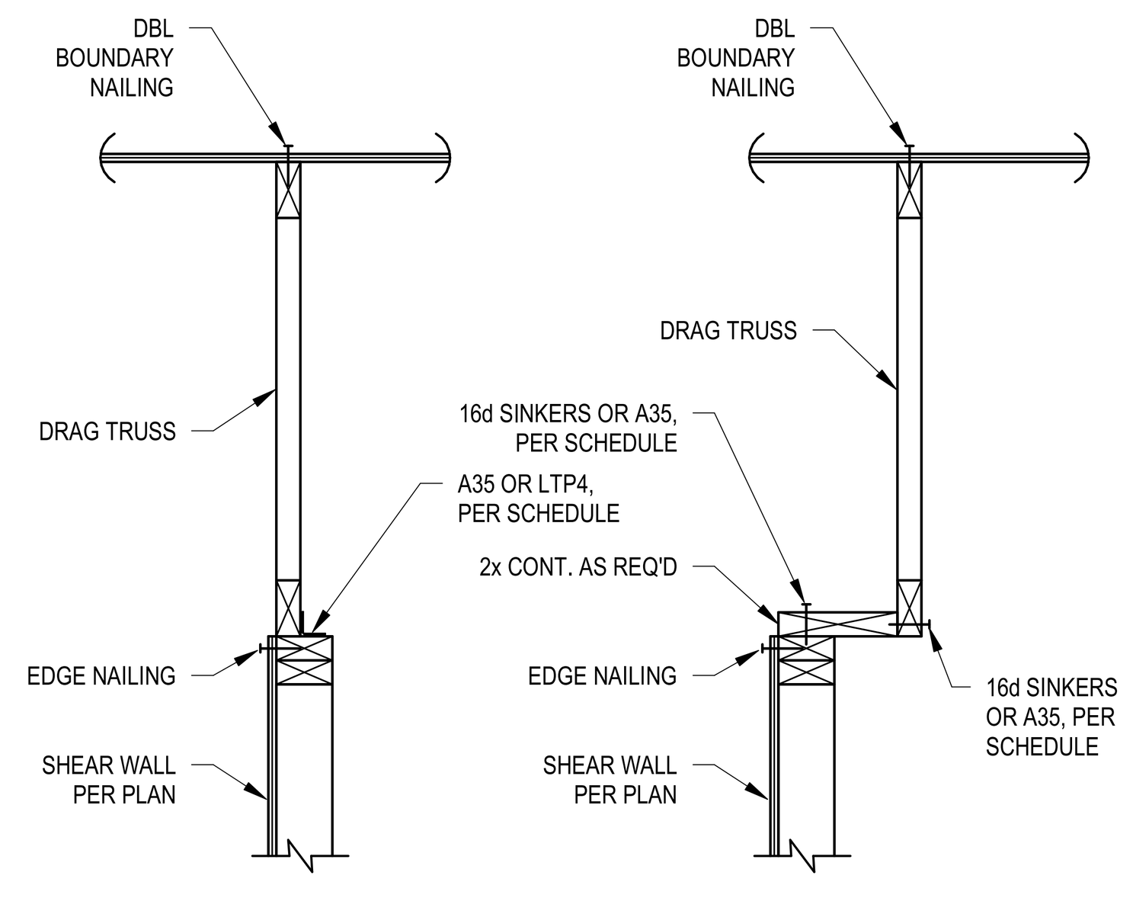

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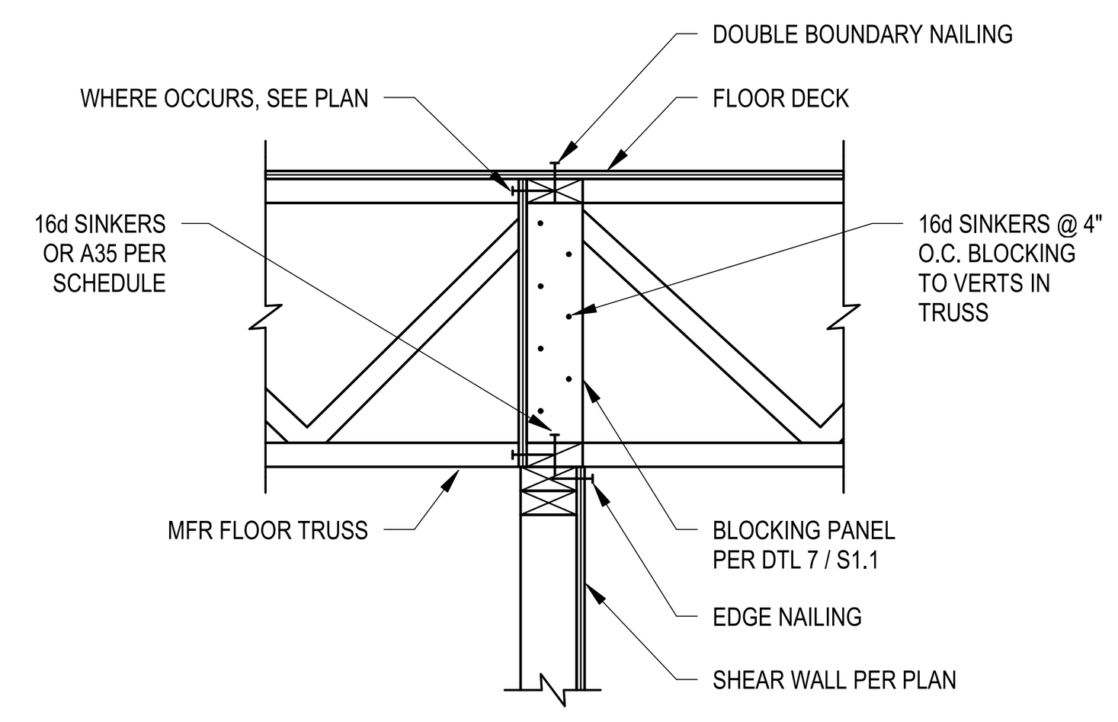

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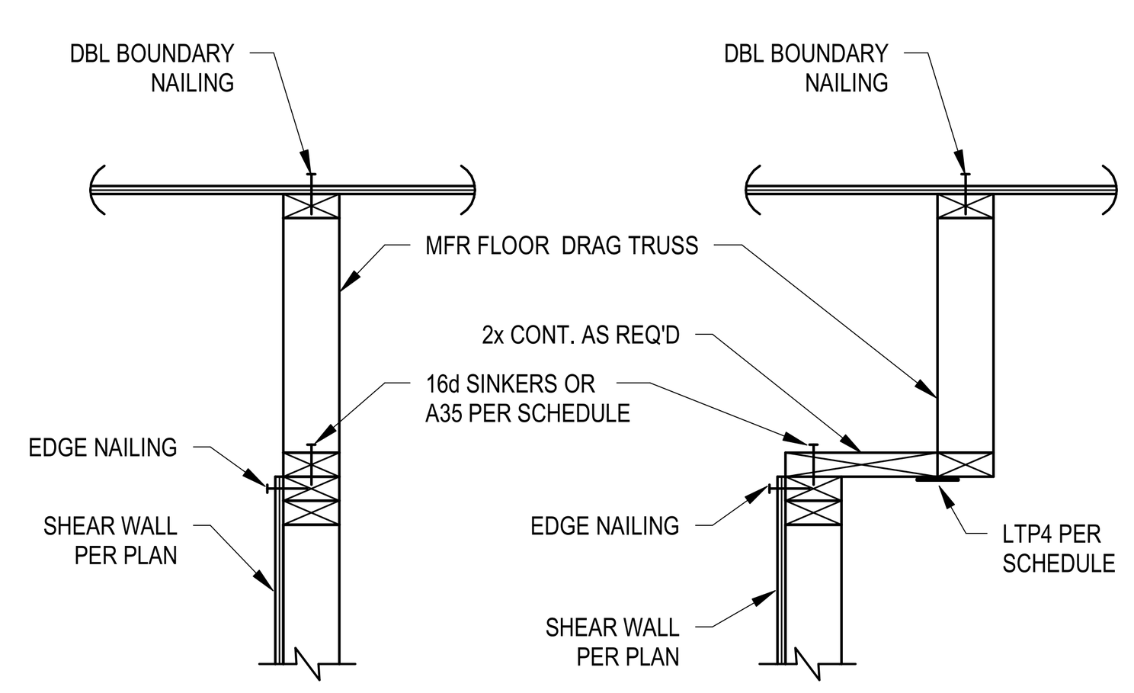
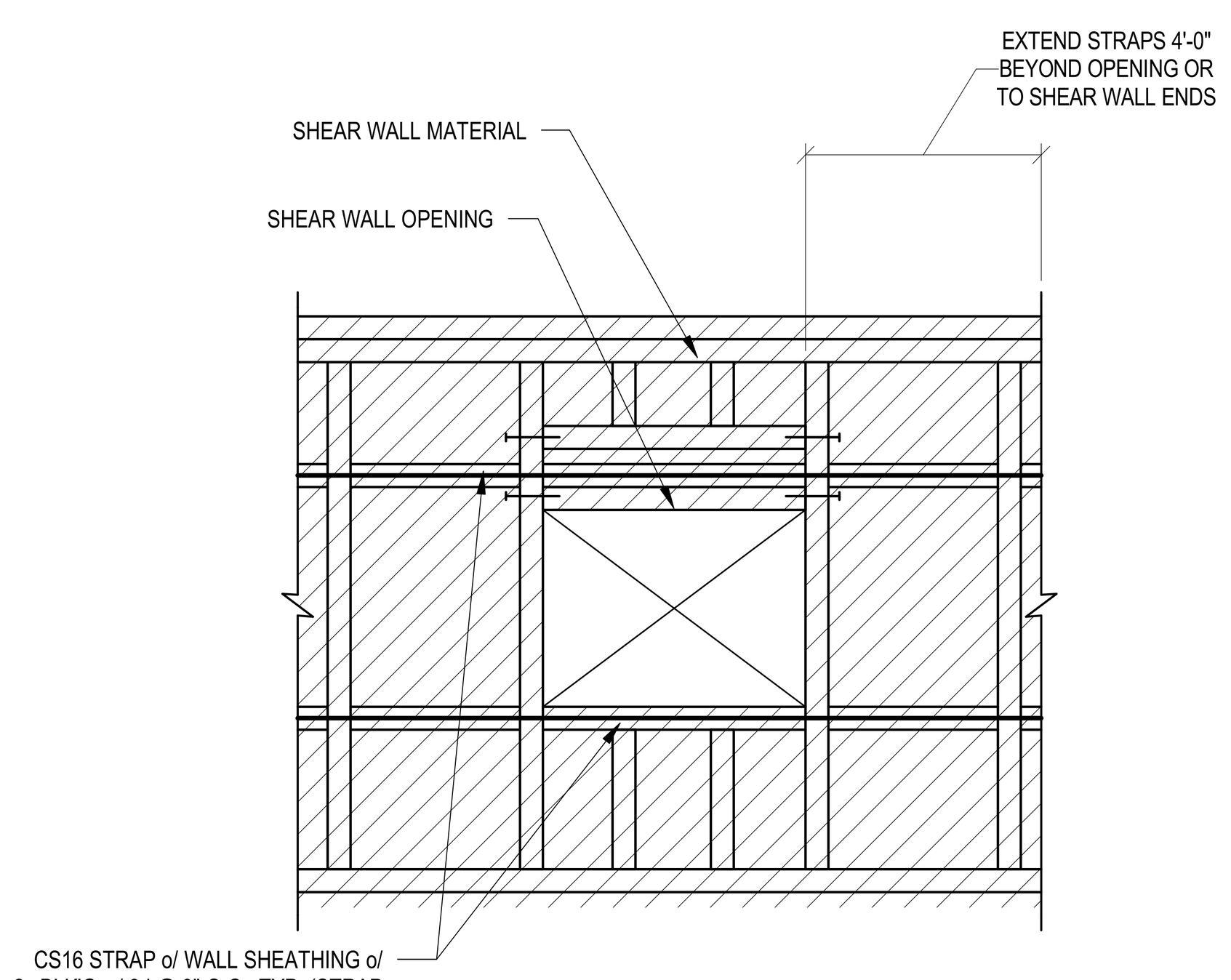


TYPE D

CONNECTION SCHEDULE		
SHEAR WALL	A35 OR LTP4	16d SINKERS
P1	18" O.C.	6" O.C.
P2	12" O.C.	4" O.C.
P3	10" O.C.	3" O.C. (STAGGERED)
P4	8" O.C.	2" O.C. (STAGGERED)

SHEAR TRANSFER (12)

N.T.S.

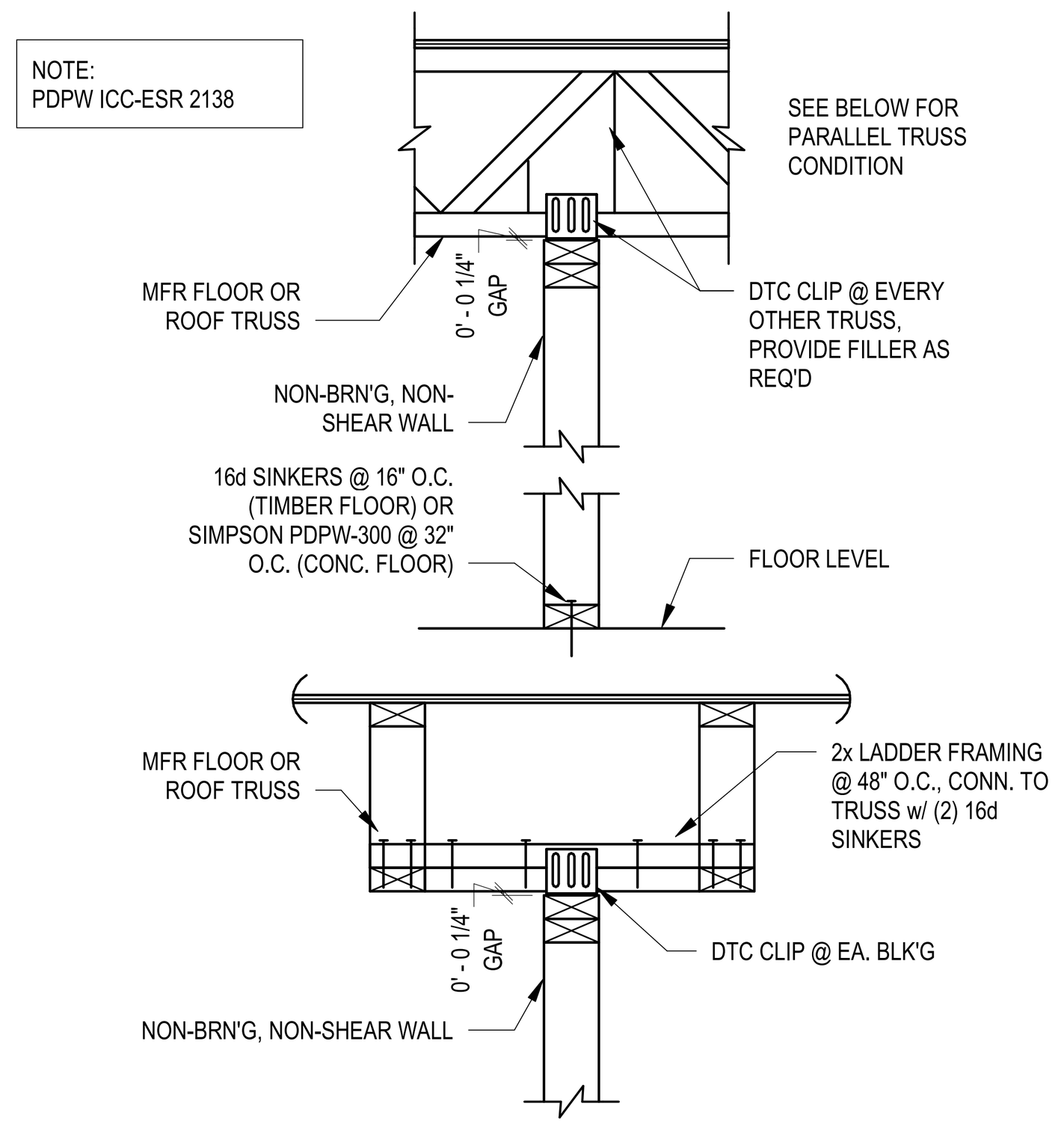


ELEVATION VIEW

SHEAR TRANSFER AT OPENING (13)

N.T.S.

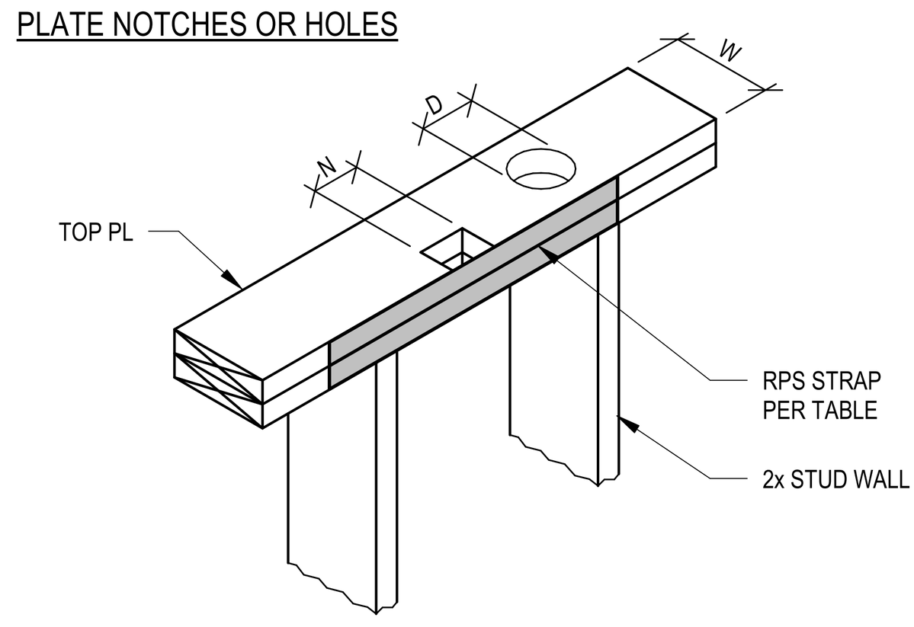
NOTE:
PDPW ICC-ESR 2138



NON-BRN'G & NON-SHEAR WALL CONN. (10)

N.T.S.

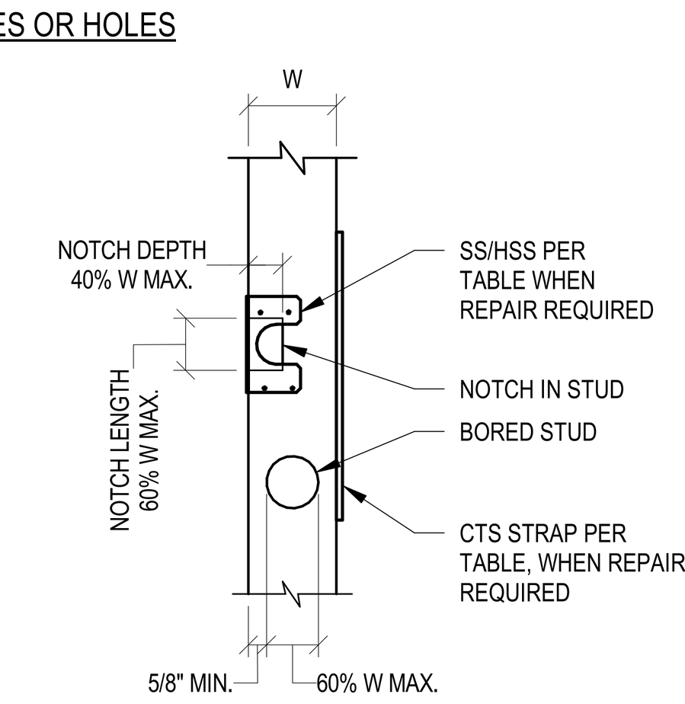
PLATE NOTCHES OR HOLES



2x4 PLATE	2x6 PLATE	2x4 & 2x6 PLATE	
HOLE DIA 'D'	HOLE DIA 'D'	NOTCH WIDTH 'N' (MAX NOTCH DEPTH = W/2)	RPS STRAP
≤ 7/8"	≤ 1"	≤ 1"	NONE
≤ 1"	≤ 1-3/8"	≤ 2-1/2"	(1) RPS18
≤ 1 3/8"	≤ 2-1/8"	≤ 5-1/2"	(2) RPS18
≤ 2"	≤ 3-1/4"	≤ 12"	(2) RPS28

NOTES:
1. USE RPSZ FOR SILL PLATE.
2. CENTER STRAPS @ NOTCH OR HOLE.
3. WHERE ROOF TRUSS OR FLOOR JOIST IS BEARING WITHIN STUD BAY OF THE HOLE OR NOTCH, INSTALL AN ADDITIONAL STUD DIRECTLY BELOW THE TRUSS OR JOIST UNLESS NO RPS STRAP IS REQUIRED OR WHERE EXISTING STUD FACE IS WITHIN 3" OF TRUSS OR JOIST FACE.
4. NOTCHES & HOLES MUST BE SEPARATED BY "2xD" OR "2xN".
5. WHERE MULTIPLE HOLES ARE LOCATED ADJACENT TO EACH OTHER, THE STRAP REPAIR MAY BE WITH A CS16 STRAP ON EACH SIDE OF THE UPPER PLATE. THE STRAPS AND NAILING SHALL EXTEND AT LEAST 9" BEYOND EACH END OF THE WHOLE GROUP. NAILING BETWEEN THE HOLES IS NOT REQUIRED. NAILS IN THE CS16 STRAPS MAY BE N8'S OR N10'S.

STUD NOTCHES OR HOLES



ALLOWABLE HOLES OR NOTCHES FOR NON-BEARING, NON-SHEAR OR INTERIOR PARTITIONS (NO REPAIR REQ'D)

HOLE / NOTCH SCHEDULE		
HOLE / NOTCH % OF 'W'	2x4 STUD	2X6 STUD
25%	3/4"	1-3/8"
40%	1-3/8"	2-1/8"
60%	2"	3-1/4"

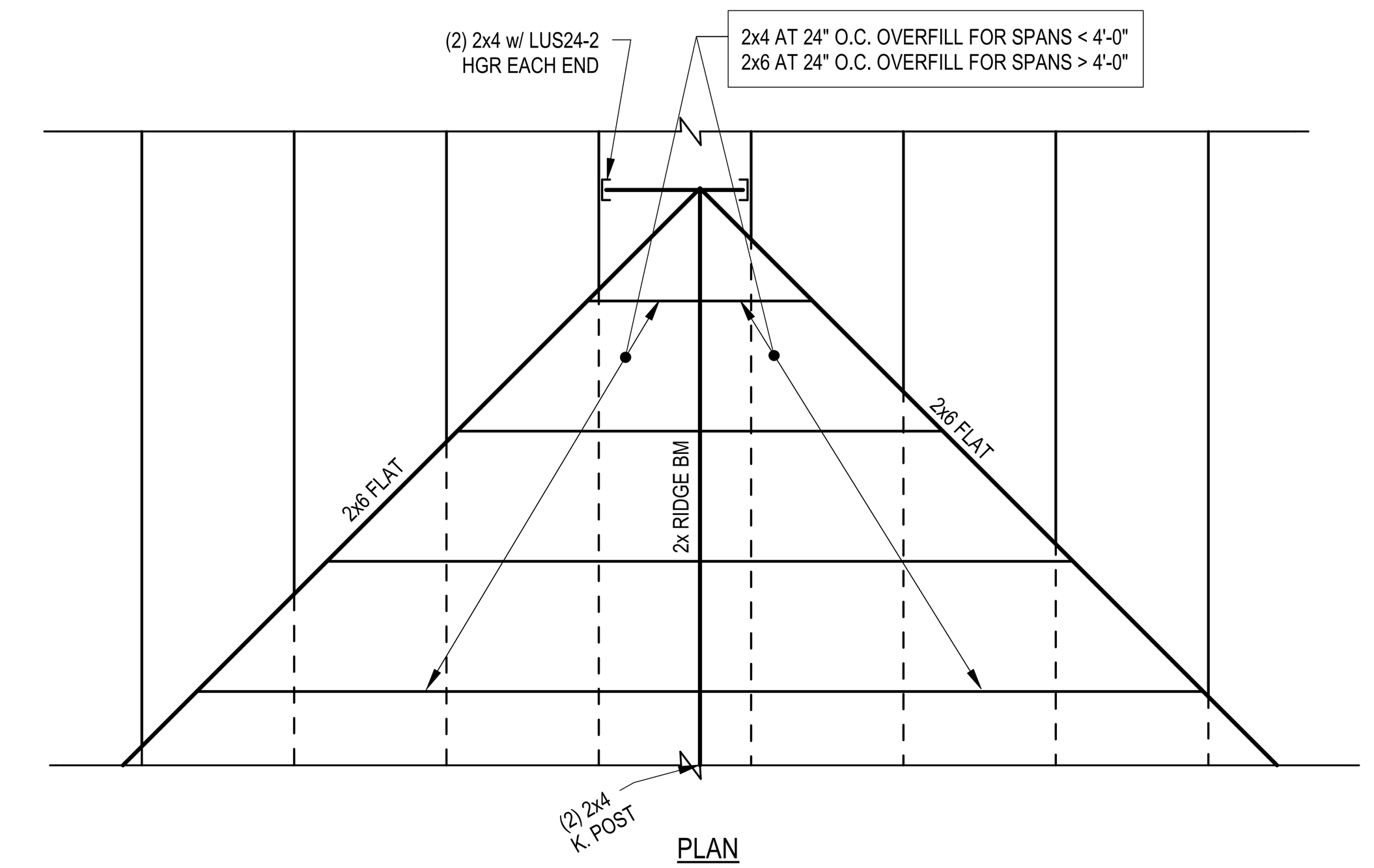
NOTES:
1. HOLES & NOTCHES SHALL NOT OCCUR IN THE SAME STUD.
2. WHERE HOLES OR NOTCHES EXCEED THOSE SHOWN ABOVE, REPAIR PER TABLE BELOW.
3. ALL NOTCHES IN BEARING OR SHEAR OR EXTERIOR WALLS REQUIRE REPAIRS.

STUD HOLE REPAIR			
	2x4 STUD	2x6 STUD	REPAIR
	HOLE DIA. 'D'	HOLE DIA. 'D'	
NON-BEARING & NON-SHEAR & INTERIOR WALL	≤ 2-3/4"	≤ 4-1/2"	(1) CTS218 w/ 10d
BEARING OR SHEAR OR EXTERIOR WALL	≤ 3/4"	≤ 1-3/8	(1) CTS218 w/ 10d
BEARING OR SHEAR OR EXTERIOR WALL	≤ 2-3/4"	≤ 4-1/2"	(2) CTS218 TWO-SIDED w/ 10d

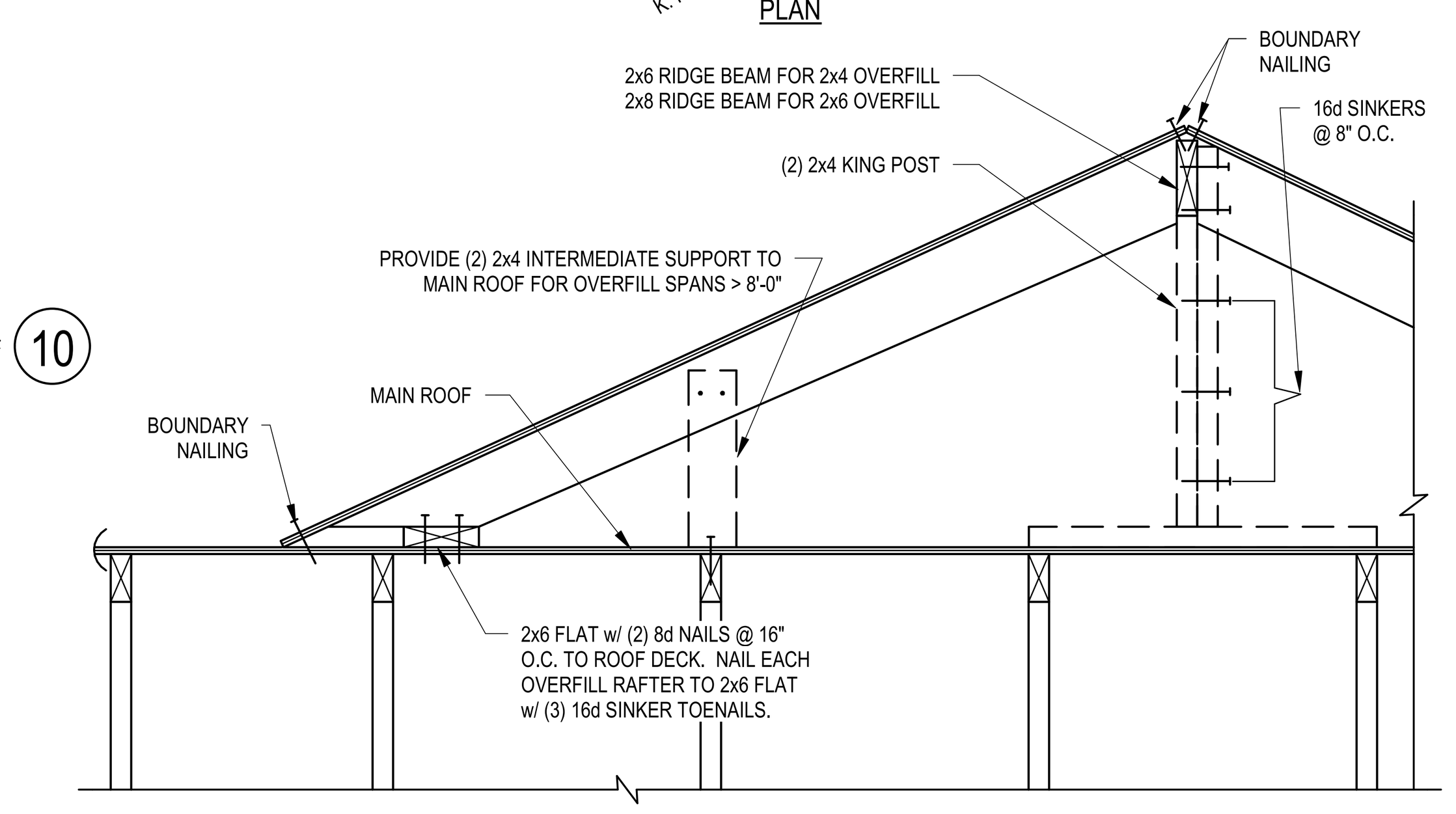
STUD NOTCH REPAIR					
	2x4 STUD	2x4 STUD	2x6 STUD	2x6 STUD	REPAIR
	NOTCH DEPTH	NOTCH LENGTH	NOTCH DEPTH	NOTCH LENGTH	
NON-BEARING & NON-SHEAR & INTERIOR...	≤ 2-1/2"	≤ 4-1/2"	≤ 3-3/4"	≤ 4-1/2"	(1) CTS218 w/ 10d
BEARING OR SHEAR OR EXTERIOR WALL	≤ 2-1/2"	≤ 2-1/2"	≤ 2-1/2"	≤ 2-1/2"	SS w/ 10d
BEARING OR SHEAR OR EXTERIOR WALL	≤ 2-3/4"	≤ 4-1/2"	≤ 4-1/2"	≤ 4-1/2"	(2) CTS218 TWO-SIDED w/ 10d

DRILLING & NOTCHING OF PLATES & STUDS (11)

N.T.S.

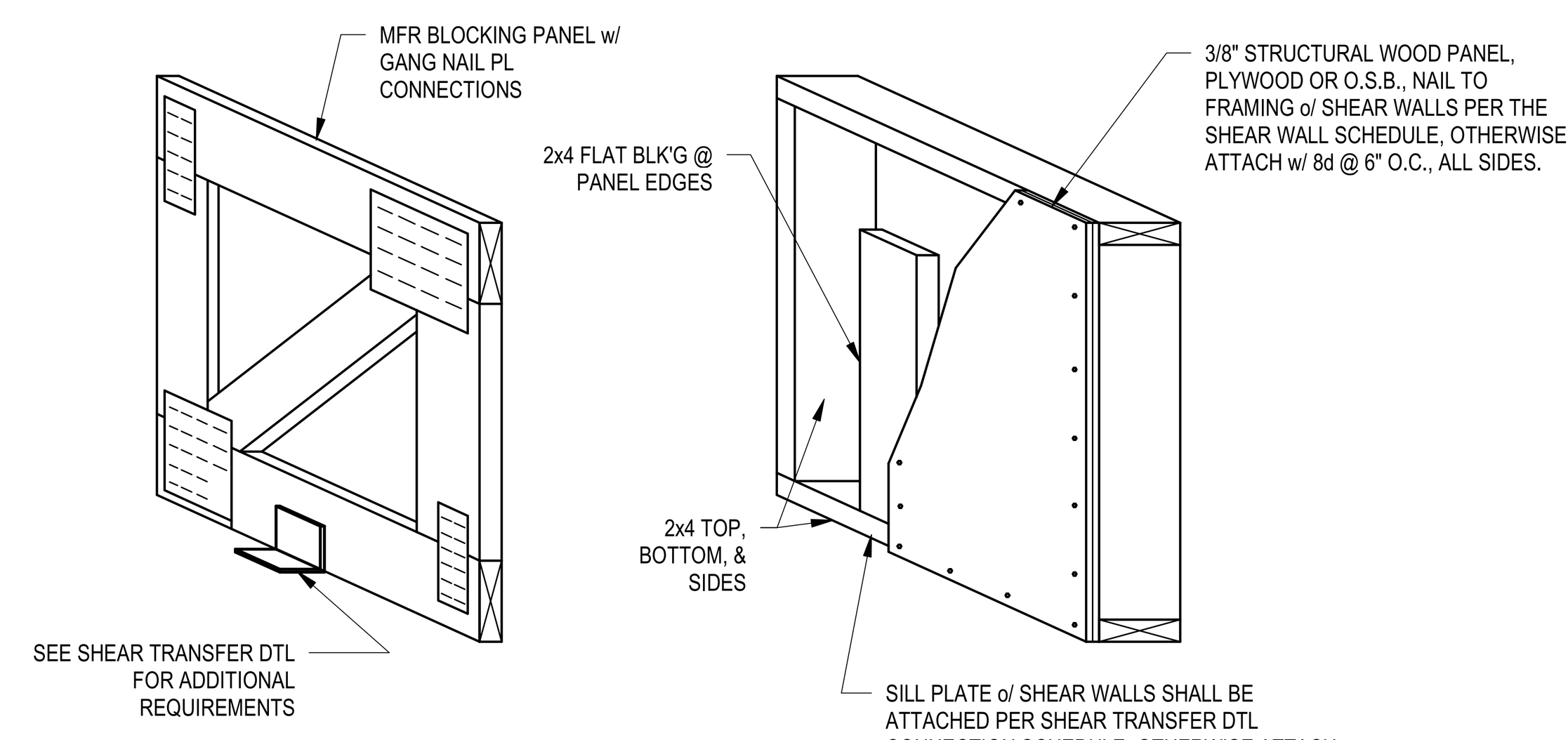


PLAN



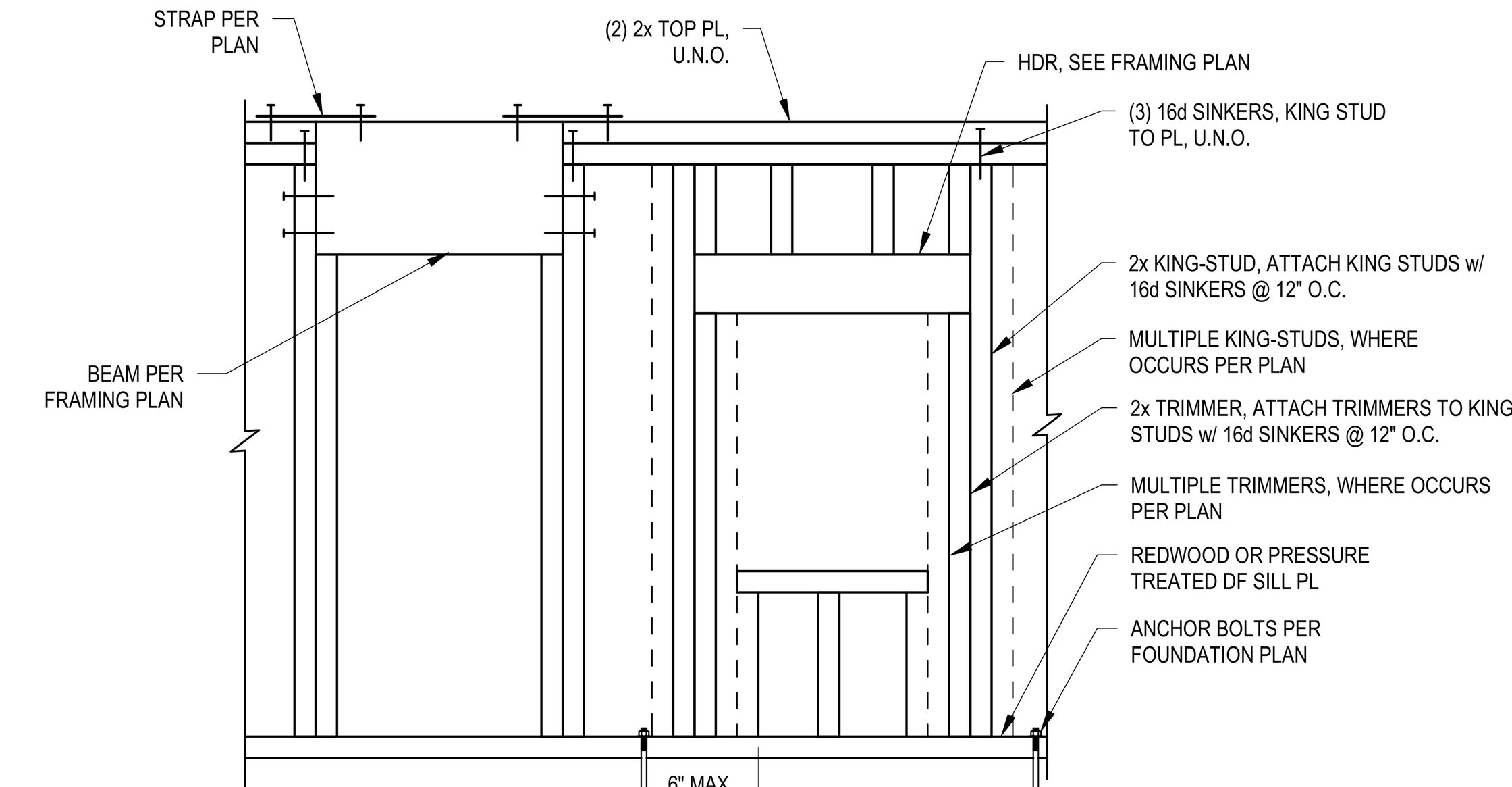
TYPICAL OVERBUILD (6)

N.T.S.



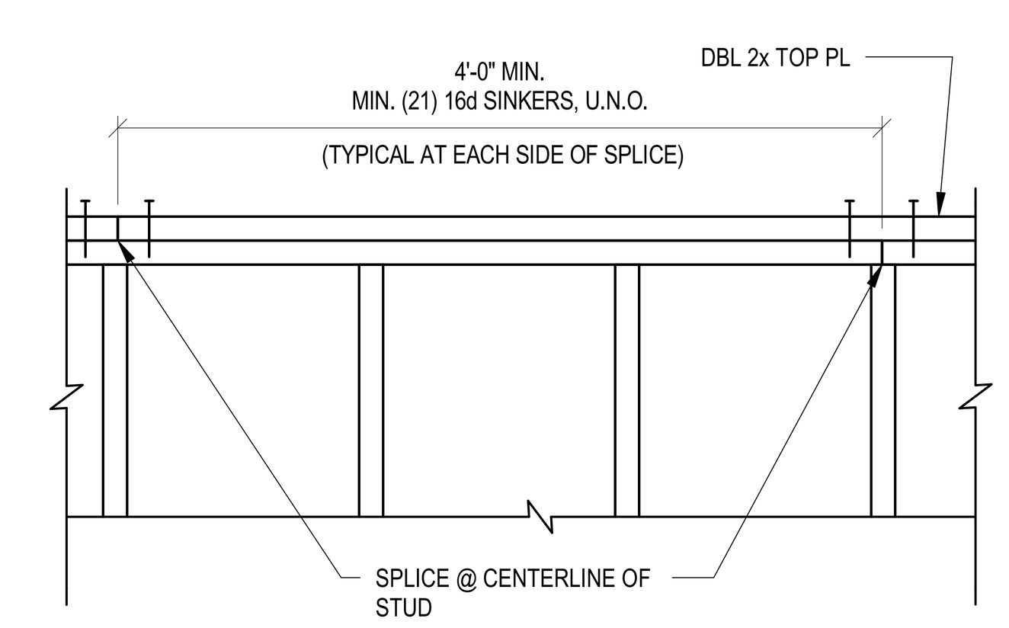
TYPICAL BLOCKING PANEL (7)

N.T.S.



TYPICAL WALL FRAMING (8)

N.T.S.



NOTE:
WHERE SPLICE LENGTH IS LESS THAN 4'-0" INSTALL ST6236 STRAP AT PLATE SPLICES. STRAPS ARE NOT REQUIRED WHERE ONE OF THE PLATES IS CONTINUOUS FOR AT LEAST 4'-0" IN EACH DIRECTION.

TYPICAL TOP PLATE SPLICE (9)

N.T.S.

SHEAR WALL SCHEDULE							
MARK	MIN. BLOCKED MATERIAL	EDGE / BOUNDARY NAILING	FIELD NAILING	SOLE PL NAILING, WHERE OCCURS	WALL CAPACITY		DEFAULT SILL ANCHORAGE, U.N.O.
					SEISMIC	WIND	
P1	3/8" PLYWOOD OR O.S.B.	8d COMMON NAILS @ 6" O.C.	8d COMMON NAILS @ 12" O.C.	16d SINKERS @ 6" O.C.	260 plf	365 plf	S1
P2	3/8" PLYWOOD OR O.S.B.	8d COMMON NAILS @ 4" O.C.	8d COMMON NAILS @ 12" O.C.	16d SINKERS @ 4" O.C.	350 plf	520 plf	S2
P3	3/8" PLYWOOD OR O.S.B.	8d COMMON NAILS @ 3" O.C.	8d COMMON NAILS @ 12" O.C	16d SINKERS @ 3" O.C.	490 plf	685 plf	S3
P4	3/8" PLYWOOD OR O.S.B.	8d COMMON NAILS @ 2" O.C.	8d COMMON NAILS @ 12" O.C.	16d SINKERS @ 2" O.C.	640 plf	895 plf	S4
P5	3/8" STRUCTURAL PLYWOOD	8d COMMON NAILS @ 2" O.C.	8d COMMON NAILS @ 12" O.C.	16d SINKERS @ 2" O.C.	730 plf	-	S4
P2 & P3 BOTH SIDES	3/8" PLYWOOD OR O.S.B.	MATCH SINGLE WALL	MATCH SINGLE WALL	16d SINKERS @ 2" O.C. (3xSOLE PLATE REQ'D)	SINGLE WALL x2		S4
P4 & P5 BOTH SIDES	MATCH SINGLE WALL	MATCH SINGLE WALL	MATCH SINGLE WALL	SDWS22600 DB SCREWS @3" O.C. STAGGERED (3xSOLE PLATE REQ'D)	SINGLE WALL x2		S5

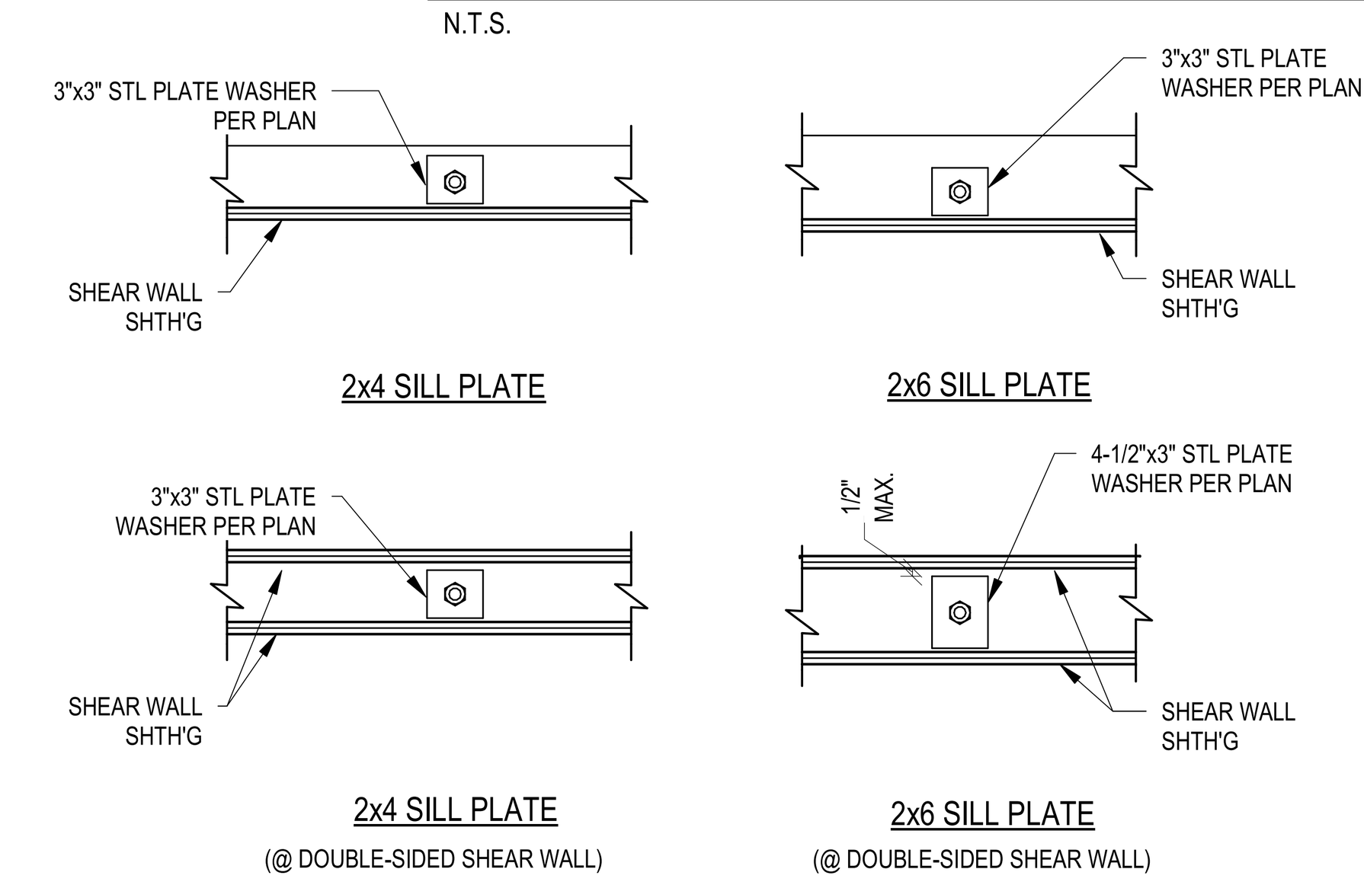
SILL ANCHORAGE SCHEDULE				
MARK	NOMINAL SILL PL THICKNESS	1/2"DIA A.B. SPACING	5/8"DIA A.B. SPACING	CAPACITY
S1	2x	32" O.C.	48" O.C.	370 plf
S2	2x	24" O.C.	32" O.C.	520 plf
S3	2x	16" O.C.	24" O.C.	740 plf
S4	2x	12" O.C.	16" O.C.	1040 plf
S5	3x	8" O.C.	10" O.C.	1560 plf

SHEAR WALL LENGTH TOLERANCES	
SPECIFIED SHEAR WALL LENGTH	ACCEPTABLE SHEAR WALL TOLERANCE
UP TO 3'-0"	± 2"
OVER 3'-0" AND UP TO 5'-0"	± 3"
OVER 5'-0" AND UP TO 7'-0"	± 4"
OVER 7'-0" AND UP TO 10'-0"	± 6"
OVER 10'-0"	± 8"

1. ALL SHEAR WALLS SHALL BE FRAMED TO THE MINIMUM LENGTHS SHOWN ON THE PLANS WITH THE TOLERANCES INDICATED ON THE TABLE ABOVE, UNLESS A MINIMUM WALL LENGTH IS SHOWN ON THE PLANS.
2. ALL SHEAR WALLS SHALL TERMINATE ON AT LEAST (1) FULL HEIGHT STUD. ADDITIONAL STUDS OR SOLID POSTS SHALL BE INSTALLED AS REQUIRED FOR HOLDOWNS WHERE THEY OCCUR.
3. 8d COMMON NAIL SHANK DIAMETER = 0.131", 16d SINKER SHANK DIAMETER = 0.148".
4. FOR "P3" AND "P4" SHEAR WALLS, ALL FRAMING RECEIVING EDGE NAILING FROM ADJOINING PANEL EDGES SHALL BE 3-INCH NOMINAL OR WIDER AND NAILS SHALL BE STAGGERED. AS AN ALTERNATE, (2) 2x STUDS MAY BE USED PROVIDED THEY ARE NAILED TOGETHER w/ (2) 16d SINKERS @ 6" O.C. FULL HEIGHT.
5. FOR "P2", "P3" AND "P4" DOUBLE-SIDED SHEAR WALLS, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, OR FRAMING SHALL BE 3-INCH NOMINAL OR WIDER AT ADJOINING PANEL EDGES AND NAILS ON EACH SIDE SHALL BE STAGGERED.
6. ALL ANCHOR BOLTS SHALL HAVE 7" MINIMUM EMBEDMENT.
7. ALL SHEAR WALL ANCHOR BOLTS SHALL INCLUDE A STEEL 3"x3"x0.229" PLATE WASHER BETWEEN THE SILL PLATE AND NUT. THE HOLE IN THE PLATE WASHER IS PERMITTED TO BE DIAGONALLY SLOTTED WITH A WIDTH OF UP TO 3/16" LARGER THAN THE BOLT DIAMETER AND A SLOT LENGTH NOT TO EXCEED 1 3/4" PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND THE NUT. ANCHOR BOLTS & PLATE WASHERS ARE TO BE OFFSET TOWARD THE SHEATHED WALL EDGE TO LIMIT THE GAP BETWEEN THE EDGE OF WASHER TO SHEATHING TO A MINIMUM OF 1/2". WHERE BOTH SIDES OF A 2x6 WALL ARE SHEATHED, A STEEL 4-1/2"x3"x0.229" PLATE WASHER SHALL BE CENTERED ON THE SILL PLATE, PER DTL 2 / S1.1.

STANDARD SHEAR WALL SCHEDULE (1)

N.T.S.



TYP. SHEAR WALL WASHERS DETAIL (2)

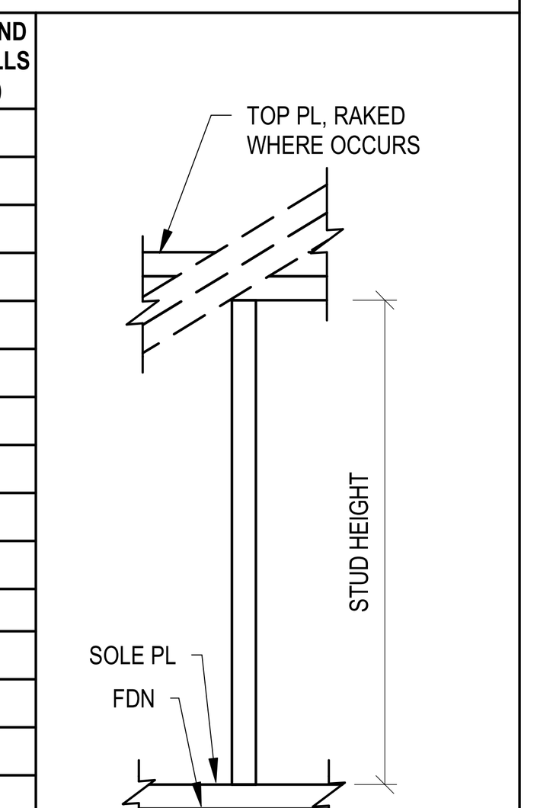
N.T.S.

FOOTING SCHEDULE		
MARK	SIZE	REINFORCING, BOTTOM
F2.0	2'-0" SQ. x 12" THICK	(3) #4 EACH WAY
F2.5	2'-6" SQ. x 12" THICK	(4) #4 EACH WAY
F3.0	3'-0" SQ. x 12" THICK	(4) #4 EACH WAY
F3.5	3'-6" SQ. x 14" THICK	(4) #5 EACH WAY
F4.0	4'-0" SQ. x 14" THICK	(4) #5 EACH WAY
F4.5	4'-6" SQ. x 16" THICK	(5) #5 EACH WAY
F5.0	5'-0" SQ. x 18" THICK	(6) #5 EACH WAY
F5.5	5'-6" SQ. x 20" THICK	(6) #5 EACH WAY
F6.0	6'-0" SQ. x 20" THICK	(8) #5 EACH WAY
F7.0	7'-0" SQ. x 24" THICK	(7) #6 EACH WAY
F8.0	8'-0" SQ. x 28" THICK	(9) #6 EACH WAY
CF2.0	2'-0" WIDE x 12" THICK	(3) #4 CONT.
CF2.5	2'-6" WIDE x 12" THICK	(4) #4 CONT.
CF3.0	3'-0" WIDE x 12" THICK	(4) #4 CONT. x #4 @ 12" O.C. C.W.
CF4.0	4'-0" WIDE x 12" THICK	(6) #4 CONT. x #5 @ 12 O.C. C.W.

FOOTING SCHEDULE (3)

N.T.S.

STUD HEIGHT TABLE			
STUD WALL TYPE	BEARING AND/OR SHEAR WALLS (MAX. HEIGHT)		NON-BEARING AND NON-SHEAR WALLS (MAX. HEIGHT)
	EXTERIOR	INTERIOR	INTERIOR
2x4 STUD @ 16" O.C.	-	10'-0"	13'-0"
2x4 STUD @ 12" O.C.	-	11'-6"	14'-0"
(2) 2x4 STUD @ 16" O.C.	-	13'-6"	14'-0"
2x4 DFL #2 @ 16" O.C.	-	11'-0"	13'-0"
2x4 DFL #2 @ 12" O.C.	-	13'-0"	14'-0"
(2) 2x4 DFL #2 @ 16" O.C.	-	13'-6"	14'-0"
2x6 STUD @ 16" O.C.	14'-6"	19'-0"	20'-0"
2x6 STUD @ 12" O.C.	17'-0"	21'-0"	22'-0"
(2) 2x6 STUD @ 16" O.C.	21'-0"	22'-0"	22'-6"
2x6 DFL #2 @ 16" O.C.	16'-6"	19'-6"	20'-0"
2x6 DFL #2 @ 12" O.C.	18'-6"	21'-6"	22'-0"
(2) 2x6 DFL #2 @ 16" O.C.	22'-6"	22'-6"	22'-6"
2x8 DFL #2 @ 16" O.C.	22'-0"	26'-6"	27'-0"
2x8 DFL #2 @ 12" O.C.	25'-6"	28'-0"	30'-0"
(2) 2x8 DFL #2 @ 16" O.C.	29'-6"	29'-6"	30'-0"
1-3/4 x 5-1/2 LVL STUDS @ 16" O.C.	20'-6"	21'-6"	22'-0"
1-3/4 x 7-1/4 LVL STUDS @ 16" O.C.	27'-0"	30'-0"	30'-0"



NOTES:
1. THIS TABLE ASSUMES IBC WIND LOADS w/ 115 mph, EXP. "C" AT EXTERIOR WALLS & 5 psf LATERAL LOAD AT INTERIOR WALLS.
2. THIS TABLE ASSUMES AXIAL DL = 710 lb/ft, LL = 760 lb/ft. AT EXTERIOR AND INTERIOR WALLS.
3. THIS TABLE ASSUMES IBC 5psf LATERAL LOAD @ INTERIOR WALLS.
4. THIS TABLE IS ONLY APPLICABLE FOR THE UPPER TWO FLOORS BEARING WALLS AND NON-BEARING WALLS.
SEE FRAMING PLAN NOTES FOR WALL CALLOUTS

STANDARD STUD TABLE (4)

N.T.S.

NOT USED (5)

N.T.S.

100% CD

VECTOR ENGINEERS

CONSULTANT:

OWNER:

ELEMENT HOTEL

PROJECT:

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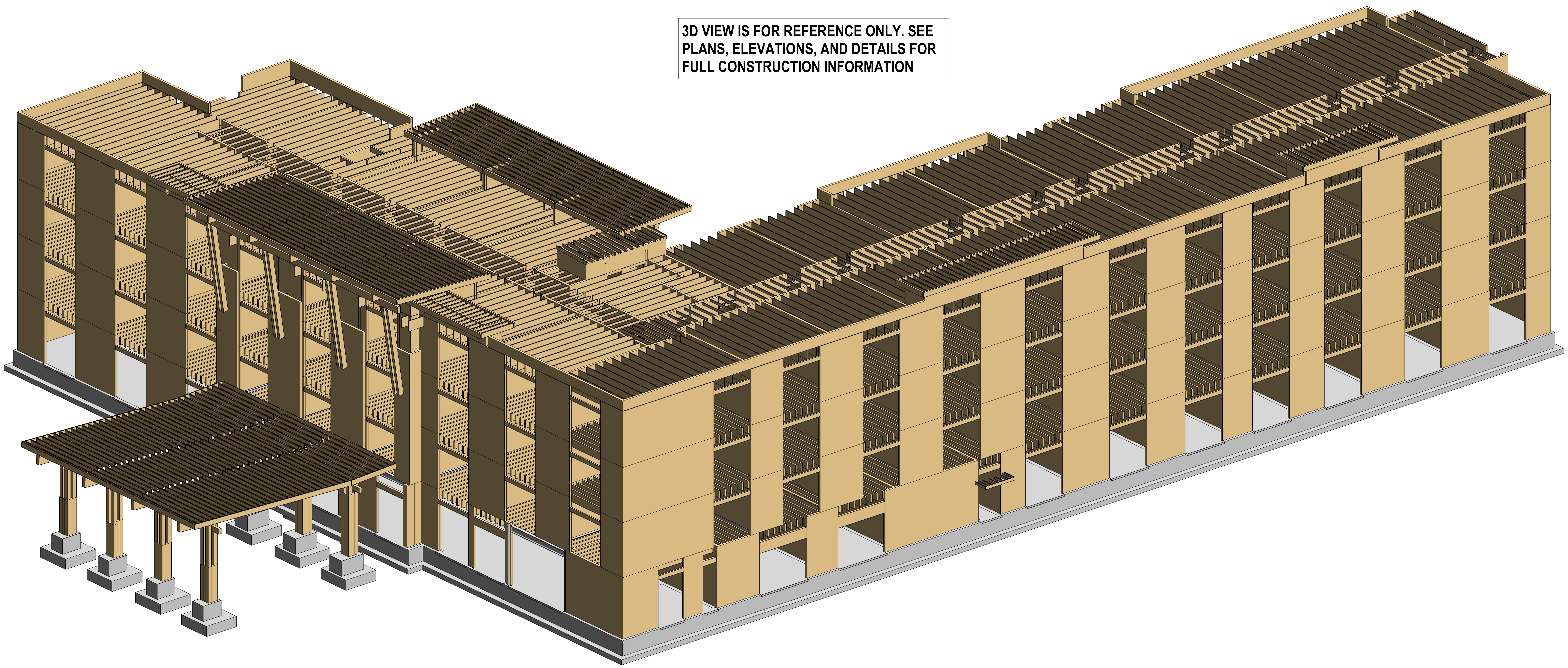
STANDARD DETAILS AND SCHEDULES

PROJECT #: U1477.005.221
DRAWN BY: SDC
CHECKED BY: BSE

SHEET #
S1.1



3D VIEW IS FOR REFERENCE ONLY. SEE PLANS, ELEVATIONS, AND DETAILS FOR FULL CONSTRUCTION INFORMATION

VECTOR ENGINEERS
BOISE, IDAHO
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ID FIRM LICENSE NUMBER 10449-0

CONSULTANT:

OWNER:

ELEMENT HOTEL

PROJECT:

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3D VIEW

PROJECT #: U1477.005.221
DRAWN BY: SDC
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SHEET #
S1.2

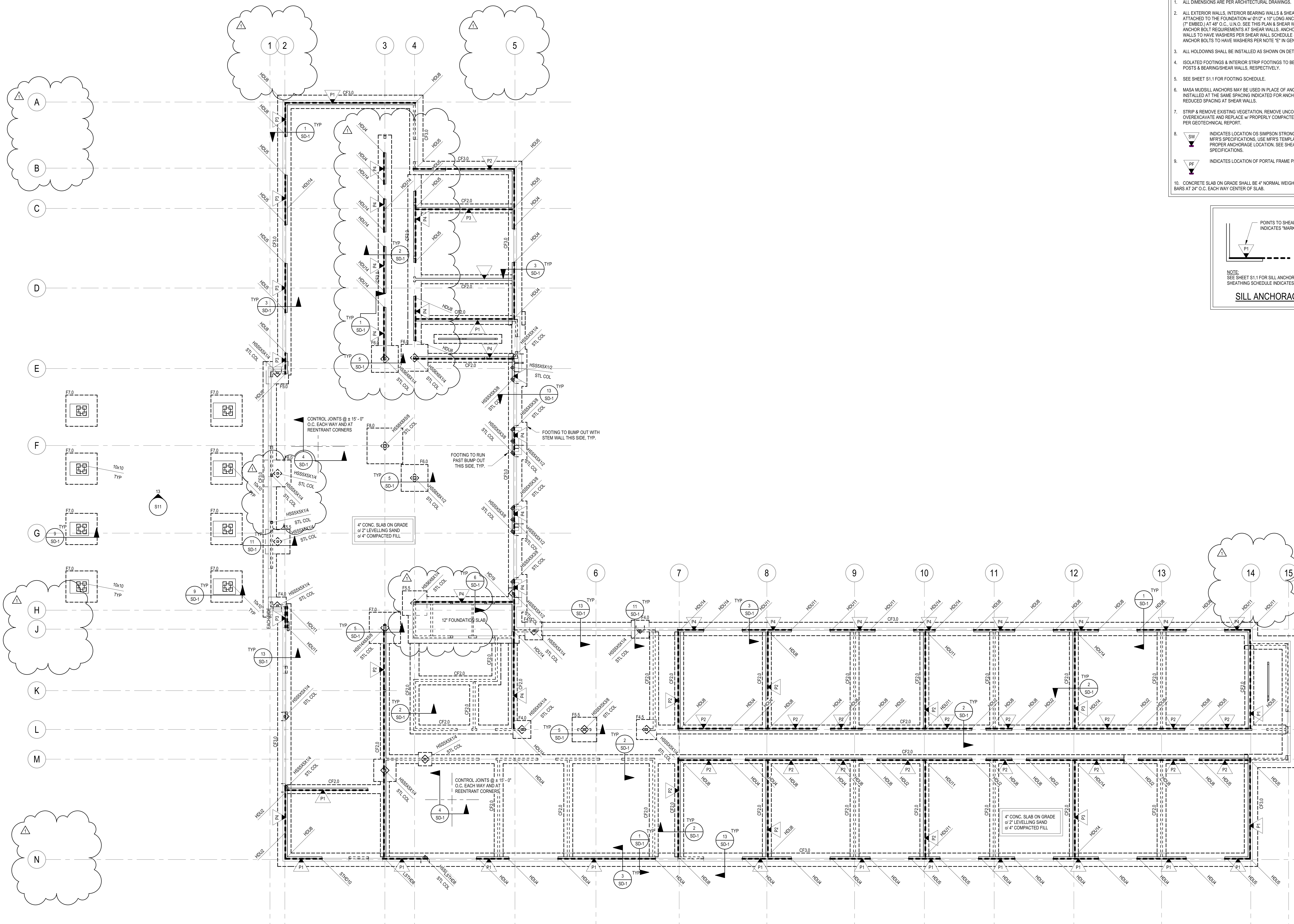
FOUNDATION NOTES:

1. ALL DIMENSIONS ARE PER ARCHITECTURAL DRAWINGS.
2. ALL EXTERIOR WALLS, INTERIOR BEARING WALLS & SHEAR WALLS TO BE ATTACHED TO THE FOUNDATION w/ Ø1/2" x 10" LONG ANCHOR BOLTS (7" EMBED.) AT 48" O.C., U.N.O. SEE THIS PLAN & SHEAR WALL SCHEDULE FOR ANCHOR BOLT REQUIREMENTS AT SHEAR WALLS. ANCHOR BOLTS AT SHEAR WALLS TO HAVE WASHERS PER SHEAR WALL SCHEDULE (S1.1). ALL OTHER ANCHOR BOLTS TO HAVE WASHERS PER NOTE "E" IN GENERAL NOTES (S1).
3. ALL HOLDOWNS SHALL BE INSTALLED AS SHOWN ON DETAIL C / SD-1.
4. ISOLATED FOOTINGS & INTERIOR STRIP FOOTINGS TO BE CENTERED BELOW POSTS & BEARING/SHEAR WALLS, RESPECTIVELY.
5. SEE SHEET S1.1 FOR FOOTING SCHEDULE.
6. MASA MUDSILL ANCHORS MAY BE USED IN PLACE OF ANCHOR BOLTS, INSTALLED AT THE SAME SPACING INDICATED FOR ANCHOR BOLTS, INCLUDING REDUCED SPACING AT SHEAR WALLS.
7. STRIP & REMOVE EXISTING VEGETATION, REMOVE UNCONTROLLED FILL, OVEREXCAVATE AND REPLACE w/ PROPERLY COMPACTED FILL AS REQUIRED PER GEOTECHNICAL REPORT.
8. SW — INDICATES LOCATION OS SIMPSON STRONG WALL, INSTALL PER MFR'S SPECIFICATIONS, USE MFR'S TEMPLATE TO ENSURE PROPER ANCHORAGE LOCATION. SEE SHEAR WALL PLANS FOR SPECIFICATIONS.
9. PF — INDICATES LOCATION OF PORTAL FRAME PER SHEAR WALL PLAN
10. CONCRETE SLAB ON GRADE SHALL BE 4" NORMAL WEIGHT CONCRETE WITH #4 BARS AT 24" O.C. EACH WAY CENTER OF SLAB.

POINTS TO SHEAR WALL & INDICATES "MARK"
P1

NOTE:
SEE SHEET S1.1 FOR SILL ANCHORAGE SCHEDULE
SHEATHING SCHEDULE INDICATES SILL ANCHORAGE

SILL ANCHORAGE KEY



FOUNDATION PLAN

1/8" = 1'-0"

VECTOR ENGINEERS
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OWNER:

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PROJECT:

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ISSUE DATE	DESCRIPTION

#	REV. DATE	DESCRIPTION
1		

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FOUNDATION PLAN

PROJECT #: U1477.005.221
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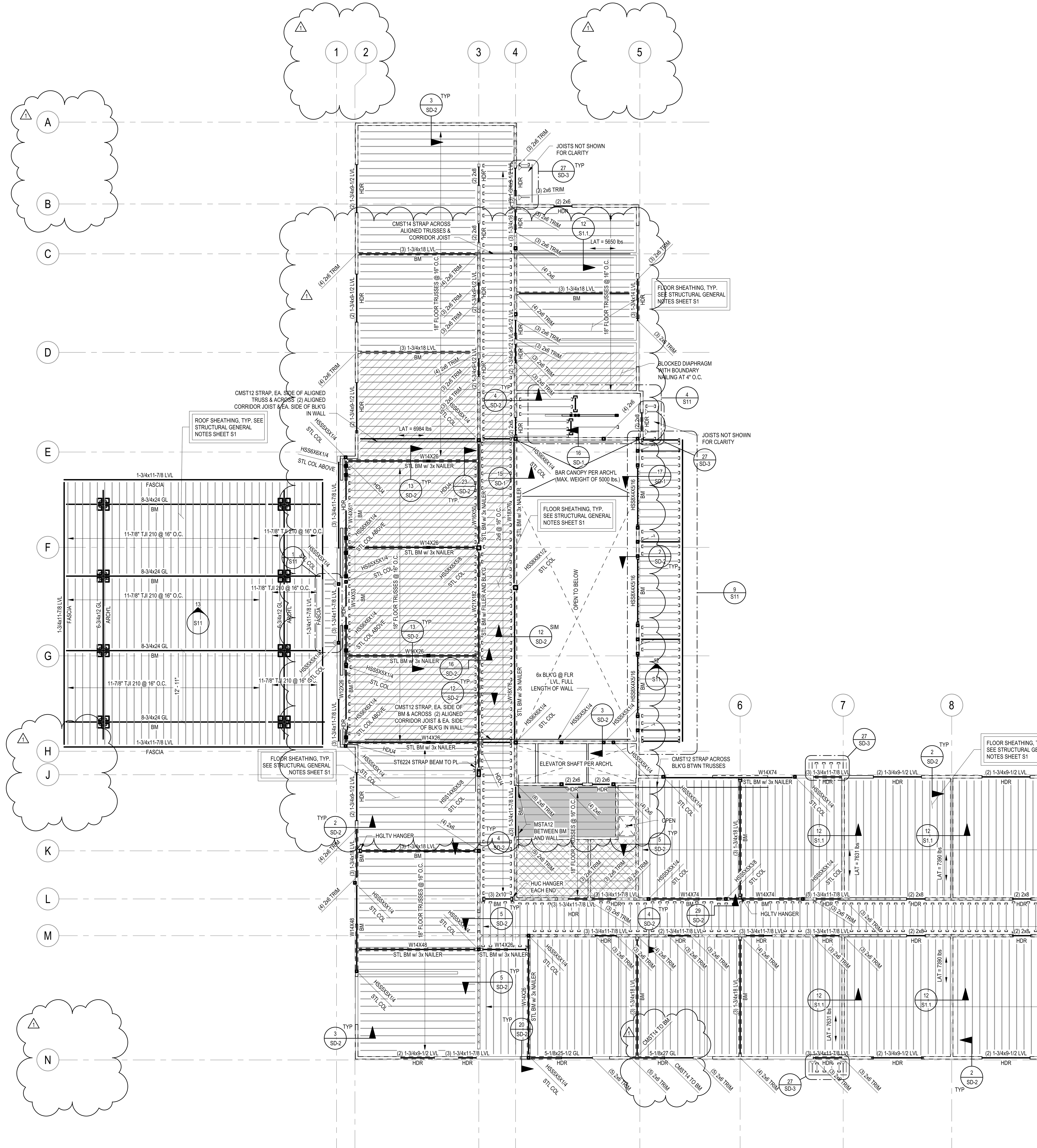
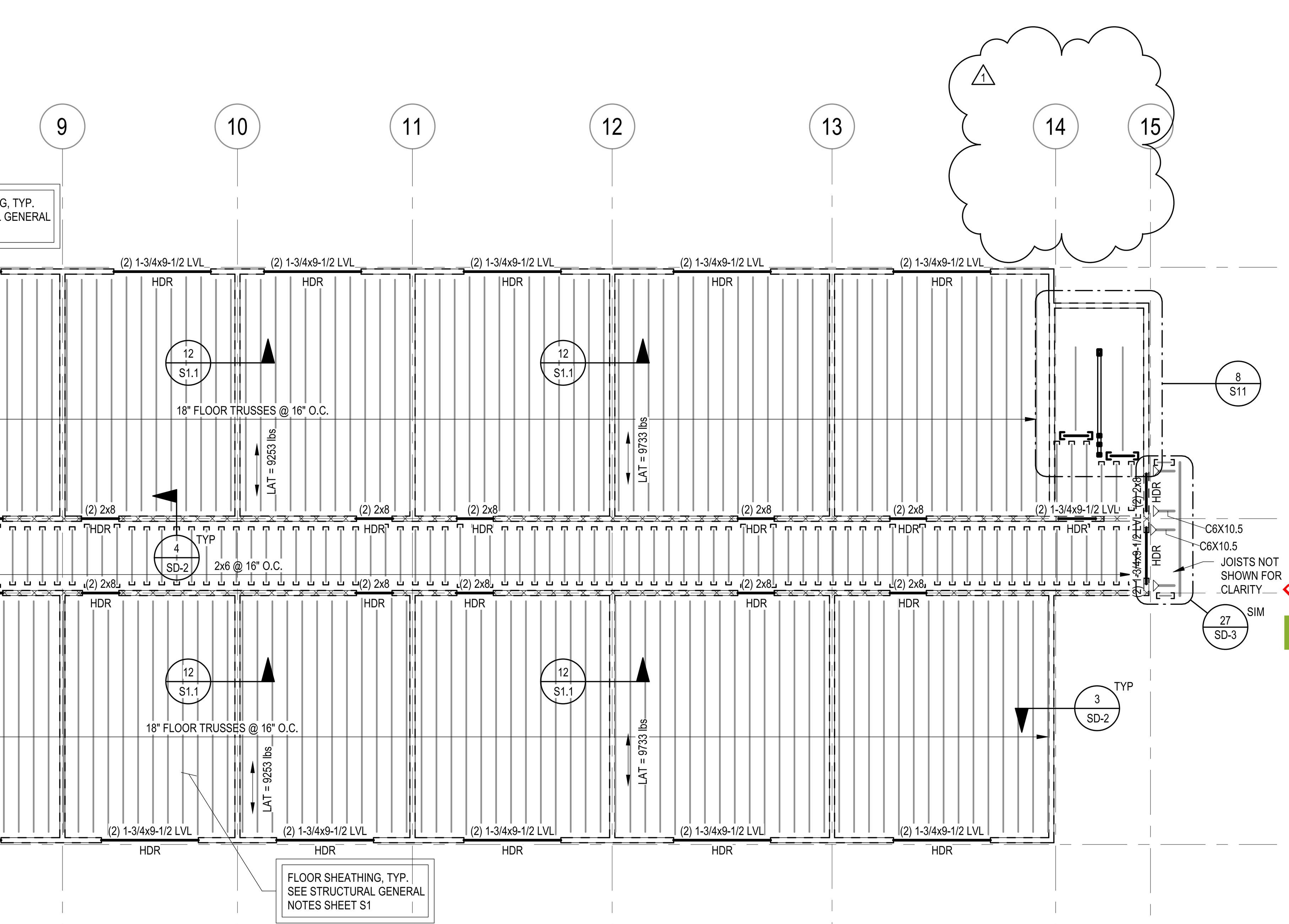
SHEET #
S2

LEVEL 2 FRAMING PLAN

1/8" = 1'-0"

FLOOR TRUSS LOADING LEGEND

EXIT/CORRIDOR AREAS, 100 psf LIVE LOAD:

STORAGE AREAS, 125 psf LIVE LOAD:

NOTE:

1. FLOOR TRUSSES IN AREAS NOT DESIGNATED BY HATCH TO BE DESIGNED FOR 40 psf LIVE LOAD
2. ALL FLOOR TRUSSES TO BE DESIGNED FOR A 20 psf DEAD LOAD
3. INSTALL FLOOR SHEATHING AT HIGH LOAD AREAS INDICATED ABOVE WITH STRENGTH AXIS APPLIED PERPENDICULAR TO THE SUPPORTS
4. FLOOR TRUSS SPACING CAN BE ADJUSTED UP TO 24" O.C. BY SUPPLIER TO OPTIMIZE LAYOUT. TRUSS DEPTH LIMITED TO 18" MAX.
5. DRAG TRUSSES AT SHEAR WALLS SHALL BE DESIGNED TO TRANSFER THE AXIAL/LATERAL SHEAR LOAD INDICATED ON PLAN BY 'LAT = XX" FROM THE TOP CHORD TO THE BOTTOM CHORD AND ATTACHED TO THE SHEAR WALL PER DETAIL 12 / S1.1.
6. BRACE TRUSSES TO BRACE STEEL COLUMNS SHALL BE DESIGNED FOR A TENSION LOAD OF 3500 lbs. (FACTORED WIND UPLIFT) AND A COMPRESSION LOAD OF 6000 lbs. (FACTORED GRAVITY) APPLIED TO THE TOP CHORD OF THE TRUSS AT THE COLUMN END.

FRAMING NOTES:

1. ALL FRAMED WALLS TO BE 2x @ 16" O.C. (MAX) PER ARCHITECTURAL PLANS AND SHALL MEET REQUIREMENTS OF WALL TABLE ON SHEET S1.1.
   A. ALL FRAMED EXTERIOR WALLS BELOW LEVEL 2 SHALL BE 2x6 DFL #2 @ 12" O.C. (MAX.)
   B. ALL FRAMED EXTERIOR WALLS ABOVE LEVEL 2 SHALL BE 2x6 DFL #2 @ 16" O.C. (MAX.)
   C. ALL INTERIOR BEARING WALLS SHALL BE 2x6 DFL #2 @ 16" O.C. (MAX.)
   D. ALL WALLS TO MEET THE REQUIREMENTS OF THE STUD TABLE ON DTL 4 / S1.1.

2. FOR EXTERIOR 2x6 FRAMED WALLS AT STACKED HEADERS (HDR):
   A. AT OPENINGS < 4'-0" PROVIDE:
      (1) 2x6 TRIMMER (T) & (1) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (2) 2x6 K BELOW LEVEL 4 FRAMING
      (1) 2x6 T & (3) 2x6 K BELOW LEVEL 3 FRAMING
      (1) 2x6 T & (4) 2x6 K BELOW LEVEL 2 FRAMING
   B. AT OPENINGS > 4'-0" AND < 9'-0" PROVIDE:
      (2) 2x6 TRIMMER (T) & (2) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (3) 2x6 K BELOW LEVEL 4 FRAMING
      (1) 2x6 T & (4) 2x6 K BELOW LEVEL 3 FRAMING
      (1) 2x6 T & (5) 2x6 K BELOW LEVEL 2 FRAMING

4. FOR INTERIOR 2x6 FRAMED WALLS AT STACKED HEADERS (HDR):
   A. AT OPENINGS < 4'-0" PROVIDE:
      (1) 2x6 TRIMMER (T) & (1) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (2) 2x6 K BELOW LEVEL 3 FRAMING

5. TRIMMERS AT BEAMS (BM) SHALL BE FULL HEIGHT. KING STUDS ARE NOT REQUIRED.

6. AT TRIMMERS NOTED ON PLAN ADD (1) KING STUD FOR INTERIOR HEADERS OR (2) KING STUDS FOR EXTERIOR HEADERS

7. ALL TRIMMERS AND KING STUDS SHALL BE DFL #2 OR BETTER

8. PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WITH POSTS, CRIPPLES, AND SQUASH BLOCKS AS REQUIRED

9. FACE NAIL MULTIPLE 2x POSTS WITH 16d SINKERS @ 6" O.C.

10. SHADED AREAS ARE TYPICAL OVERFILL, STICK FRAMED PER DETAIL 6 / S1.1 OR OVERBUILD TRUSSES PER TRUSS MANUFACTURER

11. INTERIOR BEARING WALLS

12. ALL GLULAM BEAMS TO HAVE STANDARD CAMBER (R = 2000') U.N.O.

13. PROVIDE (2) 2x POST EACH END OF ALL BEAMS & GIRDER TRUSSES, U.N.O. PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WITH POSTS, CRIPPLES, AND SQUASH BLOCKS AS REQUIRED.

14. BEAM AND HEADER SIZES INDICATED ON THE PLANS ARE MINIMUM SIZES. LARGER SIZES MAY BE INSTALLED AT THE CONTRACTOR'S OPTION.

15. CONTINUOUS TOP PLATE MAY BE USED IN LIEU OF ST6224 STRAP FROM BEAM TO PLATE.

16. SEE ARCH'L FOR ROOF SLOPE AND TOP OF STEEL ELEVATIONS.

17. DASHED WALLS INDICATE WALLS ABOVE

18. AT NOTES FOR INVERTED HANGERS SEE DTL 22 / SD-3 FOR FULL CONNECTION INFORMATION

JOIST HANGER SCHEDULE

JOIST LENGTH	FACE MOUNTED HANGER		TOP FLANGE HANGER	
	FLOOR	ROOF	FLOOR	ROOF
CORRIDOR 2x6	LUS26	LUS26	JB	JB
STAIR LANDINGS	LUS28	-	JB	-
UP TO 16'-0"	IUS	HU	ITS	ITS
16'-0" TO 25'-0"	IUS	HU	ITS	MIT
25'-0" TO 32'-0"	HU	MIU	ITS	WP
32'-0" TO 40'-0"	HU	-	MIT	BA

VECTOR ENGINEERS
BOISE, IDAHO
(208) 996-0303
WWW.VECTORSE.COM
ID FIRM LICENSE NUMBER 10449-D

CONSULTANT:

OWNER:

ELEMENT HOTEL

PROJECT:

#	ISSUE DATE	DESCRIPTION
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1		

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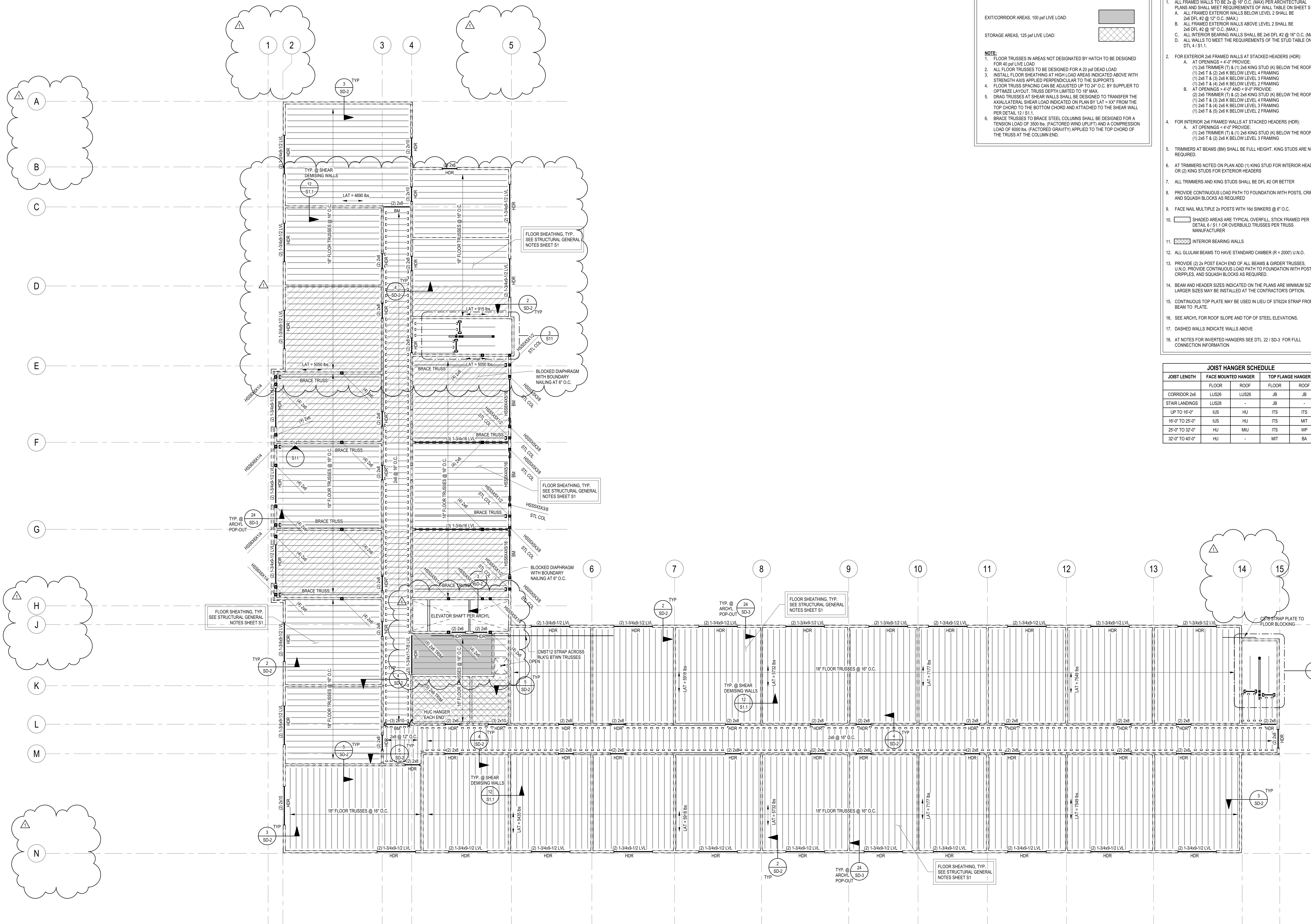
02/24/2025 10:13:22 AM

100% CD

LEVEL 2 FRAMING PLAN

PROJECT #: U1477.005.221
DRAWN BY: SDC
CHECKED BY: BSE

SHEET #
S3



LEVEL 3 FRAMING PLAN

1/8" = 1'-0"

FLOOR TRUSS LOADING LEGEND

EXIT/CORRIDOR AREAS, 100 psf LIVE LOAD: (solid grey shading)

STORAGE AREAS, 125 psf LIVE LOAD: (cross-hatch)

NOTE:

1. FLOOR TRUSSES IN AREAS NOT DESIGNATED BY HATCH TO BE DESIGNED FOR 40 psf LIVE LOAD
2. ALL FLOOR TRUSSES TO BE DESIGNED FOR A 20 psf DEAD LOAD
3. INSTALL FLOOR SHEATHING AT HIGH LOAD AREAS INDICATED ABOVE WITH STRENGTH AXIS APPLIED PERPENDICULAR TO THE SUPPORTS
4. FLOOR TRUSS SPACING CAN BE ADJUSTED UP TO 24" O.C. BY SUPPLIER TO OPTIMIZE LAYOUT. TRUSS DEPTH LIMITED TO 18" MAX.
5. DRAG TRUSSES AT SHEAR WALLS SHALL BE DESIGNED TO TRANSFER THE AXIAL/LATERAL SHEAR LOAD INDICATED ON PLAN BY 'LAT = XX" FROM THE TOP CHORD TO THE BOTTOM CHORD AND ATTACHED TO THE SHEAR WALL PER DETAIL 12 / S1.1.
6. BRACE TRUSSES TO BRACE STEEL COLUMNS SHALL BE DESIGNED FOR A TENSION LOAD OF 3500 lbs. (FACTORED WIND UPLIFT) AND A COMPRESSION LOAD OF 6000 lbs. (FACTORED GRAVITY) APPLIED TO THE TOP CHORD OF THE TRUSS AT THE COLUMN END.

FRAMING NOTES:

1. ALL FRAMED WALLS TO BE 2x @ 16" O.C. (MAX) PER ARCHITECTURAL PLANS AND SHALL MEET REQUIREMENTS OF WALL TABLE ON SHEET S1.1.
   A. ALL FRAMED EXTERIOR WALLS BELOW LEVEL 2 SHALL BE 2x6 DFL #2 @ 12" O.C. (MAX.)
   B. ALL FRAMED EXTERIOR WALLS ABOVE LEVEL 2 SHALL BE 2x6 DFL #2 @ 16" O.C. (MAX.)
   C. ALL INTERIOR BEARING WALLS SHALL BE 2x6 DFL #2 @ 16" O.C. (MAX.)
   D. ALL WALLS TO MEET THE REQUIREMENTS OF THE STUD TABLE ON DTL 4 / S1.1.
2. FOR EXTERIOR 2x6 FRAMED WALLS AT STACKED HEADERS (HDR):
   A. AT OPENINGS < 4'-0" PROVIDE:
      (1) 2x6 TRIMMER (T) & (1) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (2) 2x6 K BELOW LEVEL 4 FRAMING
      (1) 2x6 T & (3) 2x6 K BELOW LEVEL 3 FRAMING
      (1) 2x6 T & (4) 2x6 K BELOW LEVEL 2 FRAMING
   B. AT OPENINGS > 4'-0" AND < 9'-0" PROVIDE:
      (2) 2x6 TRIMMER (T) & (2) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (3) 2x6 K BELOW LEVEL 4 FRAMING
      (1) 2x6 T & (4) 2x6 K BELOW LEVEL 3 FRAMING
      (1) 2x6 T & (5) 2x6 K BELOW LEVEL 2 FRAMING
4. FOR INTERIOR 2x6 FRAMED WALLS AT STACKED HEADERS (HDR):
   A. AT OPENINGS < 4'-0" PROVIDE:
      (1) 2x6 TRIMMER (T) & (1) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (2) 2x6 K BELOW LEVEL 3 FRAMING
5. TRIMMERS AT BEAMS (BM) SHALL BE FULL HEIGHT. KING STUDS ARE NOT REQUIRED.
6. AT TRIMMERS NOTED ON PLAN ADD (1) KING STUD FOR INTERIOR HEADERS OR (2) KING STUDS FOR EXTERIOR HEADERS
7. ALL TRIMMERS AND KING STUDS SHALL BE DFL #2 OR BETTER
8. PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WITH POSTS, CRIPPLES, AND SQUASH BLOCKS AS REQUIRED
9. FACE NAIL MULTIPLE 2x POSTS WITH 16d SINKERS @ 6" O.C.
10. (dotted box) SHADED AREAS ARE TYPICAL OVERFILL, STICK FRAMED PER DETAIL 6 / S1.1 OR OVERBUILD TRUSSES PER TRUSS MANUFACTURER
11. (cross-hatch box) INTERIOR BEARING WALLS
12. ALL GLULAM BEAMS TO HAVE STANDARD CAMBER (R = 2000') U.N.O.
13. PROVIDE (2) 2x POST EACH END OF ALL BEAMS & GIRDER TRUSSES, U.N.O. PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WITH POSTS, CRIPPLES, AND SQUASH BLOCKS AS REQUIRED.
14. BEAM AND HEADER SIZES INDICATED ON THE PLANS ARE MINIMUM SIZES. LARGER SIZES MAY BE INSTALLED AT THE CONTRACTOR'S OPTION.
15. CONTINUOUS TOP PLATE MAY BE USED IN LIEU OF ST6224 STRAP FROM BEAM TO PLATE.
16. SEE ARCH'L FOR ROOF SLOPE AND TOP OF STEEL ELEVATIONS.
17. DASHED WALLS INDICATE WALLS ABOVE
18. AT NOTES FOR INVERTED HANGERS SEE DTL 22 / SD-3 FOR FULL CONNECTION INFORMATION

JOIST HANGER SCHEDULE

JOIST LENGTH	FACE MOUNTED HANGER		TOP FLANGE HANGER	
	FLOOR	ROOF	FLOOR	ROOF
CORRIDOR 2x6	LUS26	LUS26	JB	JB
STAIR LANDINGS	LUS28	-	JB	-
UP TO 16'-0"	IUS	HU	ITS	ITS
16'-0" TO 25'-0"	IUS	HU	ITS	MIT
25'-0" TO 32'-0"	HU	MIU	ITS	WP
32'-0" TO 40'-0"	HU	-	MIT	BA

VECTOR ENGINEERS

CONSULTANT:

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LEVEL 3 FRAMING PLAN

PROJECT #: U1477.005.221
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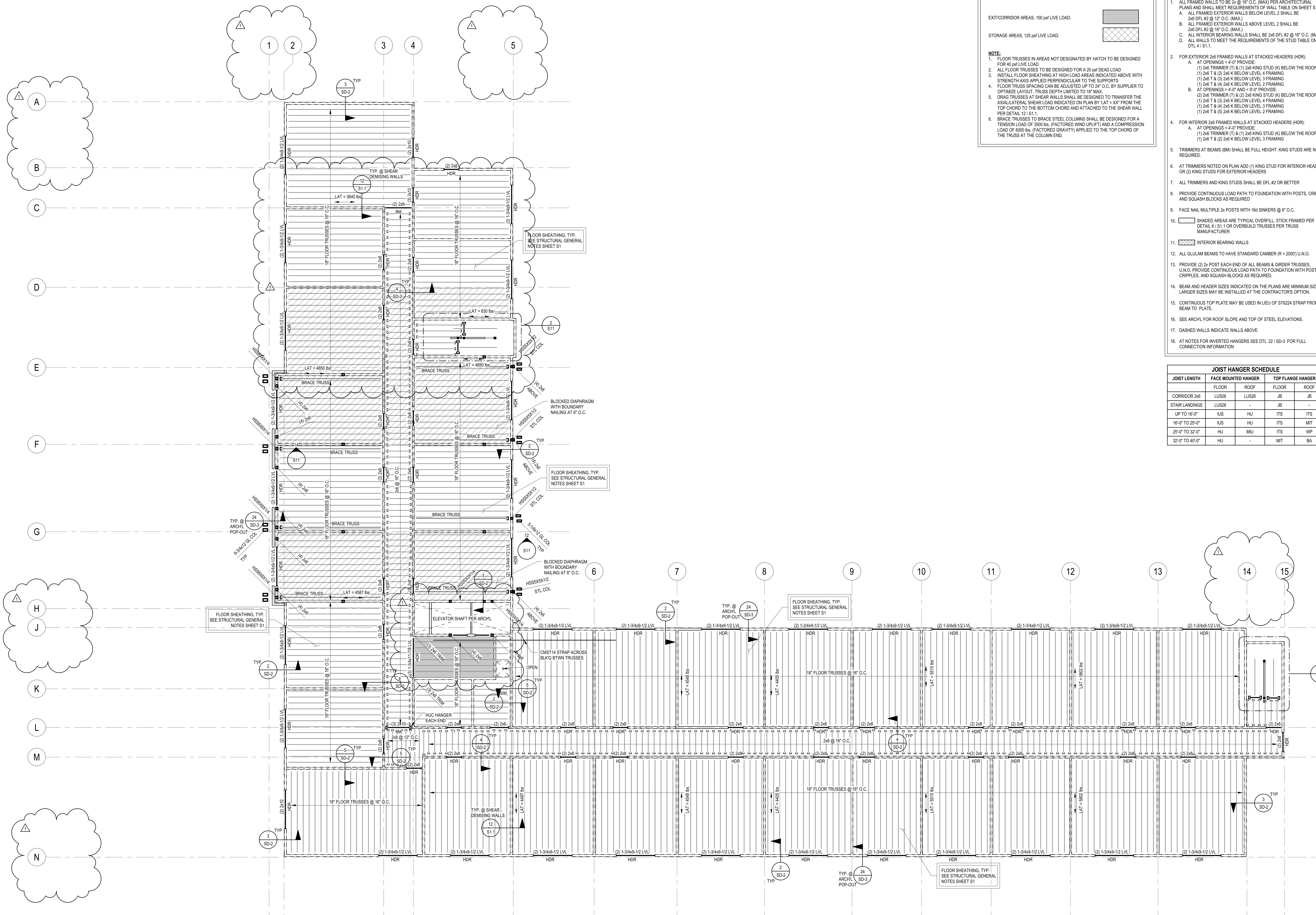
SHEET #
S4



LEVEL 4 FRAMING PLAN

1/8" = 1'-0"

FLOOR TRUSS LOADING LEGEND

EXIT/CORRIDOR AREAS, 100 psf LIVE LOAD:

STORAGE AREAS, 125 psf LIVE LOAD:

NOTE:

1. FLOOR TRUSSES IN AREAS NOT DESIGNATED BY HATCH TO BE DESIGNED FOR 40 psf LIVE LOAD
2. ALL FLOOR TRUSSES TO BE DESIGNED FOR A 20 psf DEAD LOAD
3. INSTALL FLOOR SHEATHING AT HIGH LOAD AREAS INDICATED ABOVE WITH STRENGTH AXIS APPLIED PERPENDICULAR TO THE SUPPORTS
4. FLOOR TRUSS SPACING CAN BE ADJUSTED UP TO 24" O.C. BY SUPPLIER TO OPTIMIZE LAYOUT. TRUSS DEPTH LIMITED TO 18" MAX.
5. DRAG TRUSSES AT SHEAR WALLS SHALL BE DESIGNED TO TRANSFER THE AXIAL/LATERAL SHEAR LOAD INDICATED ON PLAN BY 'LAT = XX" FROM THE TOP CHORD TO THE BOTTOM CHORD AND ATTACHED TO THE SHEAR WALL PER DETAIL 12 / S1.1.
6. BRACE TRUSSES TO BRACE STEEL COLUMNS SHALL BE DESIGNED FOR A TENSION LOAD OF 3500 lbs. (FACTORED WIND UPLIFT) AND A COMPRESSION LOAD OF 6000 lbs. (FACTORED GRAVITY) APPLIED TO THE TOP CHORD OF THE TRUSS AT THE COLUMN END.

FRAMING NOTES:

1. ALL FRAMED WALLS TO BE 2x @ 16" O.C. (MAX) PER ARCHITECTURAL PLANS AND SHALL MEET REQUIREMENTS OF WALL TABLE ON SHEET S1.1.
   A. ALL FRAMED EXTERIOR WALLS BELOW LEVEL 2 SHALL BE 2x6 DFL #2 @ 12" O.C. (MAX.)
   B. ALL FRAMED EXTERIOR WALLS ABOVE LEVEL 2 SHALL BE 2x6 DFL #2 @ 16" O.C. (MAX.)
   C. ALL INTERIOR BEARING WALLS SHALL BE 2x6 DFL #2 @ 16" O.C. (MAX.)
   D. ALL WALLS TO MEET THE REQUIREMENTS OF THE STUD TABLE ON DTL 4 / S1.1.
2. FOR EXTERIOR 2x6 FRAMED WALLS AT STACKED HEADERS (HDR):
   A. AT OPENINGS < 4'-0" PROVIDE:
      (1) 2x6 TRIMMER (T) & (1) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (2) 2x6 K BELOW LEVEL 4 FRAMING
      (1) 2x6 T & (3) 2x6 K BELOW LEVEL 3 FRAMING
      (1) 2x6 T & (4) 2x6 K BELOW LEVEL 2 FRAMING
   B. AT OPENINGS > 4'-0" AND < 9'-0" PROVIDE:
      (2) 2x6 TRIMMER (T) & (2) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (3) 2x6 K BELOW LEVEL 4 FRAMING
      (1) 2x6 T & (4) 2x6 K BELOW LEVEL 3 FRAMING
      (1) 2x6 T & (5) 2x6 K BELOW LEVEL 2 FRAMING
4. FOR INTERIOR 2x6 FRAMED WALLS AT STACKED HEADERS (HDR):
   A. AT OPENINGS < 4'-0" PROVIDE:
      (1) 2x6 TRIMMER (T) & (1) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (2) 2x6 K BELOW LEVEL 3 FRAMING
5. TRIMMERS AT BEAMS (BM) SHALL BE FULL HEIGHT. KING STUDS ARE NOT REQUIRED.
6. AT TRIMMERS NOTED ON PLAN ADD (1) KING STUD FOR INTERIOR HEADERS OR (2) KING STUDS FOR EXTERIOR HEADERS
7. ALL TRIMMERS AND KING STUDS SHALL BE DFL #2 OR BETTER
8. PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WITH POSTS, CRIPPLES, AND SQUASH BLOCKS AS REQUIRED
9. FACE NAIL MULTIPLE 2x POSTS WITH 16d SINKERS @ 6" O.C.
10. SHADED AREAS ARE TYPICAL OVERFILL, STICK FRAMED PER DETAIL 6 / S1.1 OR OVERBUILD TRUSSES PER TRUSS MANUFACTURER
11. INTERIOR BEARING WALLS
12. ALL GLULAM BEAMS TO HAVE STANDARD CAMBER (R = 2000') U.N.O.
13. PROVIDE (2) 2x POST EACH END OF ALL BEAMS & GIRDER TRUSSES, U.N.O. PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WITH POSTS, CRIPPLES, AND SQUASH BLOCKS AS REQUIRED.
14. BEAM AND HEADER SIZES INDICATED ON THE PLANS ARE MINIMUM SIZES. LARGER SIZES MAY BE INSTALLED AT THE CONTRACTOR'S OPTION.
15. CONTINUOUS TOP PLATE MAY BE USED IN LIEU OF ST6224 STRAP FROM BEAM TO PLATE.
16. SEE ARCH'L FOR ROOF SLOPE AND TOP OF STEEL ELEVATIONS.
17. DASHED WALLS INDICATE WALLS ABOVE
18. AT NOTES FOR INVERTED HANGERS SEE DTL 22 / SD-3 FOR FULL CONNECTION INFORMATION

JOIST HANGER SCHEDULE

JOIST LENGTH	FACE MOUNTED HANGER		TOP FLANGE HANGER	
	FLOOR	ROOF	FLOOR	ROOF
CORRIDOR 2x6	LUS26	LUS26	JB	JB
STAIR LANDINGS	LUS28	-	JB	-
UP TO 16'-0"	IUS	HU	ITS	ITS
16'-0" TO 25'-0"	IUS	HU	ITS	MIT
25'-0" TO 32'-0"	HU	MIU	ITS	WP
32'-0" TO 40'-0"	HU	-	MIT	BA

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CONSULTANT:

OWNER:

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LEVEL 4 FRAMING PLAN

PROJECT #: U1477.005.221
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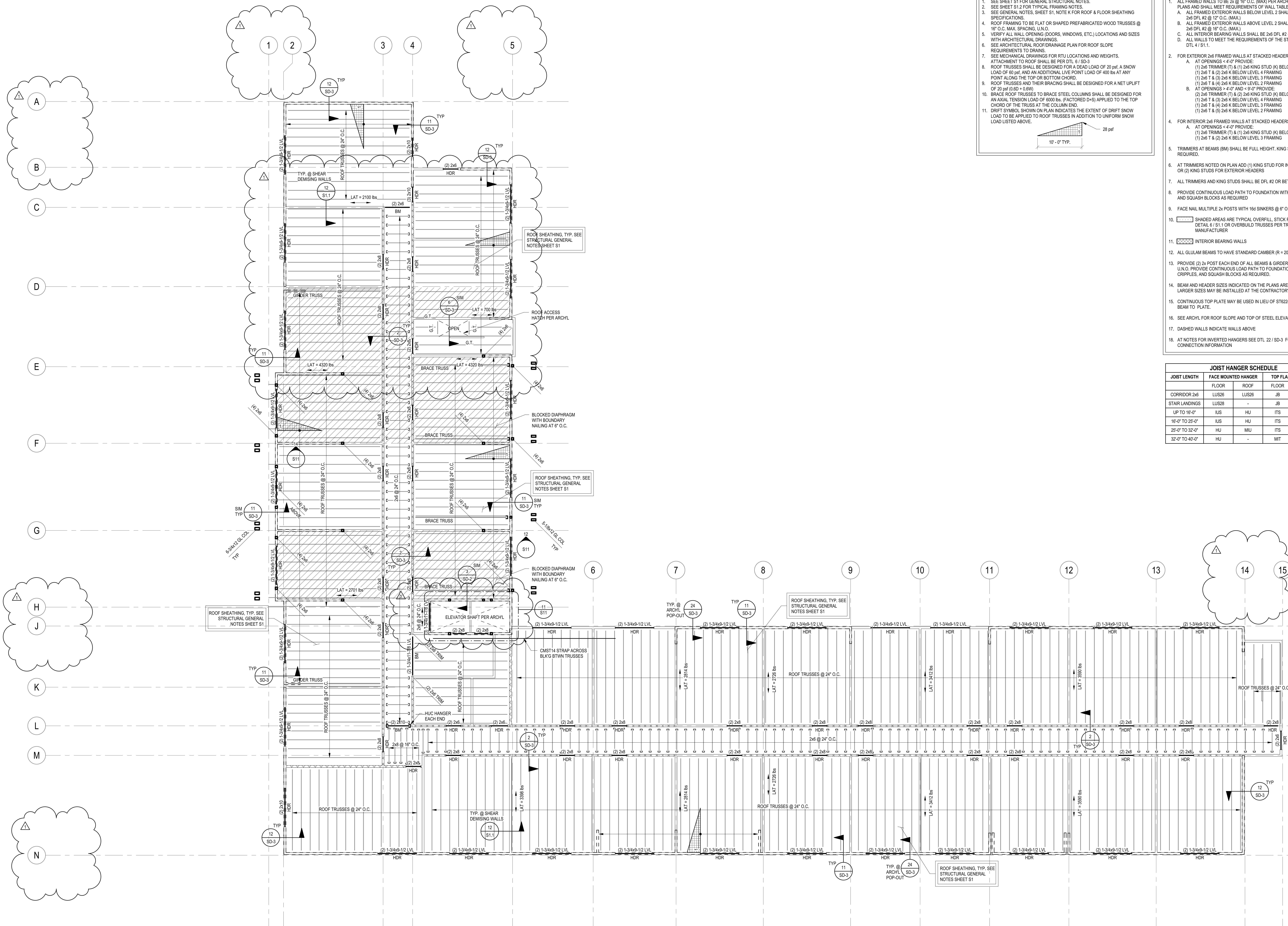
SHEET #
S5



ROOF FRAMING PLAN

1/8" = 1'-0"

ROOF SHEET NOTES:

1. SEE SHEET S1 FOR GENERAL STRUCTURAL NOTES.
2. SEE SHEET S1.2 FOR TYPICAL FRAMING NOTES.
3. SEE GENERAL NOTES, SHEET S1, NOTE K FOR ROOF & FLOOR SHEATHING SPECIFICATIONS.
4. ROOF FRAMING TO BE FLAT OR SHAPED PREFABRICATED WOOD TRUSSES @ 16" O.C. MAX. SPACING, U.N.O.
5. VERIFY ALL WALL OPENING (DOORS, WINDOWS, ETC.) LOCATIONS AND SIZES WITH ARCHITECTURAL DRAWINGS.
6. SEE ARCHITECTURAL ROOF/DRAINAGE PLAN FOR ROOF SLOPE REQUIREMENTS TO DRAINS.
7. SEE MECHANICAL DRAWINGS FOR RTU LOCATIONS AND WEIGHTS. ATTACHMENT TO ROOF SHALL BE PER DTL 6 / SD-3
8. ROOF TRUSSES SHALL BE DESIGNED FOR A DEAD LOAD OF 20 psf, A SNOW LOAD OF 60 psf, AND AN ADDITIONAL LIVE POINT LOAD OF 400 lbs AT ANY POINT ALONG THE TOP OR BOTTOM CHORD.
9. ROOF TRUSSES AND THEIR BRACING SHALL BE DESIGNED FOR A NET UPLIFT OF 20 psf (0.6D + 0.6W)
10. BRACE ROOF TRUSSES TO BRACE STEEL COLUMNS SHALL BE DESIGNED FOR AN AXIAL TENSION LOAD OF 6000 lbs. (FACTORED D+S) APPLIED TO THE TOP CHORD OF THE TRUSS AT THE COLUMN END.
11. DRIFT SYMBOL SHOWN ON PLAN INDICATES THE EXTENT OF DRIFT SNOW LOAD TO BE APPLIED TO ROOF TRUSSES IN ADDITION TO UNIFORM SNOW LOAD LISTED ABOVE.

1
28 psf
10' - 0" TYP.

FRAMING NOTES:

1. ALL FRAMED WALLS TO BE 2x @ 16" O.C. (MAX) PER ARCHITECTURAL PLANS AND SHALL MEET REQUIREMENTS OF WALL TABLE ON SHEET S1.1.
   A. ALL FRAMED EXTERIOR WALLS BELOW LEVEL 2 SHALL BE 2x6 DFL #2 @ 12" O.C. (MAX.)
   B. ALL FRAMED EXTERIOR WALLS ABOVE LEVEL 2 SHALL BE 2x6 DFL #2 @ 16" O.C. (MAX.)
   C. ALL INTERIOR BEARING WALLS SHALL BE 2x6 DFL #2 @ 16" O.C. (MAX.)
   D. ALL WALLS TO MEET THE REQUIREMENTS OF THE STUD TABLE ON DTL 4 / S1.1.

2. FOR EXTERIOR 2x6 FRAMED WALLS AT STACKED HEADERS (HDR):
   A. AT OPENINGS < 4'-0" PROVIDE:
      (1) 2x6 TRIMMER (T) & (1) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (2) 2x6 K BELOW LEVEL 4 FRAMING
      (1) 2x6 T & (3) 2x6 K BELOW LEVEL 3 FRAMING
      (1) 2x6 T & (4) 2x6 K BELOW LEVEL 2 FRAMING
   B. AT OPENINGS > 4'-0" AND < 9'-0" PROVIDE:
      (2) 2x6 TRIMMER (T) & (2) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (3) 2x6 K BELOW LEVEL 4 FRAMING
      (1) 2x6 T & (4) 2x6 K BELOW LEVEL 3 FRAMING
      (1) 2x6 T & (5) 2x6 K BELOW LEVEL 2 FRAMING

4. FOR INTERIOR 2x6 FRAMED WALLS AT STACKED HEADERS (HDR):
   A. AT OPENINGS < 4'-0" PROVIDE:
      (1) 2x6 TRIMMER (T) & (1) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (2) 2x6 K BELOW LEVEL 3 FRAMING

5. TRIMMERS AT BEAMS (BM) SHALL BE FULL HEIGHT. KING STUDS ARE NOT REQUIRED.

6. AT TRIMMERS NOTED ON PLAN ADD (1) KING STUD FOR INTERIOR HEADERS OR (2) KING STUDS FOR EXTERIOR HEADERS

7. ALL TRIMMERS AND KING STUDS SHALL BE DFL #2 OR BETTER

8. PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WITH POSTS, CRIPPLES, AND SQUASH BLOCKS AS REQUIRED

9. FACE NAIL MULTIPLE 2x POSTS WITH 16d SINKERS @ 6" O.C.

10. SHADED AREAS ARE TYPICAL OVERFILL, STICK FRAMED PER DETAIL 6 / S1.1 OR OVERBUILD TRUSSES PER TRUSS MANUFACTURER

11. INTERIOR BEARING WALLS

12. ALL GLULAM BEAMS TO HAVE STANDARD CAMBER (R = 2000') U.N.O.

13. PROVIDE (2) 2x POST EACH END OF ALL BEAMS & GIRDER TRUSSES, U.N.O. PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WITH POSTS, CRIPPLES, AND SQUASH BLOCKS AS REQUIRED.

14. BEAM AND HEADER SIZES INDICATED ON THE PLANS ARE MINIMUM SIZES. LARGER SIZES MAY BE INSTALLED AT THE CONTRACTOR'S OPTION.

15. CONTINUOUS TOP PLATE MAY BE USED IN LIEU OF ST6224 STRAP FROM BEAM TO PLATE.

16. SEE ARCH'L FOR ROOF SLOPE AND TOP OF STEEL ELEVATIONS.

17. DASHED WALLS INDICATE WALLS ABOVE

18. AT NOTES FOR INVERTED HANGERS SEE DTL 22 / SD-3 FOR FULL CONNECTION INFORMATION

JOIST HANGER SCHEDULE

JOIST LENGTH	FACE MOUNTED HANGER		TOP FLANGE HANGER	
	FLOOR	ROOF	FLOOR	ROOF
CORRIDOR 2x6	LUS26	LUS26	JB	JB
STAIR LANDINGS	LUS28	-	JB	-
UP TO 16'-0"	IUS	HU	ITS	ITS
16'-0" TO 25'-0"	IUS	HU	ITS	MIT
25'-0" TO 32'-0"	HU	MIU	ITS	WP
32'-0" TO 40'-0"	HU	-	MIT	BA

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CONSULTANT:

OWNER:

ELEMENT HOTEL

PROJECT:

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ROOF FRAMING PLAN

PROJECT #: U1477.005.221
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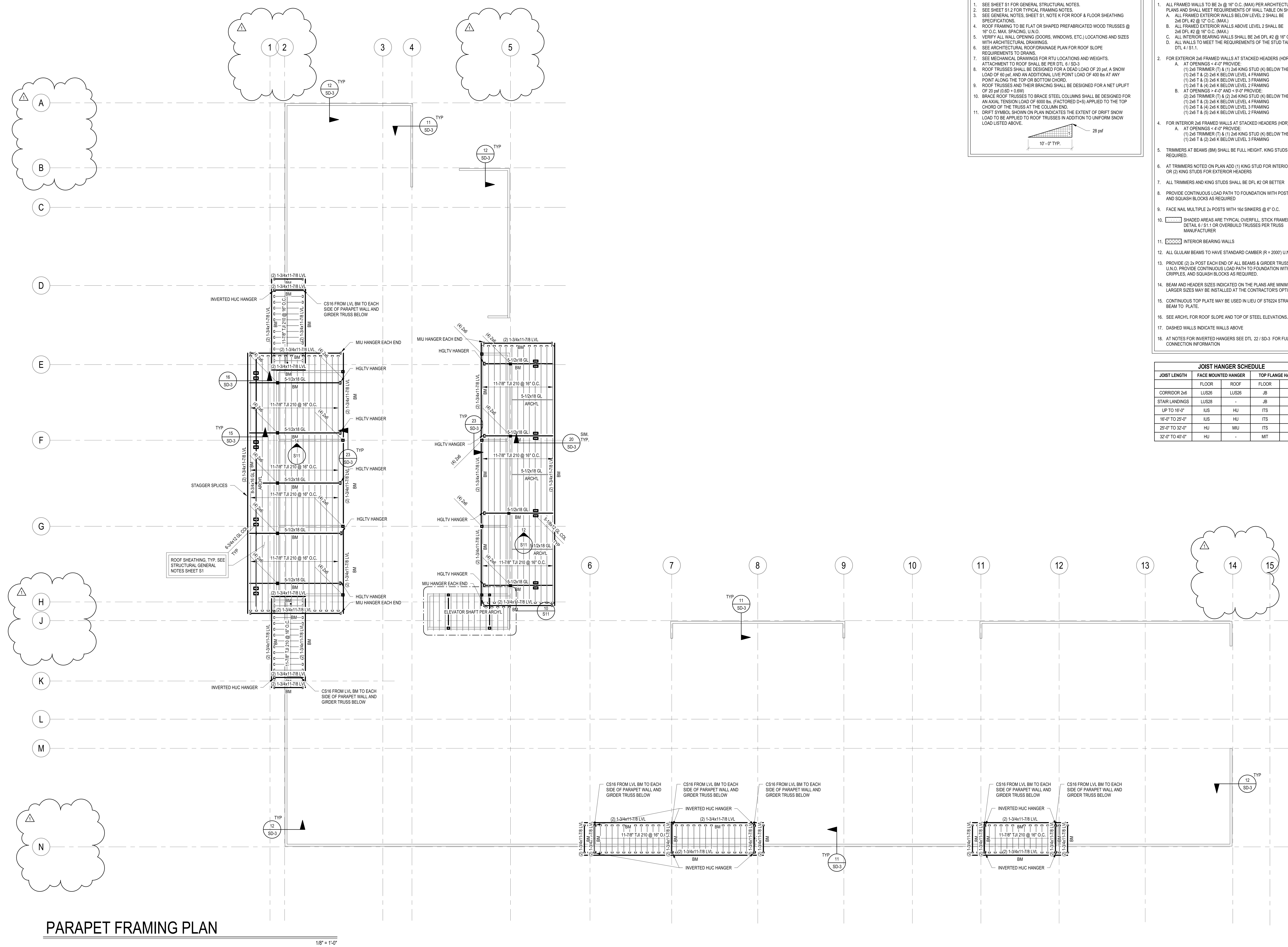
CHECKED BY: BSE

SHEET #
S6



ROOF SHEET NOTES:

1. SEE SHEET S1 FOR GENERAL STRUCTURAL NOTES.
2. SEE SHEET S1.2 FOR TYPICAL FRAMING NOTES.
3. SEE GENERAL NOTES, SHEET S1, NOTE K FOR ROOF & FLOOR SHEATHING SPECIFICATIONS.
4. ROOF FRAMING TO BE FLAT OR SHAPED PREFABRICATED WOOD TRUSSES @ 16" O.C. MAX. SPACING, U.N.O.
5. VERIFY ALL WALL OPENING (DOORS, WINDOWS, ETC.) LOCATIONS AND SIZES WITH ARCHITECTURAL DRAWINGS.
6. SEE ARCHITECTURAL ROOF/DRAINAGE PLAN FOR ROOF SLOPE REQUIREMENTS TO DRAINS.
7. SEE MECHANICAL DRAWINGS FOR RTU LOCATIONS AND WEIGHTS. ATTACHMENT TO ROOF SHALL BE PER DTL 6 / SD-3
8. ROOF TRUSSES SHALL BE DESIGNED FOR A DEAD LOAD OF 20 psf, A SNOW LOAD OF 60 psf, AND AN ADDITIONAL LIVE POINT LOAD OF 400 lbs AT ANY POINT ALONG THE TOP OR BOTTOM CHORD.
9. ROOF TRUSSES AND THEIR BRACING SHALL BE DESIGNED FOR A NET UPLIFT OF 20 psf (0.6D + 0.6W)
10. BRACE ROOF TRUSSES TO BRACE STEEL COLUMNS SHALL BE DESIGNED FOR AN AXIAL TENSION LOAD OF 6000 lbs. (FACTORED D+S) APPLIED TO THE TOP CHORD OF THE TRUSS AT THE COLUMN END.
11. DRIFT SYMBOL SHOWN ON PLAN INDICATES THE EXTENT OF DRIFT SNOW LOAD TO BE APPLIED TO ROOF TRUSSES IN ADDITION TO UNIFORM SNOW LOAD LISTED ABOVE.

1
28 psf
10' - 0" TYP.

FRAMING NOTES:

1. ALL FRAMED WALLS TO BE 2x @ 16" O.C. (MAX) PER ARCHITECTURAL PLANS AND SHALL MEET REQUIREMENTS OF WALL TABLE ON SHEET S1.1.
   A. ALL FRAMED EXTERIOR WALLS BELOW LEVEL 2 SHALL BE 2x6 DFL #2 @ 12" O.C. (MAX.)
   B. ALL FRAMED EXTERIOR WALLS ABOVE LEVEL 2 SHALL BE 2x6 DFL #2 @ 16" O.C. (MAX.)
   C. ALL INTERIOR BEARING WALLS SHALL BE 2x6 DFL #2 @ 16" O.C. (MAX.)
   D. ALL WALLS TO MEET THE REQUIREMENTS OF THE STUD TABLE ON DTL 4 / S1.1.

2. FOR EXTERIOR 2x6 FRAMED WALLS AT STACKED HEADERS (HDR):
   A. AT OPENINGS < 4'-0" PROVIDE:
      (1) 2x6 TRIMMER (T) & (1) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (2) 2x6 K BELOW LEVEL 4 FRAMING
      (1) 2x6 T & (3) 2x6 K BELOW LEVEL 3 FRAMING
      (1) 2x6 T & (4) 2x6 K BELOW LEVEL 2 FRAMING
   B. AT OPENINGS > 4'-0" AND < 9'-0" PROVIDE:
      (2) 2x6 TRIMMER (T) & (2) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (3) 2x6 K BELOW LEVEL 4 FRAMING
      (1) 2x6 T & (4) 2x6 K BELOW LEVEL 3 FRAMING
      (1) 2x6 T & (5) 2x6 K BELOW LEVEL 2 FRAMING

4. FOR INTERIOR 2x6 FRAMED WALLS AT STACKED HEADERS (HDR):
   A. AT OPENINGS < 4'-0" PROVIDE:
      (1) 2x6 TRIMMER (T) & (1) 2x6 KING STUD (K) BELOW THE ROOF
      (1) 2x6 T & (2) 2x6 K BELOW LEVEL 3 FRAMING

5. TRIMMERS AT BEAMS (BM) SHALL BE FULL HEIGHT. KING STUDS ARE NOT REQUIRED.

6. AT TRIMMERS NOTED ON PLAN ADD (1) KING STUD FOR INTERIOR HEADERS OR (2) KING STUDS FOR EXTERIOR HEADERS

7. ALL TRIMMERS AND KING STUDS SHALL BE DFL #2 OR BETTER

8. PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WITH POSTS, CRIPPLES, AND SQUASH BLOCKS AS REQUIRED

9. FACE NAIL MULTIPLE 2x POSTS WITH 16d SINKERS @ 6" O.C.

10. SHADED AREAS ARE TYPICAL OVERFILL, STICK FRAMED PER DETAIL 6 / S1.1 OR OVERBUILD TRUSSES PER TRUSS MANUFACTURER

11. INTERIOR BEARING WALLS

12. ALL GLULAM BEAMS TO HAVE STANDARD CAMBER (R = 2000') U.N.O.

13. PROVIDE (2) 2x POST EACH END OF ALL BEAMS & GIRDER TRUSSES, U.N.O. PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WITH POSTS, CRIPPLES, AND SQUASH BLOCKS AS REQUIRED.

14. BEAM AND HEADER SIZES INDICATED ON THE PLANS ARE MINIMUM SIZES. LARGER SIZES MAY BE INSTALLED AT THE CONTRACTOR'S OPTION.

15. CONTINUOUS TOP PLATE MAY BE USED IN LIEU OF ST6224 STRAP FROM BEAM TO PLATE.

16. SEE ARCH'L FOR ROOF SLOPE AND TOP OF STEEL ELEVATIONS.

17. DASHED WALLS INDICATE WALLS ABOVE

18. AT NOTES FOR INVERTED HANGERS SEE DTL 22 / SD-3 FOR FULL CONNECTION INFORMATION

JOIST HANGER SCHEDULE

JOIST LENGTH	FACE MOUNTED HANGER		TOP FLANGE HANGER	
	FLOOR	ROOF	FLOOR	ROOF
CORRIDOR 2x6	LUS26	LUS26	JB	JB
STAIR LANDINGS	LUS28	-	JB	-
UP TO 16'-0"	IUS	HU	ITS	ITS
16'-0" TO 25'-0"	IUS	HU	ITS	MIT
25'-0" TO 32'-0"	HU	MIU	ITS	WP
32'-0" TO 40'-0"	HU	-	MIT	BA

VECTOR ENGINEERS

CONSULTANT:

OWNER:

ELEMENT HOTEL

PROJECT:

PROPRIETARY DO NOT DISTRIBUTE

#	ISSUE DATE	DESCRIPTION
#	REV. DATE	DESCRIPTION

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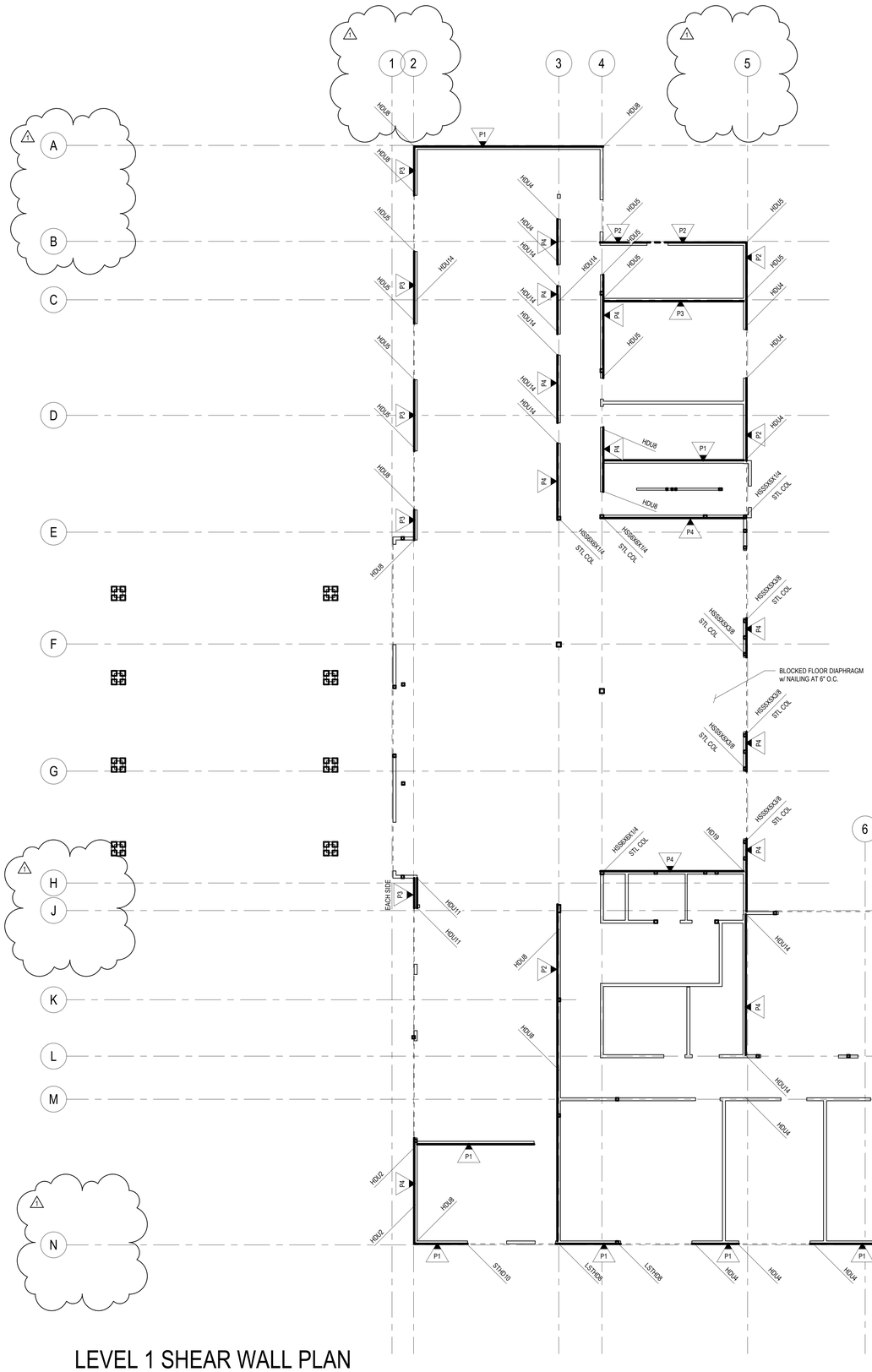
HIGH ROOF FRAMING PLAN

PROJECT #: U1477.005.221
DRAWN BY: SDC
CHECKED BY: BSE

SHEET #
S6.1

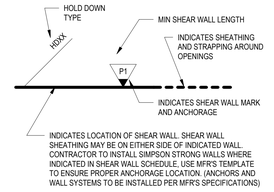


LEVEL 1 SHEAR WALL PLAN

1/8" = 1'-0"

SHEAR WALL NOTES:

1. ALL SHEAR WALLS TO BE ATTACHED TO THE FOUNDATION OR PODIUM SLAB PER SHEAR WALL SCHEDULE w/ 7" MIN EMBEDMENT U.N.O.
2. ALL HOLDOWNS SHALL BE INSTALLED AS SHOWN ON DETAIL (1 / SD-1)
3. FOR SILL ANCHORAGE SEE SHEAR WALL SCHEDULE ON SHEET (2 / S1.1)
4. ISOLATED FOOTINGS & INTERIOR STRIP FOOTINGS TO BE CENTERED BELOW POSTS & BEARING/SHEAR WALLS, RESPECTIVELY.
5. MASA MUDSILL ANCHORS MAY BE USED IN PLACE OF ANCHOR BOLTS, INSTALLED AT THE SAME SPACING INDICATED FOR SHEAR WALL ANCHORS.
6. CONTRACTOR TO USE MANUFACTURERS TEMPLATE FOR PLACEMENT OF HOLD DOWNS.
7. FOR SHEAR WALL SCHEDULE & ANCHORAGE HOLDOWN SCHEDULE SEE SHEET (SD-1)
8. WHERE STRAP HOLDOWNS ARE ATTACHED TO A SINGLE KING STUD & A SINGLE TRIMMER, ATTACH THE TWO TOGETHER w/ (2) 16d SINKERS @ 6" O.C. FULL HEIGHT OR w/ LTP4 @ 12" O.C. FULL HEIGHT.



SHEAR WALL KEY

VECTOR ENGINEERS
BOISE, IDAHO
(208) 906-0013
WWW.VECTORSE.COM
ID FIRM LICENSE NUMBER
1049PO
CONSULTANT:

OWNER:

ELEMENT HOTEL

PROJECT:

ISSUE DATE	DESCRIPTION

#	REV. DATE	DESCRIPTION
1		

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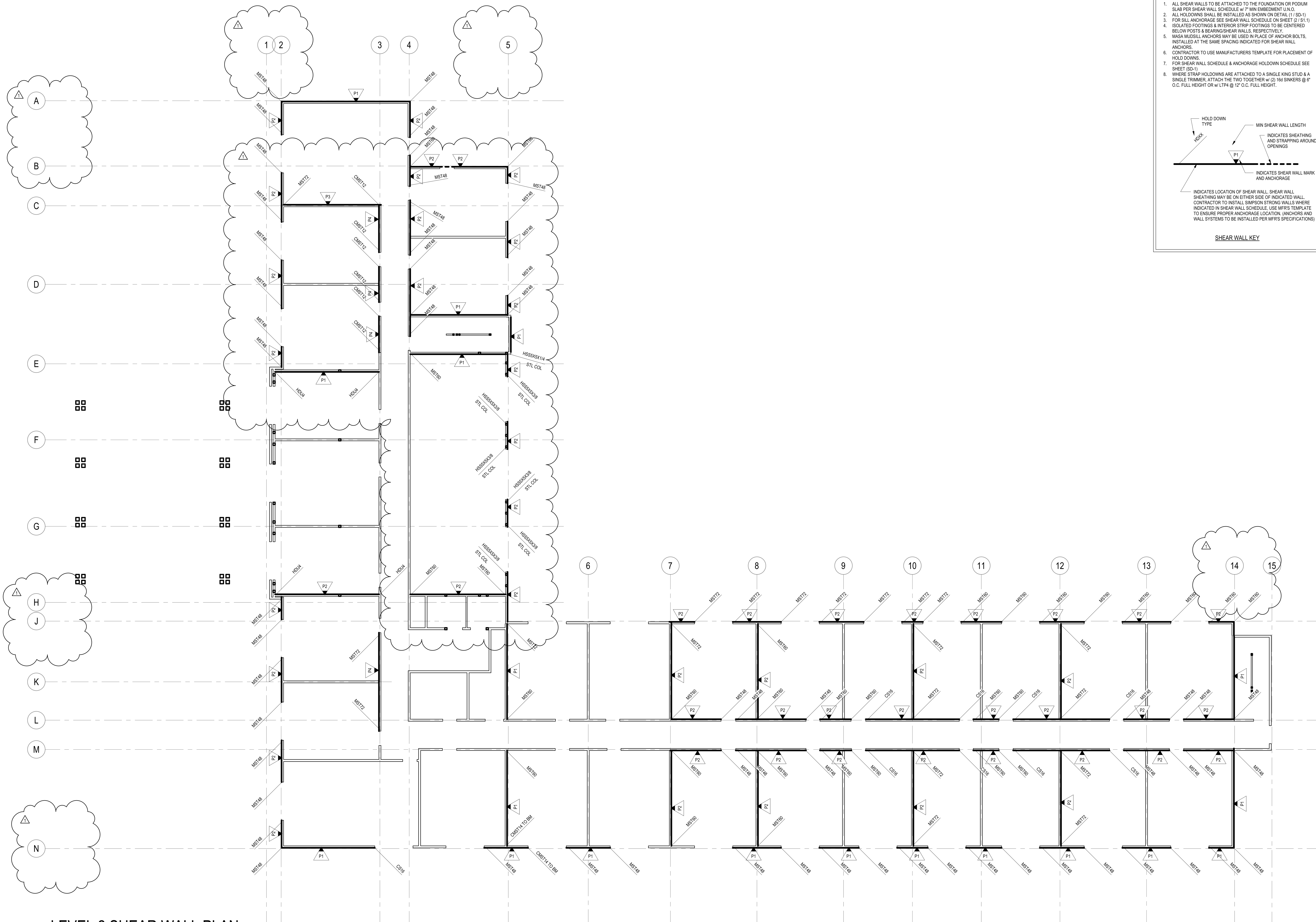
LEVEL 1 SHEAR WALL PLAN

PROJECT #: 1/07/2021
DRAWN BY: SC
CHECKED BY: BE

SHEET #
S7



LEVEL 2 SHEAR WALL PLAN

1/8" = 1'-0"

SHEAR WALL NOTES:

1. ALL SHEAR WALLS TO BE ATTACHED TO THE FOUNDATION OR PODIUM SLAB PER SHEAR WALL SCHEDULE w/ 7" MIN EMBEDMENT U.N.O.
2. ALL HOLDOWNS SHALL BE INSTALLED AS SHOWN ON DETAIL (1 / SD-1)
3. FOR SILL ANCHORAGE SEE SHEAR WALL SCHEDULE ON SHEET (2 / S1.1)
4. ISOLATED FOOTINGS & INTERIOR STRIP FOOTINGS TO BE CENTERED BELOW POSTS & BEARING/SHEAR WALLS, RESPECTIVELY.
5. MASA MUDSILL ANCHORS MAY BE USED IN PLACE OF ANCHOR BOLTS, INSTALLED AT THE SAME SPACING INDICATED FOR SHEAR WALL ANCHORS.
6. CONTRACTOR TO USE MANUFACTURERS TEMPLATE FOR PLACEMENT OF HOLD DOWNS.
7. FOR SHEAR WALL SCHEDULE & ANCHORAGE HOLDOWN SCHEDULE SEE SHEET (SD-1)
8. WHERE STRAP HOLDOWNS ARE ATTACHED TO A SINGLE KING STUD & A SINGLE TRIMMER, ATTACH THE TWO TOGETHER w/ (2) 16d SINKERS @ 6" O.C. FULL HEIGHT OR w/ LTP4 @ 12" O.C. FULL HEIGHT.

SHEAR WALL KEY

VECTOR ENGINEERS
BOISE, IDAHO
(208) 996-0303
WWW.VECTORSE.COM
ID FIRM LICENSE NUMBER 10449-O

CONSULTANT:

OWNER:

ELEMENT HOTEL

PROJECT:

ISSUE DATE	DESCRIPTION

#	REV. DATE	DESCRIPTION
1		

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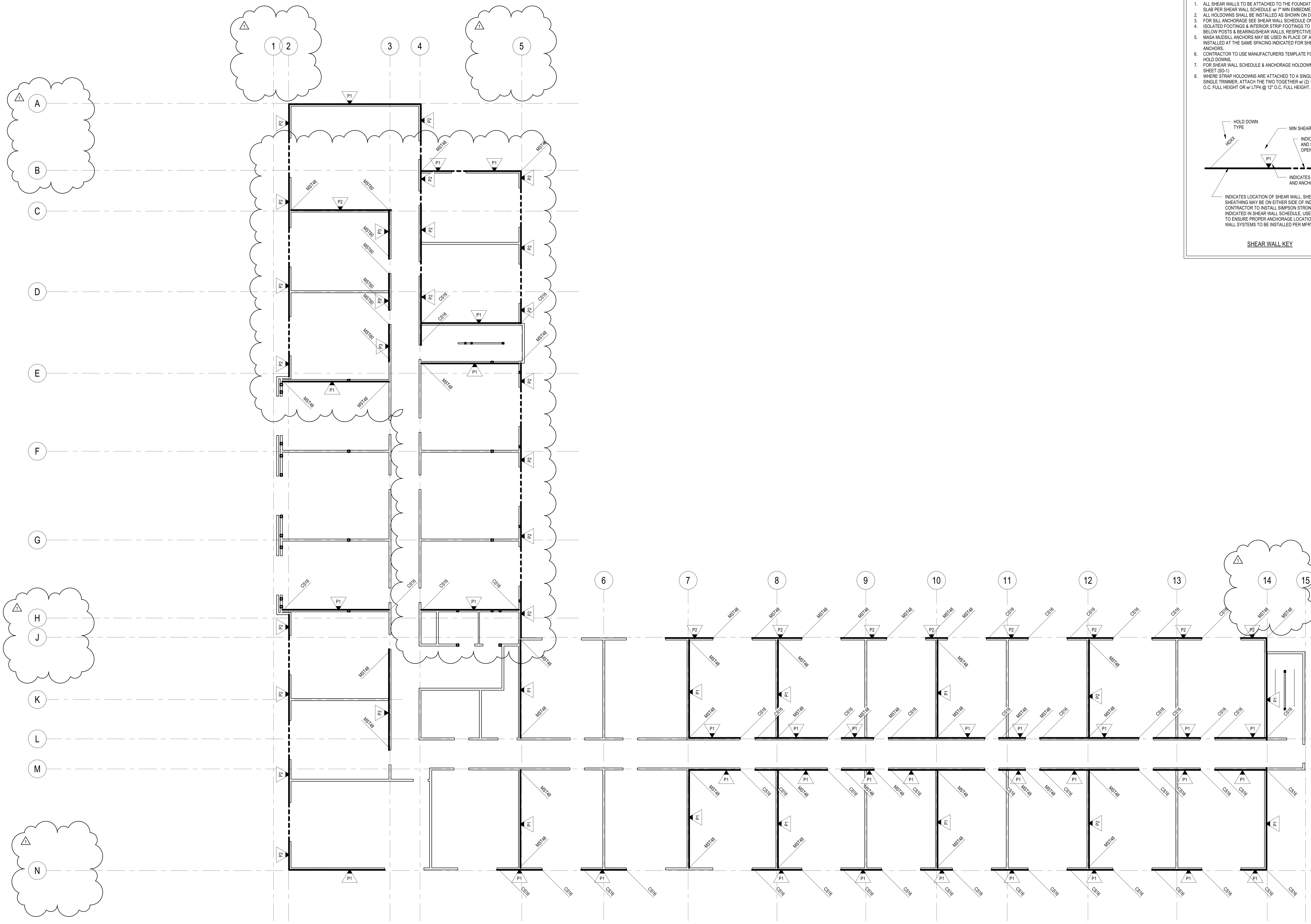
LEVEL 2 SHEAR WALL PLAN

PROJECT #: U1477.005.221
DRAWN BY: SDC
CHECKED BY: BSE

SHEET #
S8



LEVEL 3 SHEAR WALL PLAN

1/8" = 1'-0"

SHEAR WALL NOTES:

1. ALL SHEAR WALLS TO BE ATTACHED TO THE FOUNDATION OR PODIUM SLAB PER SHEAR WALL SCHEDULE w/ 7" MIN EMBEDMENT U.N.O.
2. ALL HOLDOWNS SHALL BE INSTALLED AS SHOWN ON DETAIL (1 / SD-1)
3. FOR SILL ANCHORAGE SEE SHEAR WALL SCHEDULE ON SHEET (2 / S1.1)
4. ISOLATED FOOTINGS & INTERIOR STRIP FOOTINGS TO BE CENTERED BELOW POSTS & BEARING/SHEAR WALLS, RESPECTIVELY.
5. MASA MUDSILL ANCHORS MAY BE USED IN PLACE OF ANCHOR BOLTS, INSTALLED AT THE SAME SPACING INDICATED FOR SHEAR WALL ANCHORS.
6. CONTRACTOR TO USE MANUFACTURERS TEMPLATE FOR PLACEMENT OF HOLD DOWNS.
7. FOR SHEAR WALL SCHEDULE & ANCHORAGE HOLDOWN SCHEDULE SEE SHEET (SD-1)
8. WHERE STRAP HOLDOWNS ARE ATTACHED TO A SINGLE KING STUD & A SINGLE TRIMMER, ATTACH THE TWO TOGETHER w/ (2) 16d SINKERS @ 6" O.C. FULL HEIGHT OR w/ LTP4 @ 12" O.C. FULL HEIGHT.

HOLD DOWN TYPE
HDXX
MIN SHEAR WALL LENGTH
INDICATES SHEATHING AND STRAPPING AROUND OPENINGS
P1
INDICATES SHEAR WALL MARK AND ANCHORAGE
INDICATES LOCATION OF SHEAR WALL. SHEAR WALL SHEATHING MAY BE ON EITHER SIDE OF INDICATED WALL. CONTRACTOR TO INSTALL SIMPSON STRONG WALLS WHERE INDICATED IN SHEAR WALL SCHEDULE, USE MFR'S TEMPLATE TO ENSURE PROPER ANCHORAGE LOCATION. (ANCHORS AND WALL SYSTEMS TO BE INSTALLED PER MFR'S SPECIFICATIONS)

SHEAR WALL KEY

VECTOR ENGINEERS
Boise, Idaho
(208) 996-0303
WWW.VECTORSE.COM
ID FIRM LICENSE NUMBER 10449-0

CONSULTANT:

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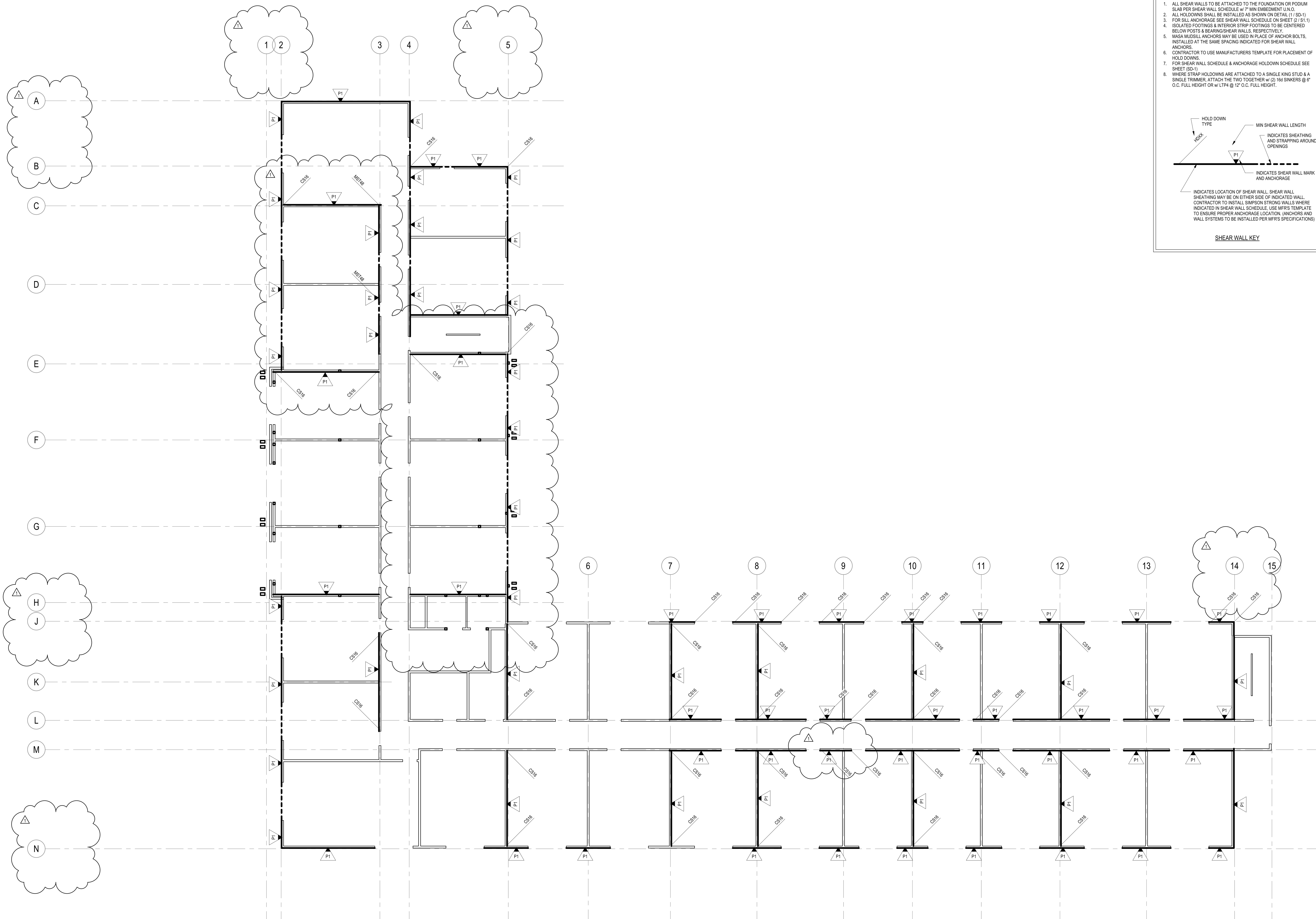
LEVEL 3 SHEAR WALL PLAN

PROJECT #: U1477.005.221
DRAWN BY: SDC
CHECKED BY: BSE

SHEET #
S9



LEVEL 4 SHEAR WALL PLAN

1/8" = 1'-0"

SHEAR WALL NOTES:

1. ALL SHEAR WALLS TO BE ATTACHED TO THE FOUNDATION OR PODIUM SLAB PER SHEAR WALL SCHEDULE w/ 7" MIN EMBEDMENT U.N.O.
2. ALL HOLDOWNS SHALL BE INSTALLED AS SHOWN ON DETAIL (1 / SD-1)
3. FOR SILL ANCHORAGE SEE SHEAR WALL SCHEDULE ON SHEET (2 / S1.1)
4. ISOLATED FOOTINGS & INTERIOR STRIP FOOTINGS TO BE CENTERED BELOW POSTS & BEARING/SHEAR WALLS, RESPECTIVELY.
5. MASA MUDSILL ANCHORS MAY BE USED IN PLACE OF ANCHOR BOLTS, INSTALLED AT THE SAME SPACING INDICATED FOR SHEAR WALL ANCHORS.
6. CONTRACTOR TO USE MANUFACTURERS TEMPLATE FOR PLACEMENT OF HOLD DOWNS.
7. FOR SHEAR WALL SCHEDULE & ANCHORAGE HOLDOWN SCHEDULE SEE SHEET (SD-1)
8. WHERE STRAP HOLDOWNS ARE ATTACHED TO A SINGLE KING STUD & A SINGLE TRIMMER, ATTACH THE TWO TOGETHER w/ (2) 16d SINKERS @ 6" O.C. FULL HEIGHT OR w/ LTP4 @ 12" O.C. FULL HEIGHT.

HOLD DOWN TYPE
HDXX
MIN SHEAR WALL LENGTH
INDICATES SHEATHING AND STRAPPING AROUND OPENINGS
P1
INDICATES SHEAR WALL MARK AND ANCHORAGE
INDICATES LOCATION OF SHEAR WALL. SHEAR WALL SHEATHING MAY BE ON EITHER SIDE OF INDICATED WALL. CONTRACTOR TO INSTALL SIMPSON STRONG WALLS WHERE INDICATED IN SHEAR WALL SCHEDULE, USE MFR'S TEMPLATE TO ENSURE PROPER ANCHORAGE LOCATION. (ANCHORS AND WALL SYSTEMS TO BE INSTALLED PER MFR'S SPECIFICATIONS)

SHEAR WALL KEY

VECTOR ENGINEERS
BOISE, IDAHO
(208) 996-0303
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LEVEL 4 SHEAR WALL PLAN

PROJECT #: U1477.005.221
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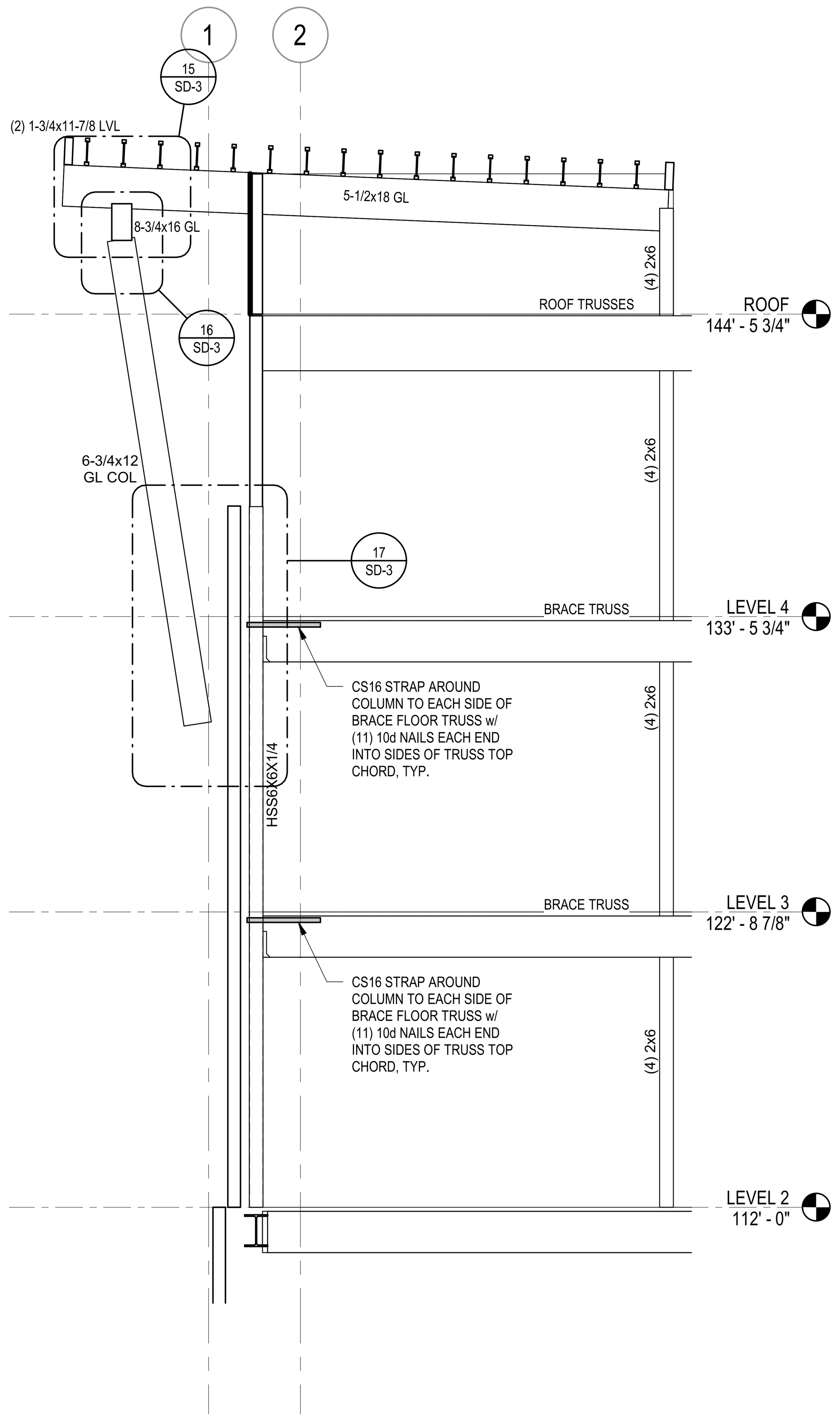
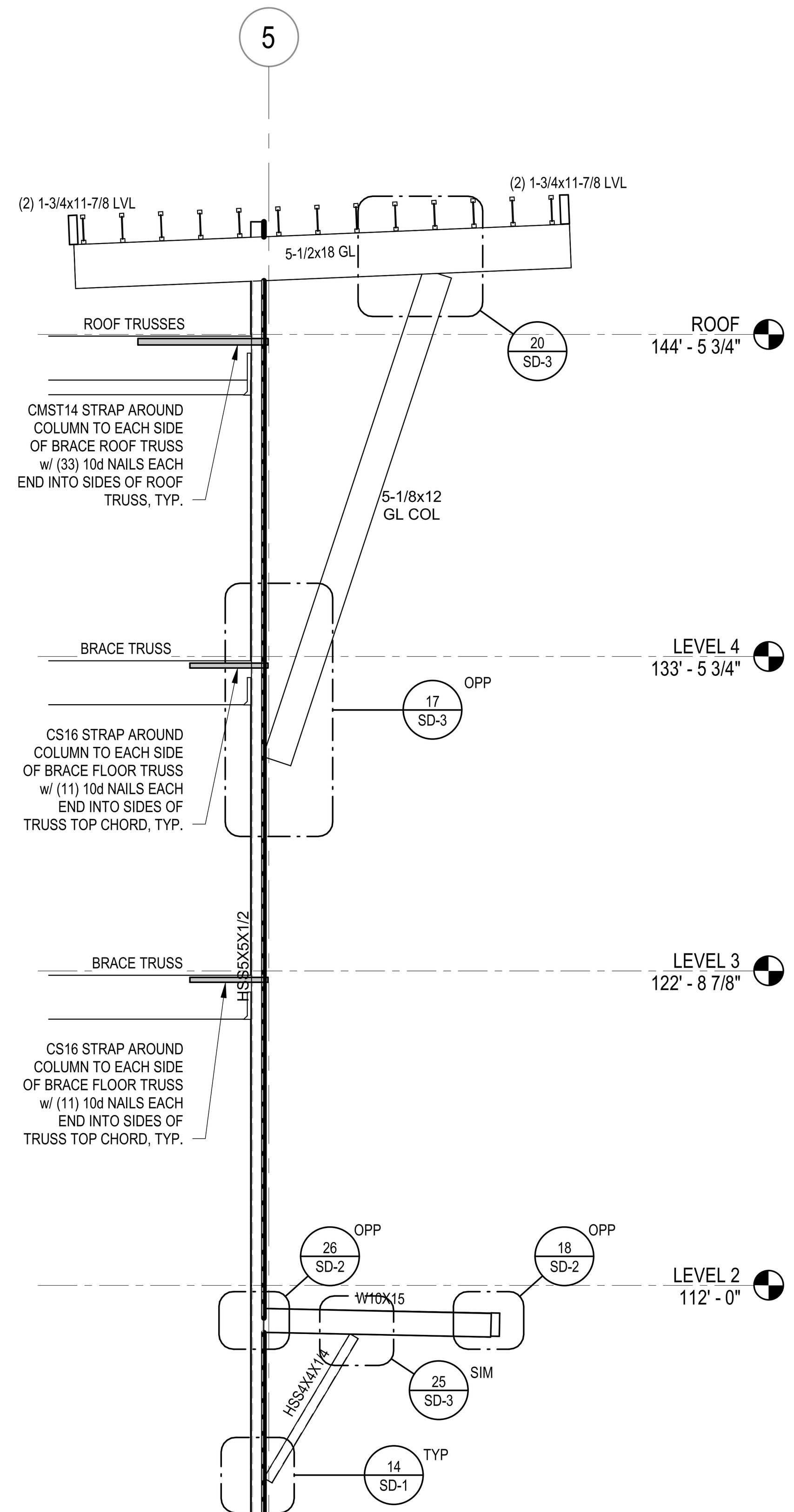
CHECKED BY: BSE

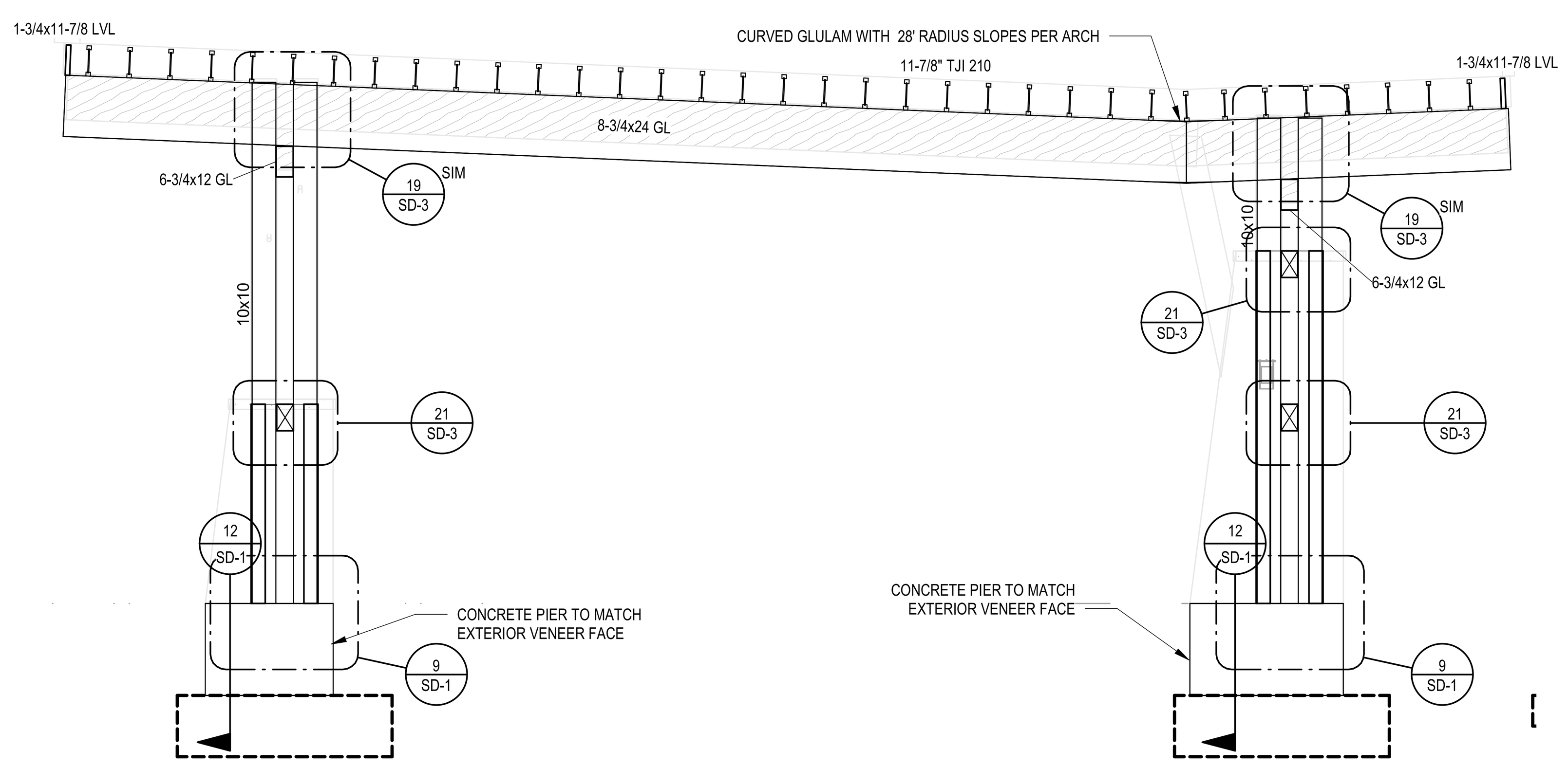
SHEET #
S10



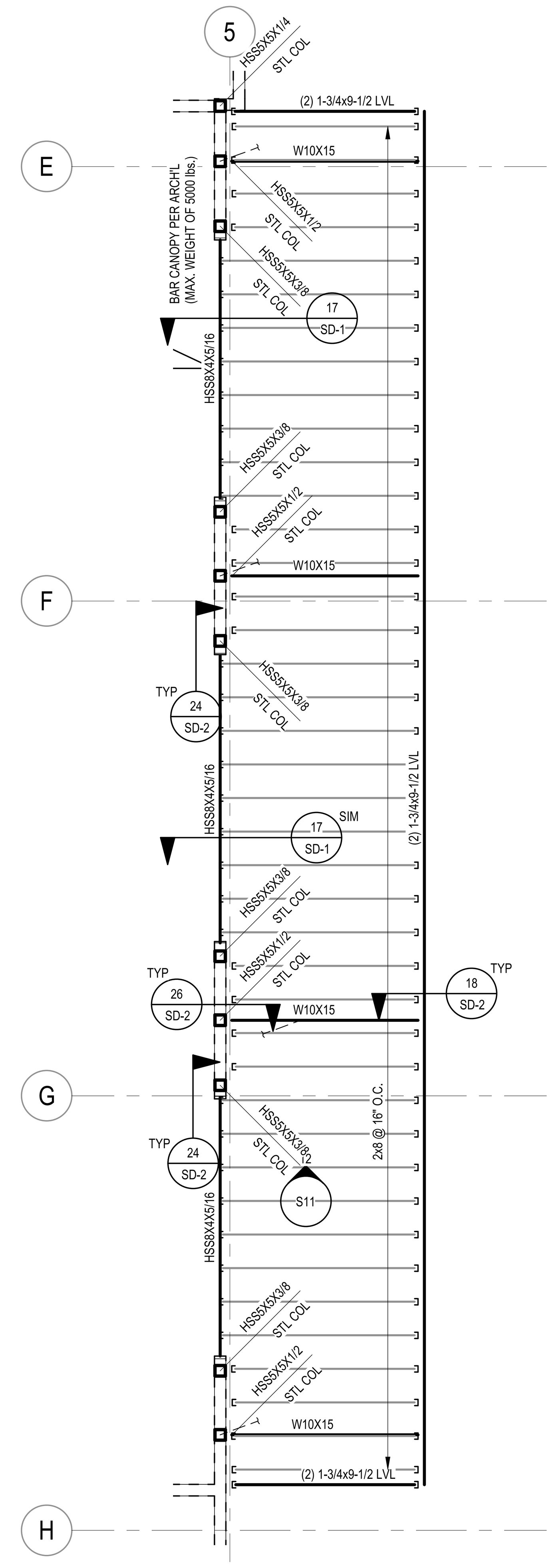
FRONT HIGH CANOPY FRAMING ELEVATION (14)
1/4" = 1'-0"



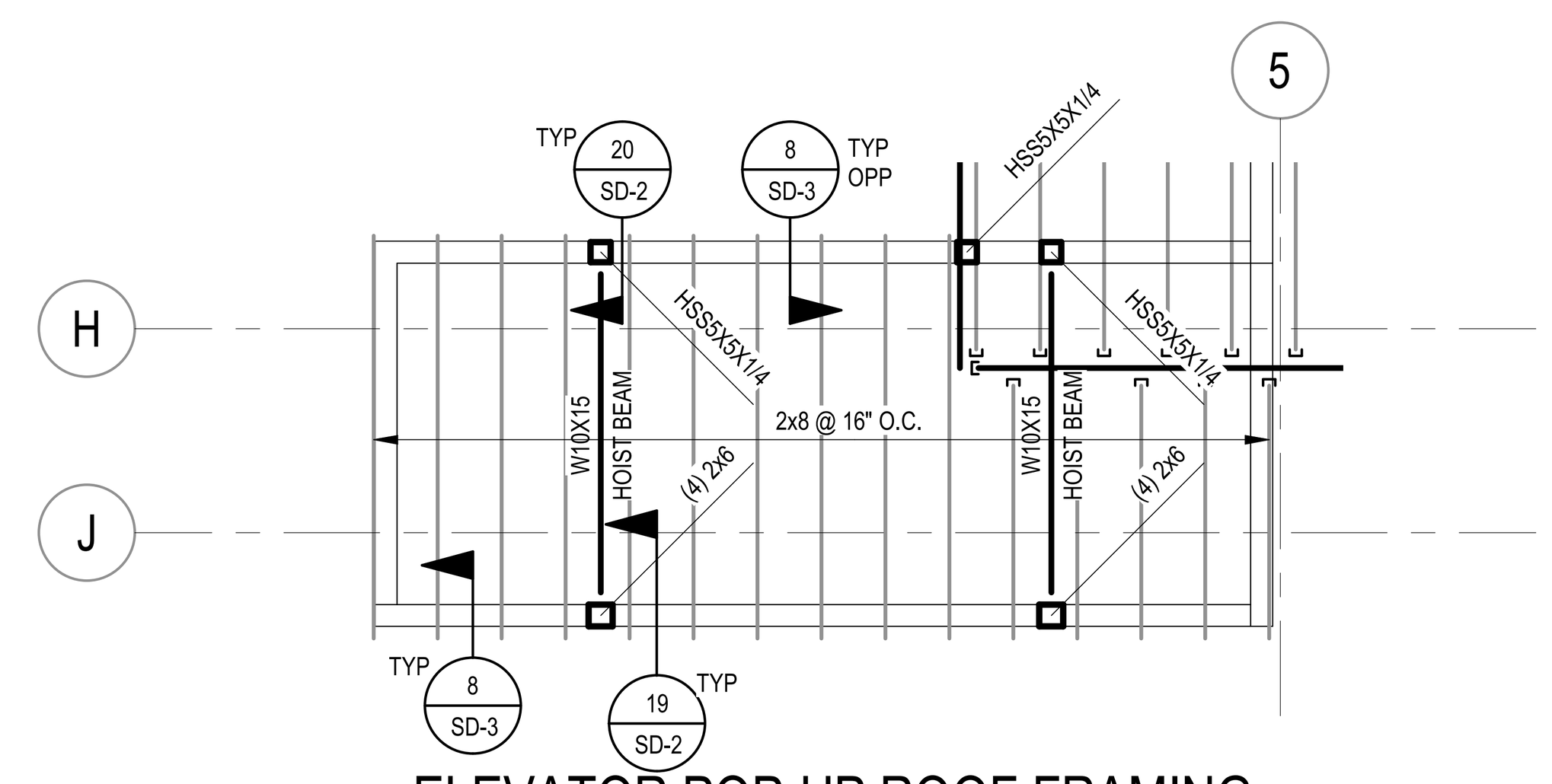
BACK HIGH CANOPY FRAMING ELEVATION (12)
1/4" = 1'-0"



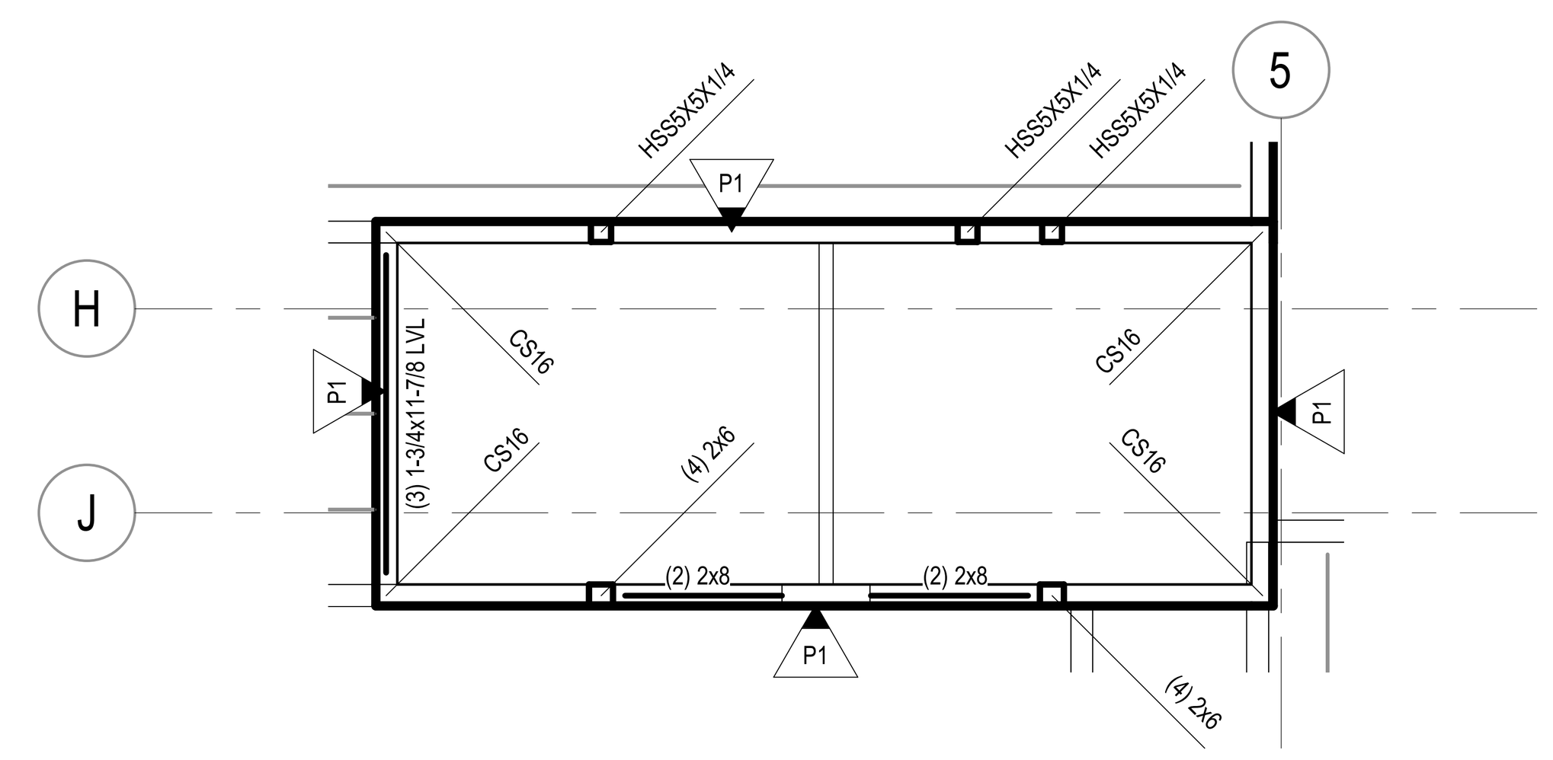
PORTE COCHERE TYPICAL FRAMING (13)
1/4" = 1'-0"



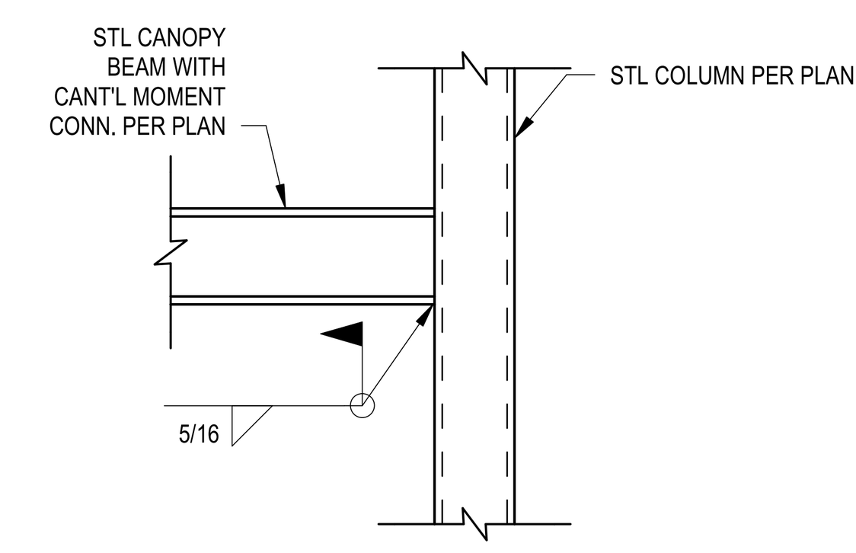
BACK LOW CANOPY FRAMING (9)
1/4" = 1'-0"



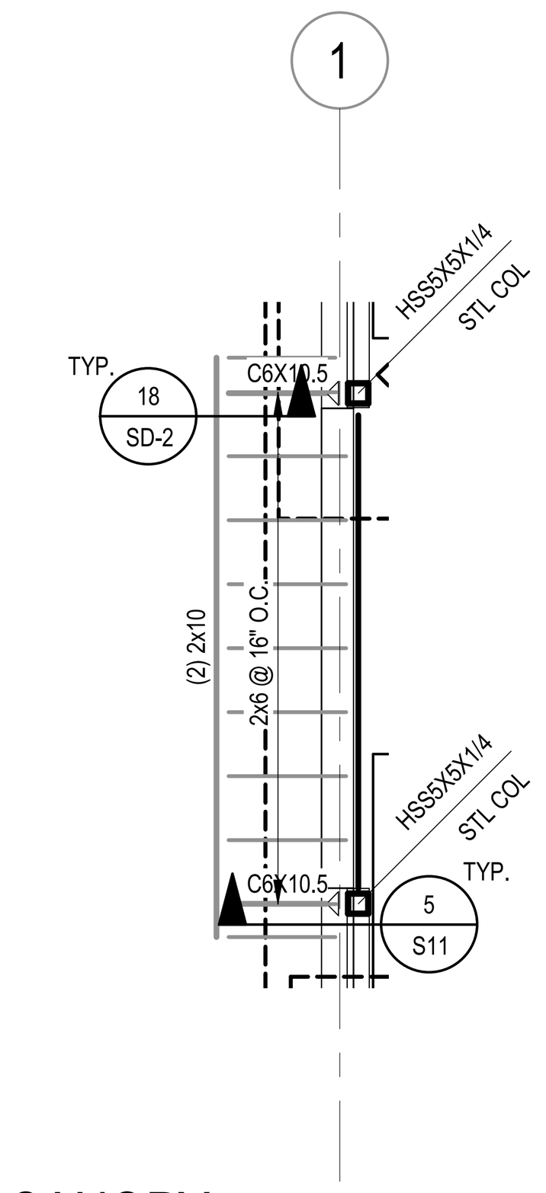
ELEVATOR POP-UP ROOF FRAMING (10)
1/4" = 1'-0"



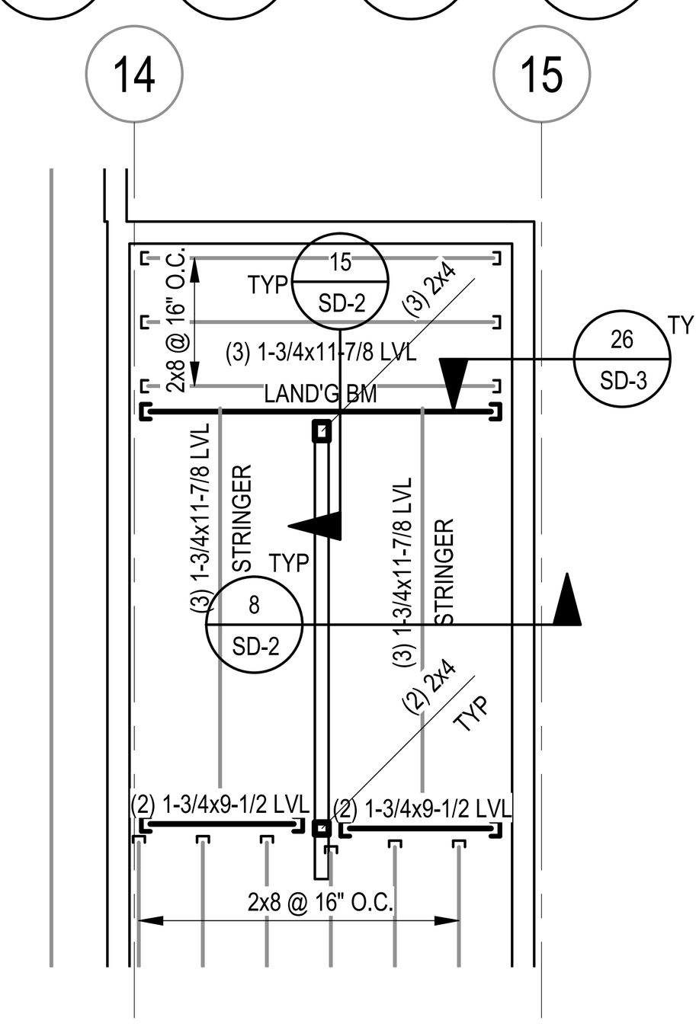
ELEVATOR FRAMING AT ROOF (11)
1/4" = 1'-0"



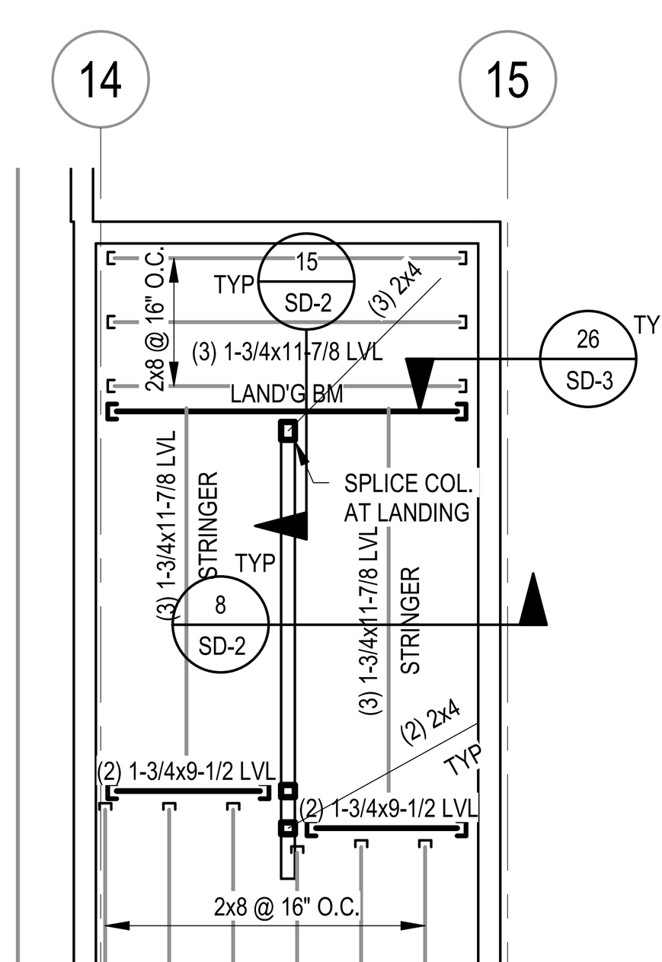
DETAIL (5)
N.T.S.



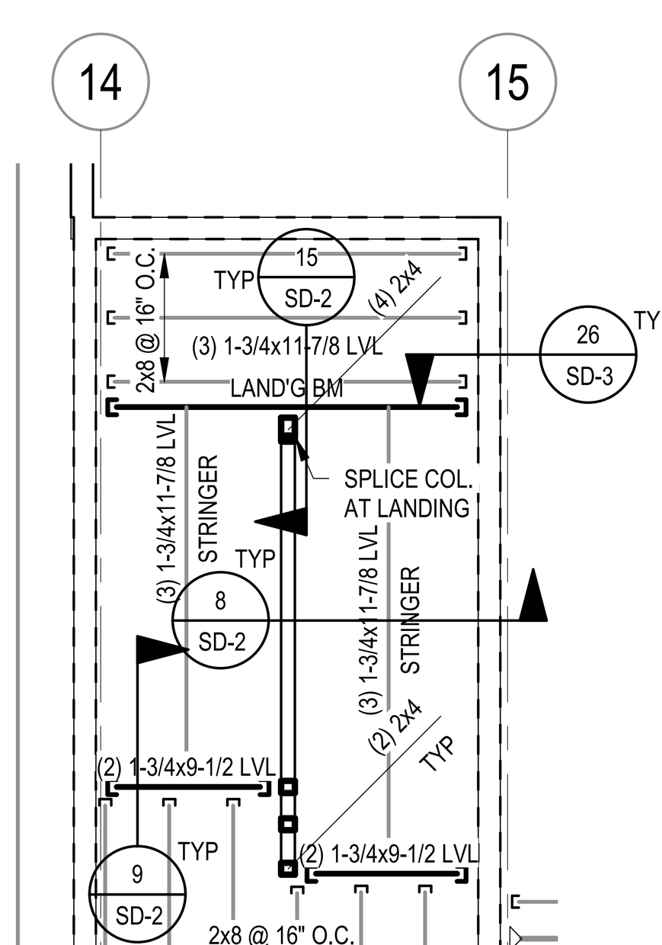
ENTRY CANOPY (1)
1/4" = 1'-0"



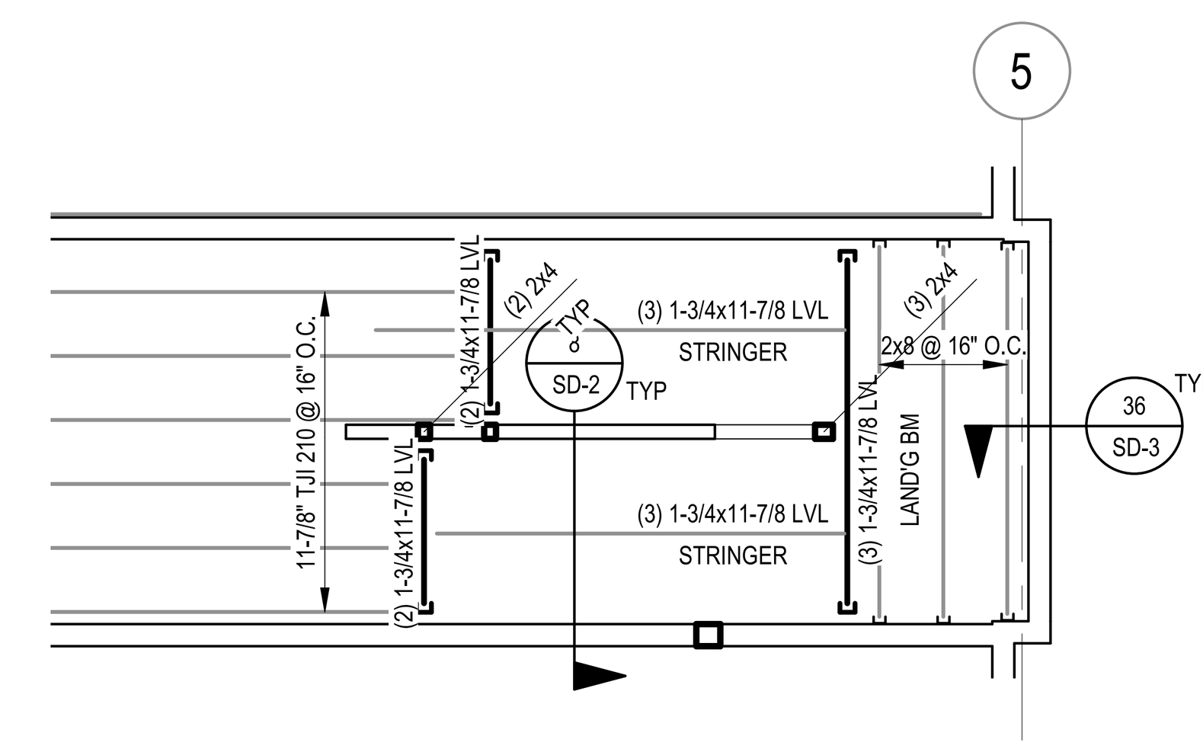
NORTH STAIRS LEVEL 4 (6)
1/4" = 1'-0"



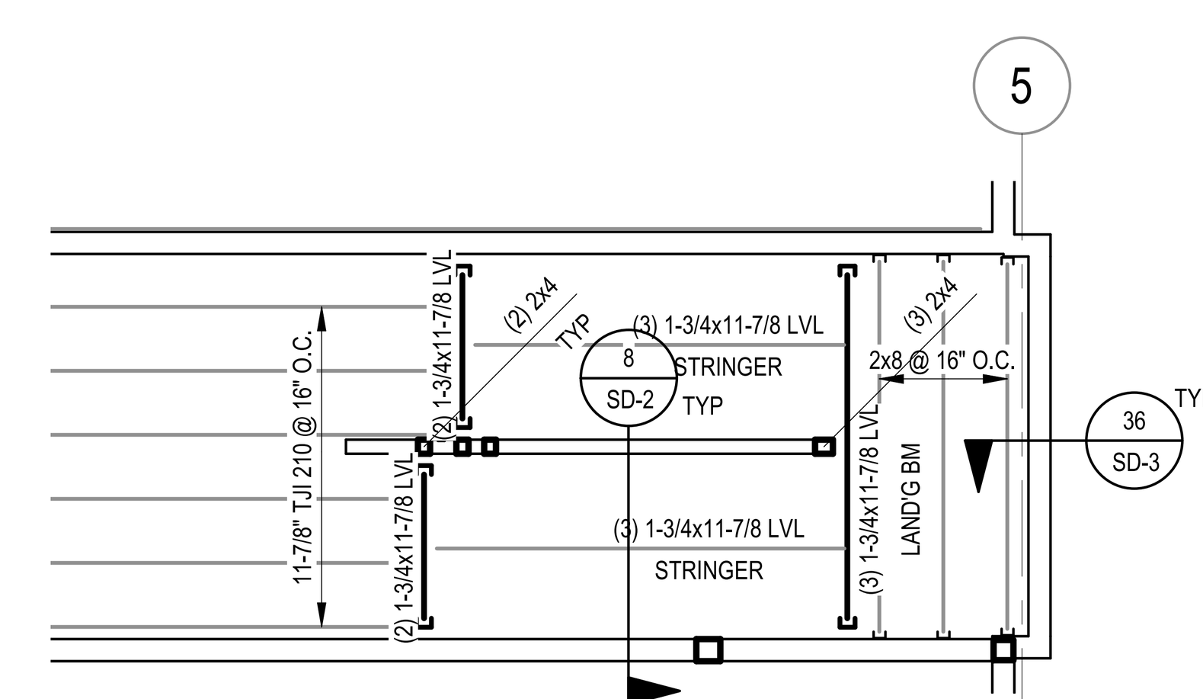
NORTH STAIRS LEVEL 3 (7)
1/4" = 1'-0"



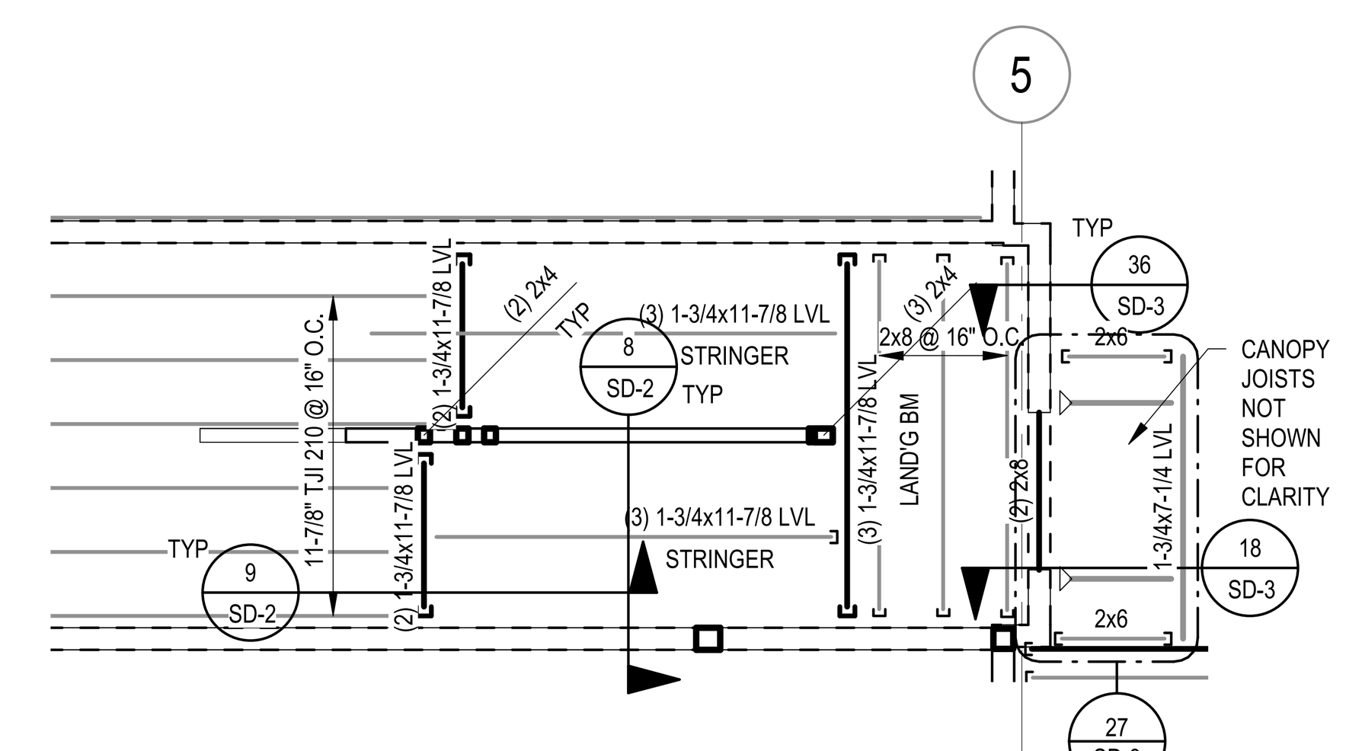
NORTH STAIRS LEVEL 2 (8)
1/4" = 1'-0"



WEST STAIRS LEVEL 4 (2)
1/4" = 1'-0"



WEST STAIRS LEVEL 3 (3)
1/4" = 1'-0"



WEST STAIRS LEVEL 2 (4)
1/4" = 1'-0"

VECTOR ENGINEERS
Boise, Idaho
(208) 996-0803
WWW.VECTORSE.COM
ID FIRM LICENSE NUMBER 1004910

CONSULTANT:

OWNER:

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PROJECT:

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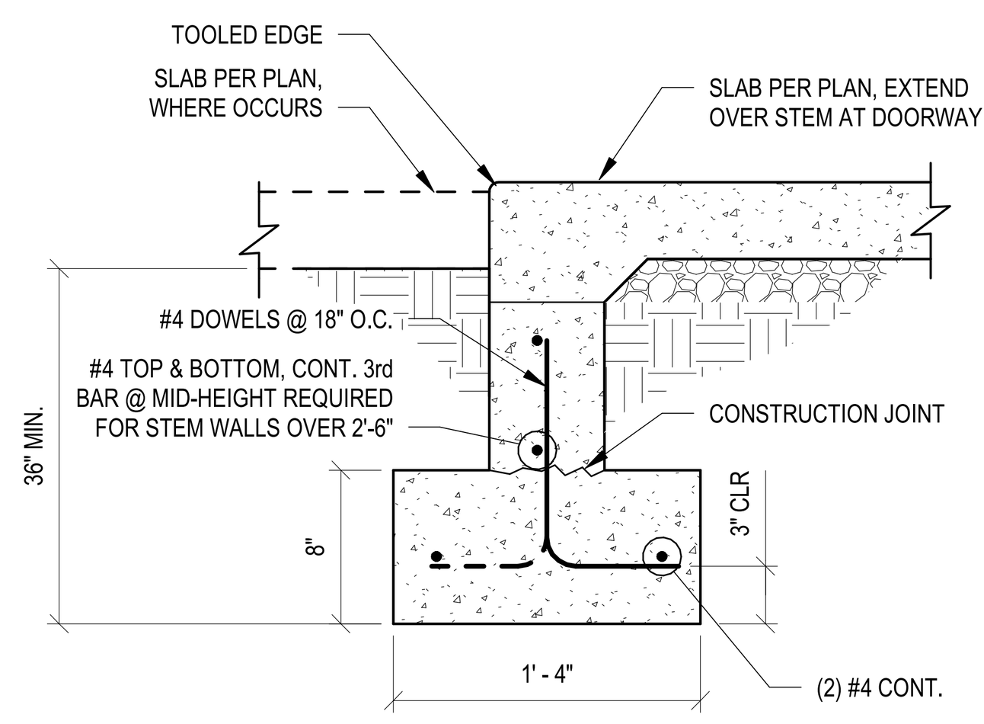
ENLARGED FRAMING PLANS & ELEVATIONS

SHEET #
S11



DETAIL
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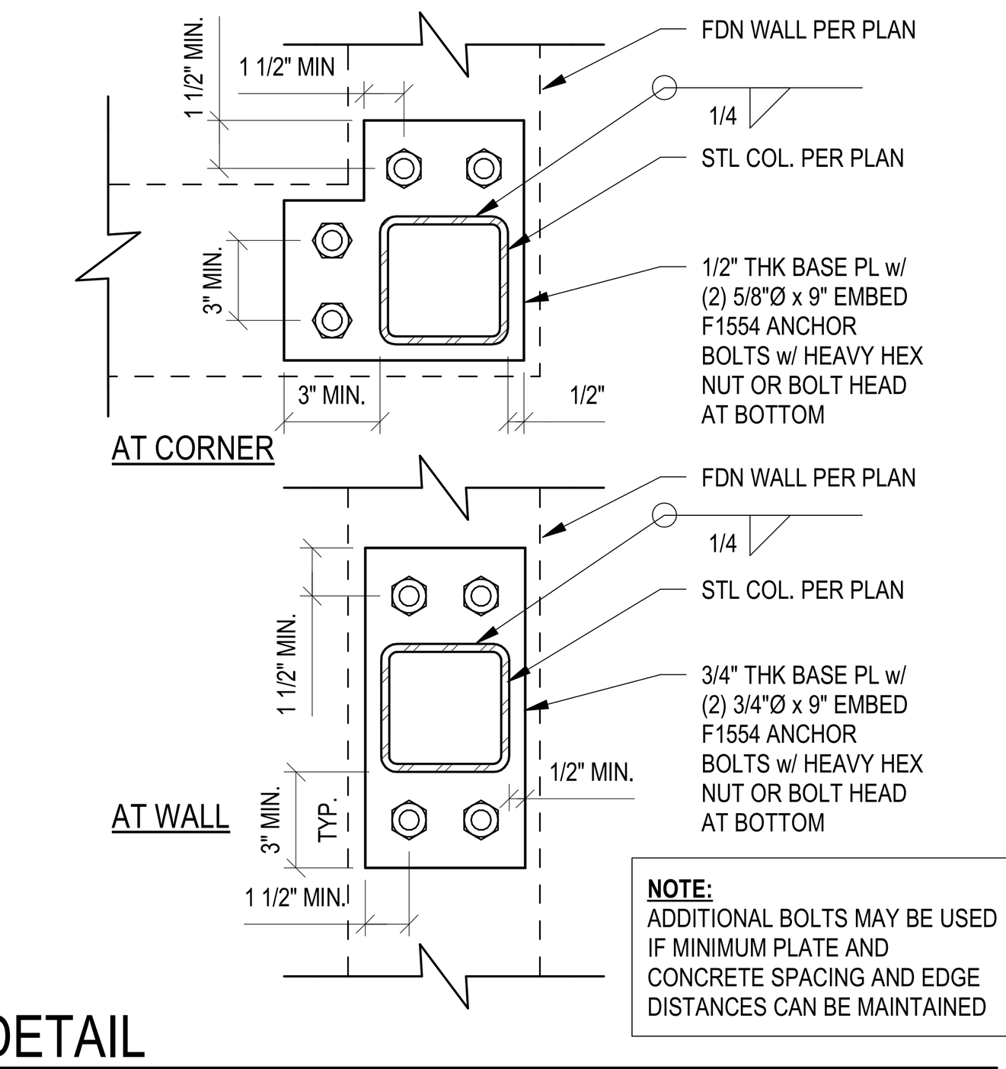
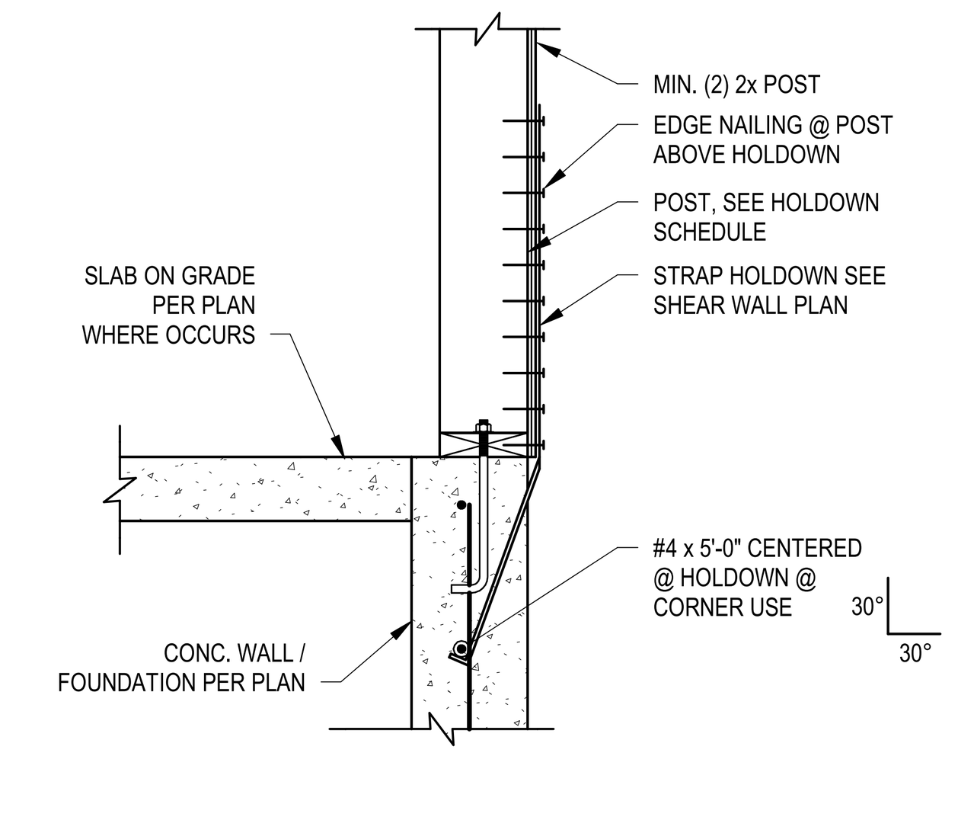
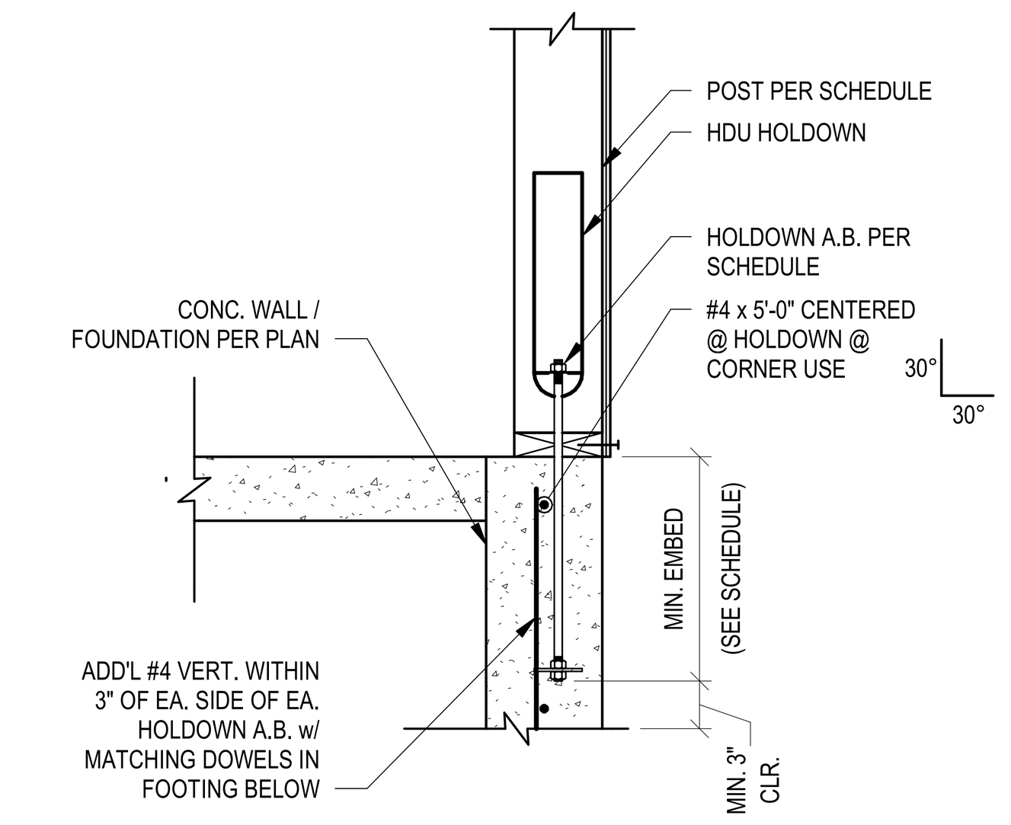
13



DETAIL
N.T.S.
7



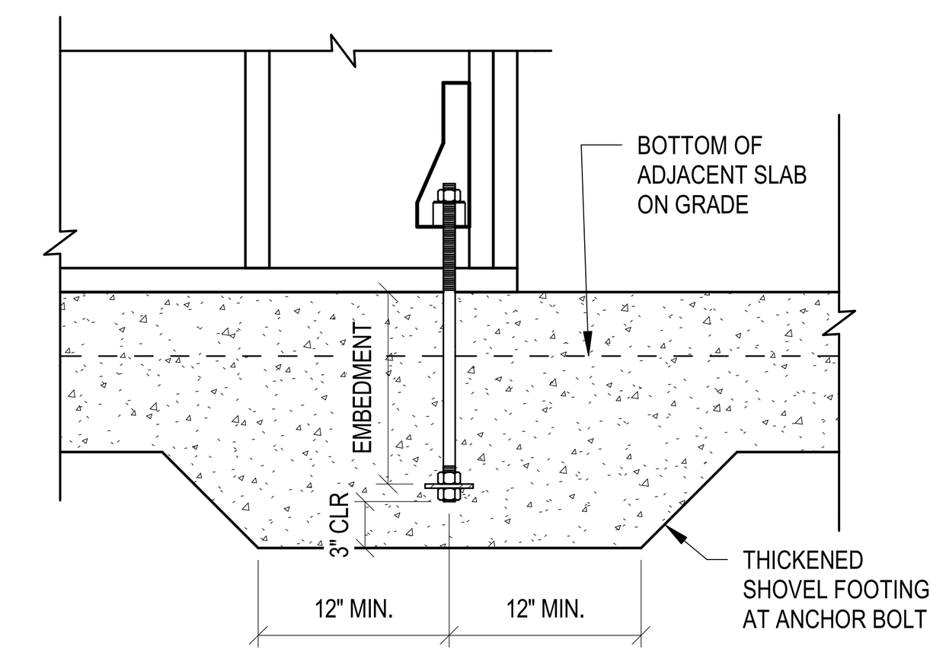
STRAP HOLDOWN



HDU HOLDOWN

ANCHORAGE						
	ANCHORAGE (CAST IN PLACE)		ANCHORAGE (RETROFIT)			
HOLDOWN	SSTB	ALL THREAD ROD (NOTE 1)	ALL THREAD ROD (NOTE 2 & 3)	HOLE DIAMETER	EMBEDMENT	POST
HDU2 - SDS2.5	SSTB24	Ø 5/8" F1554	Ø 5/8" F1554	3/4"	10"	(2) 2x
HDU4 - SDS2.5	SSTB24	Ø 5/8" F1554	Ø 5/8" F1554	3/4"	10"	(2) 2x
HDU5 - SDS2.5	NONE	Ø 5/8" F1554	Ø 5/8" F1554	3/4"	13"	(2) 2x
HDU8 - SDS2.5	NONE	Ø 7/8" F1554	Ø 7/8" F1554	1"	18"	(3) 2x
HDU11 - SDS2.5	NONE	Ø 1" F1554	Ø 1" F1554	1-1/8"	22"	(4) 2x
HDU14 - SDS2.5	NONE	Ø 1" F1554	Ø 1" F1554	1-1/8"	30"	(5) 2x

NOTES:
1. PROVIDE 2-1/8"x2-1/8"x3/8" STEEL PLATE w/ (2) NUTS @ CAST IN PLACE ANCHORS.
2. RETROFIT ALL-THREAD ROD IN HOLES w/ SIMPSON SET-XP EPOXY. PREPARE HOLES & INSTALL EPOXY PER MFR DIRECTIONS w/ EMBEDMENT AND EDGE DISTANCES AS SHOWN.
3. SIMPSON SET-XP EPOXY PER ICC-ES 2508.
4. INCREASE FOOTING DEPTH AS REQUIRED FOR 3" MIN. COVER BELOW BOLT & COORDINATE EXACT LOCATIONS WITH THE FRAMING CONTRACTOR.
5. HOLDOWNS MAY BE INSTALLED 4" MAX. FROM SHEAR WALL EDGE. BOUNDARY NAILING MUST BE PROVIDED @ STUDS ALIGNED WITH HOLDOWNS.
6. SSTB24 HOLDOWNS ARE NOT ALLOWED IN INTERIOR SHOVEL FOOTINGS.
7. INCREASE INTERIOR SHOVEL FOOTING DEPTH TO ACCOMMODATE HOLDOWN ANCHOR EMBEDMENT AT ANCHOR.



SHOVEL FOOTING HOLDOWN

HOLD DOWN ANCHORAGE SCHEDULE
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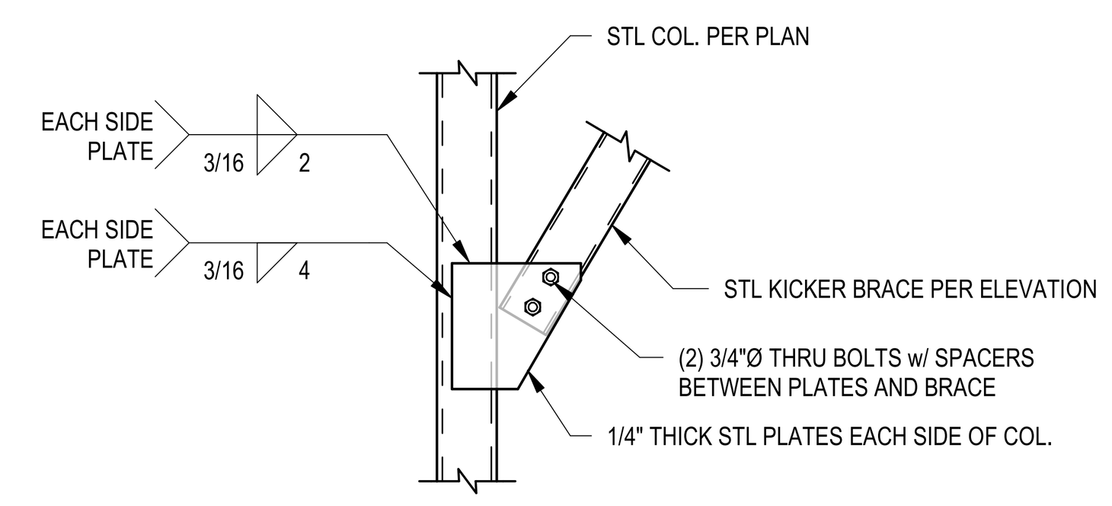
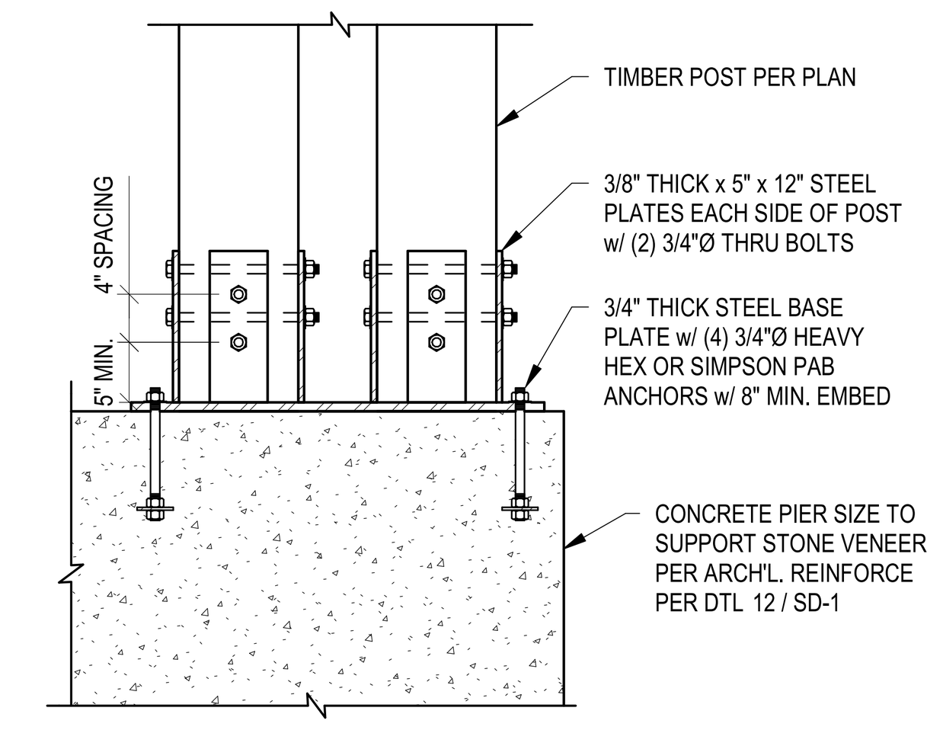
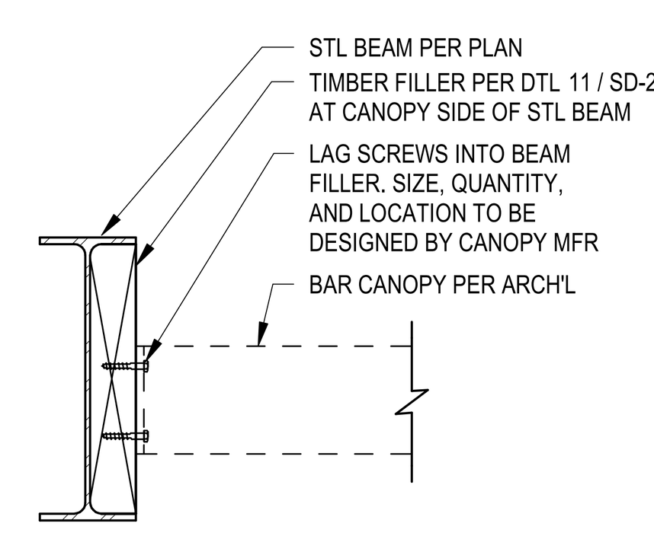
1



DETAIL
N.T.S.
14



DETAIL
N.T.S.
9



DETAIL
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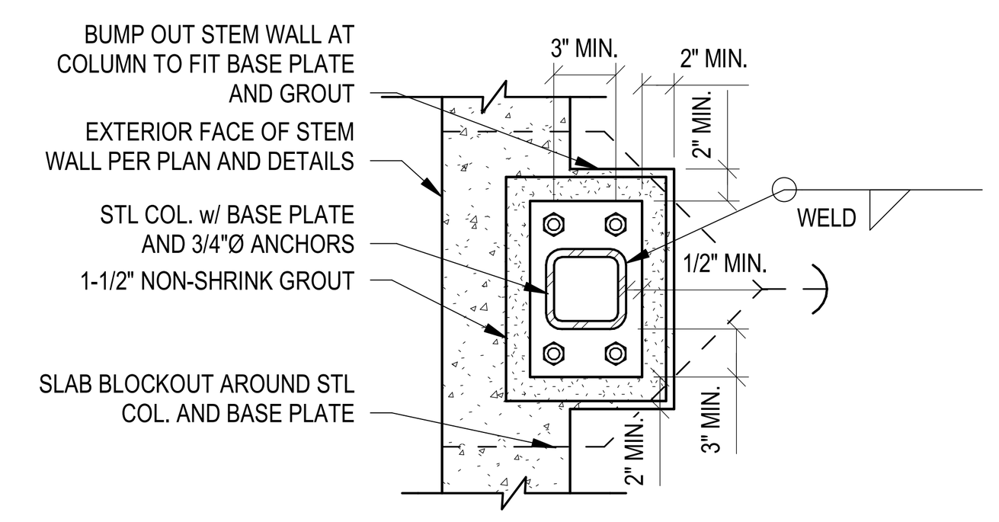
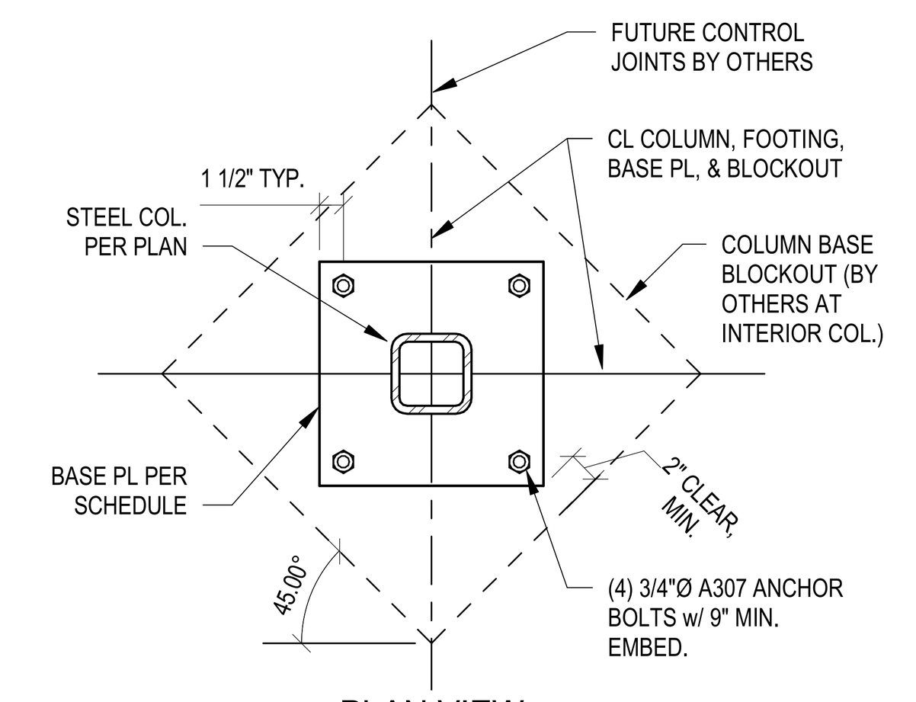
15

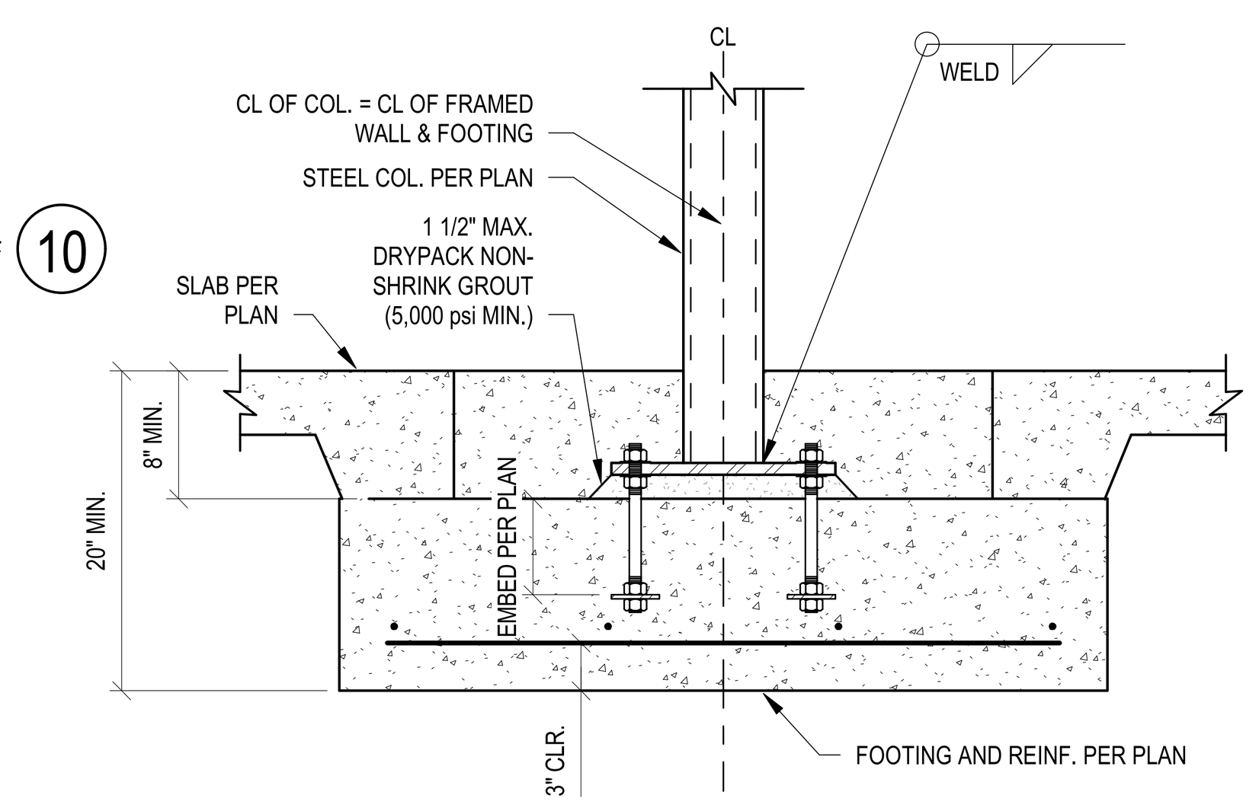


BASE PLATE SCHEDULE			
COLUMN SIZE	BASE PLATE	ANCHOR - EMBED	WELD
HSS 5x5	12"x7"x3/4"	(4) 3/4" - 8"	5/16" FILLET
HSS 6x6	12"x7"x3/4"	(4) 3/4" - 8"	5/16" FILLET
HSS 8x8	14"x9"x1"	(4) 1" - 12"	1/2" FILLET

STEM WALL COLUMN PIER
N.T.S.
10

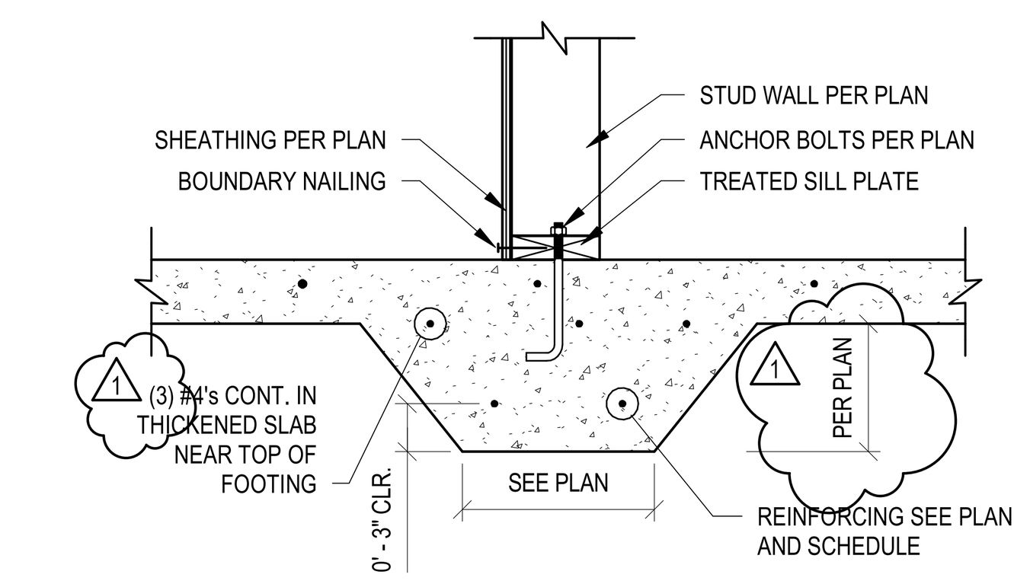


PLAN VIEW

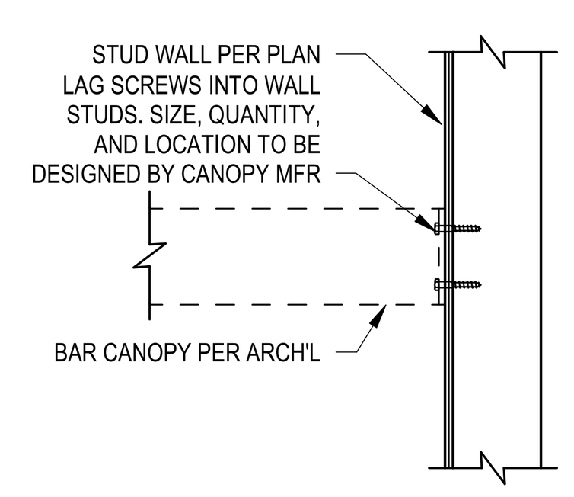


BASE PLATE SCHEDULE			
COLUMN SIZE	BASE PLATE	ANCHOR - EMBED	WELD
HSS 5x5	12"x12"x3/4"	(4) 3/4"Ø - 9"	5/16" FILLET
HSS 6x6	12"x12"x3/4"	(4) 3/4"Ø - 9"	5/16" FILLET
HSS 8x8	14"x14"x1"	(4) 1"Ø - 12"	1/2" FILLET
HSS 14x6	16"x12"x1"	(4) 1"Ø - 12"	1/2" FILLET
-	-	-	-

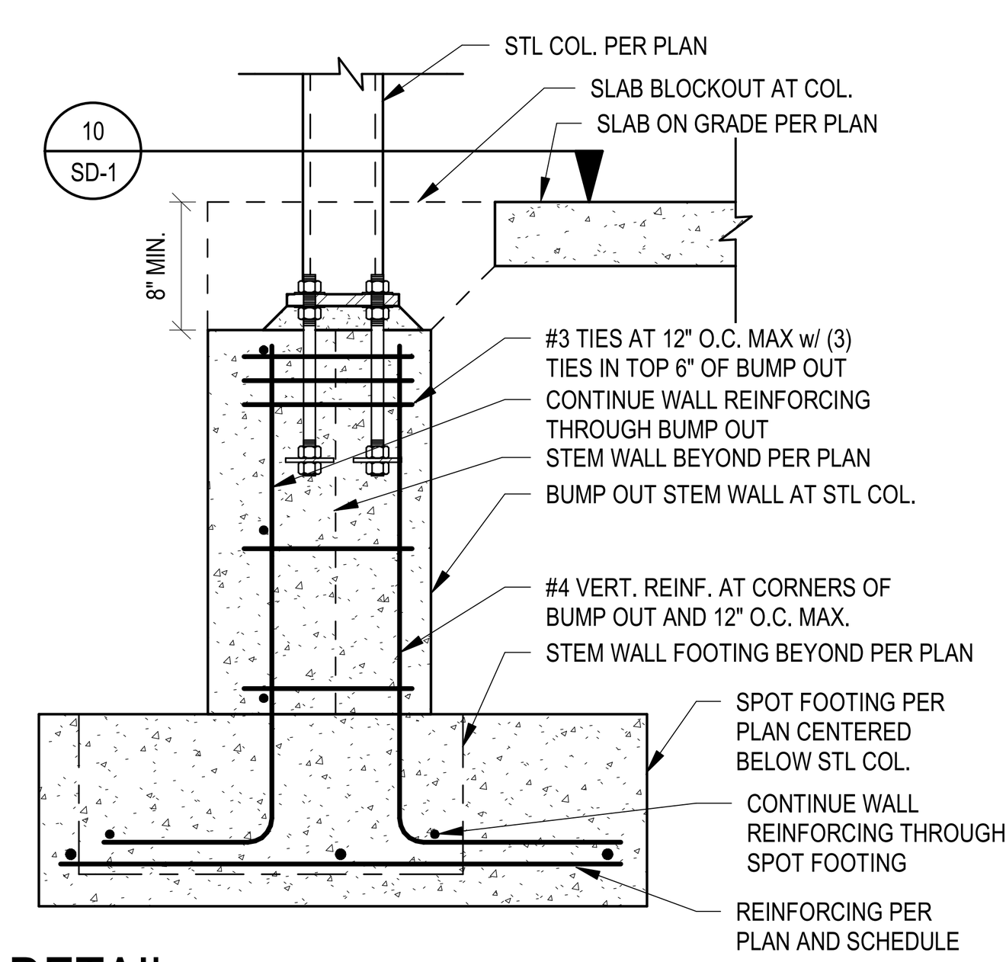
DETAIL
N.T.S.
5



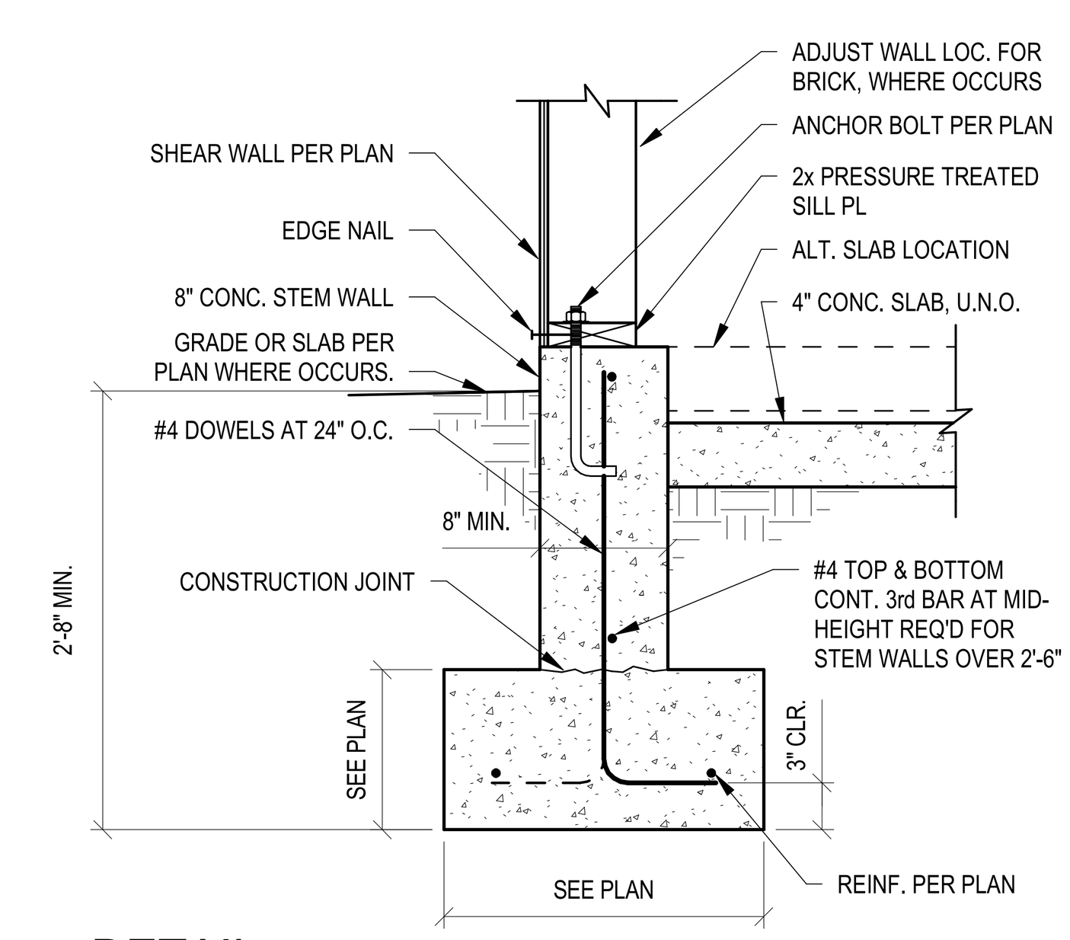
DETAIL
N.T.S.
2



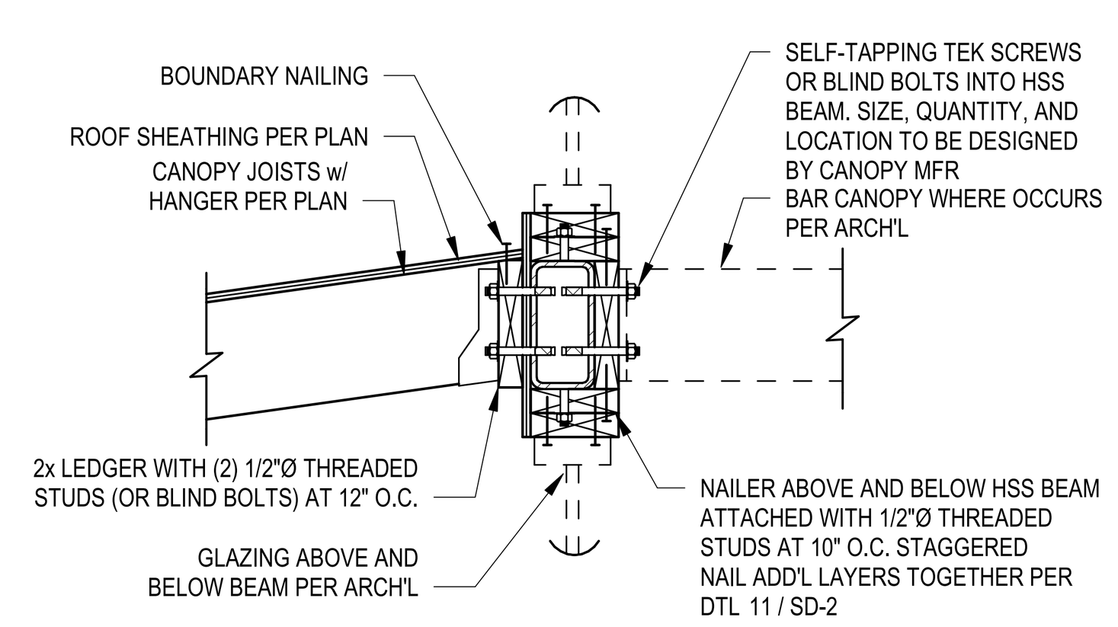
DETAIL
N.T.S.
16



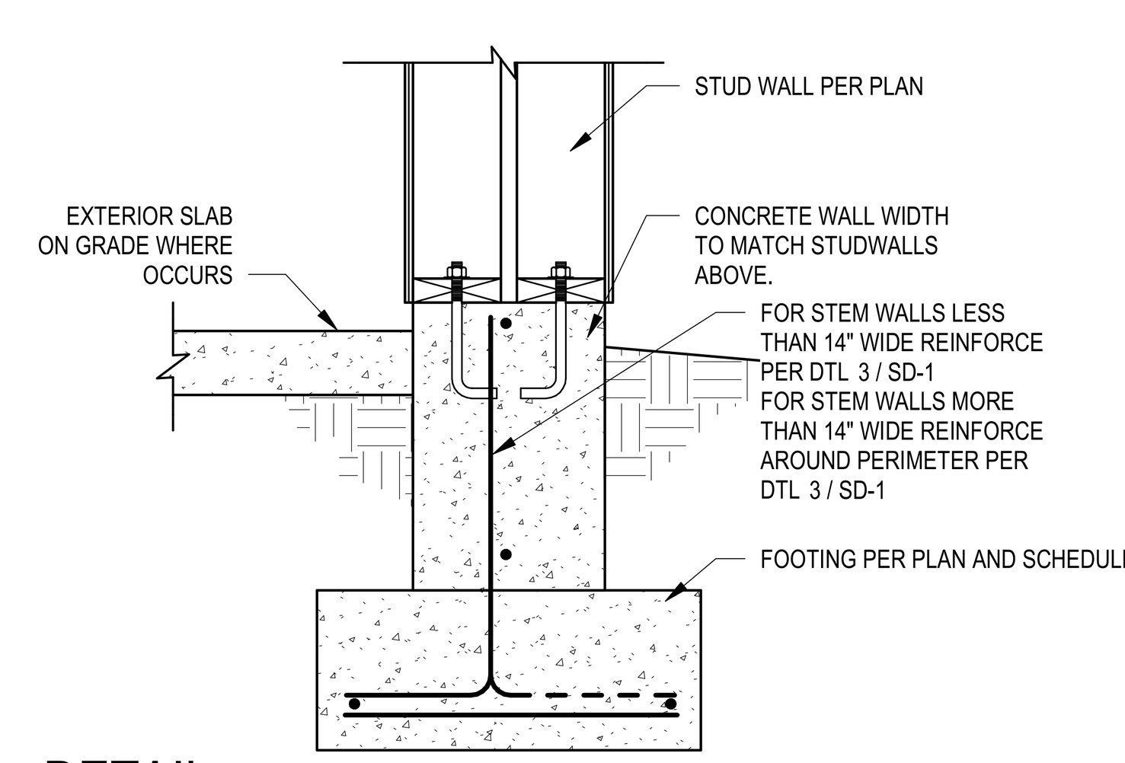
DETAIL
N.T.S.
11



DETAIL
N.T.S.
3



DETAIL
N.T.S.
17



DETAIL
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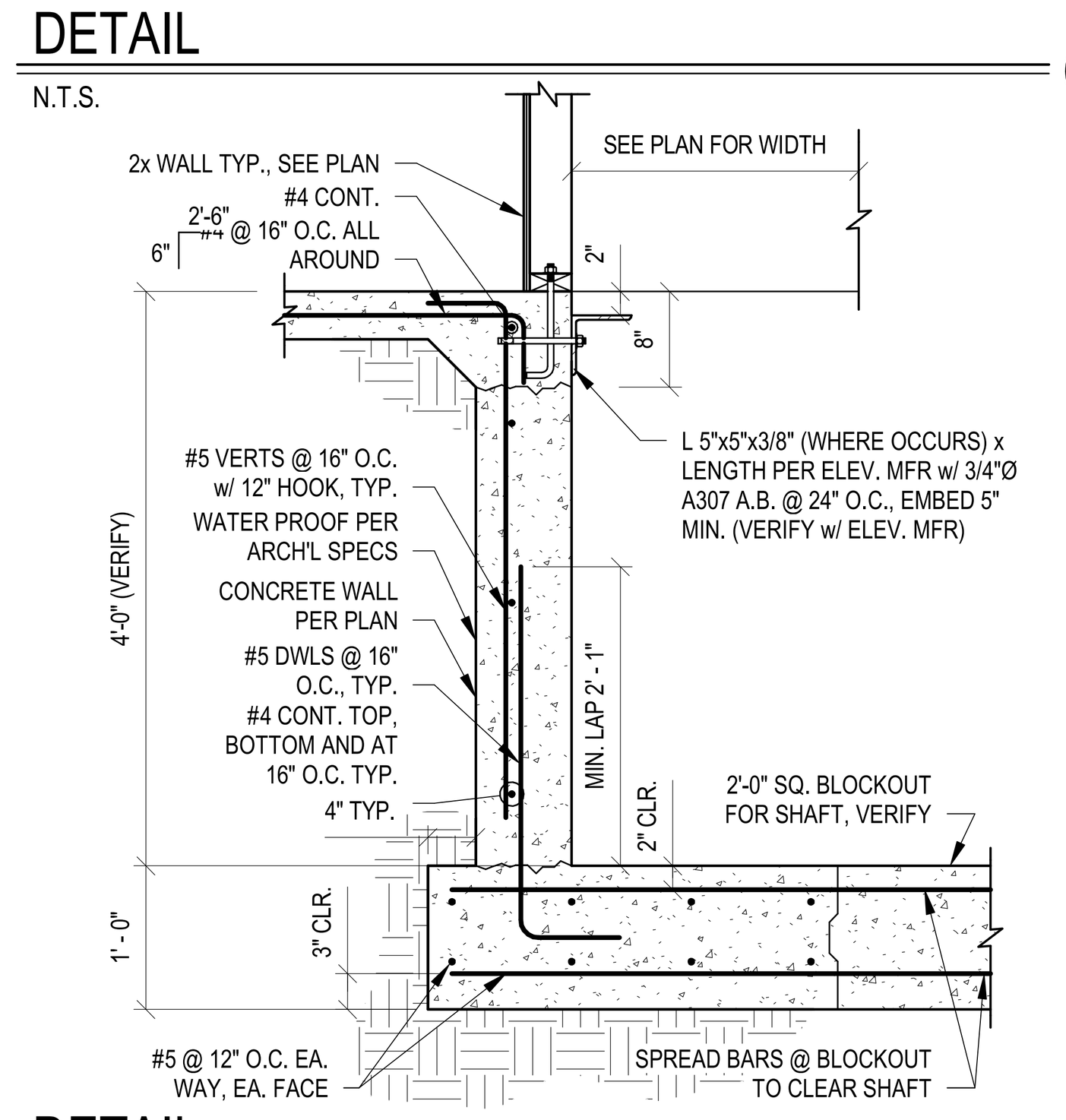
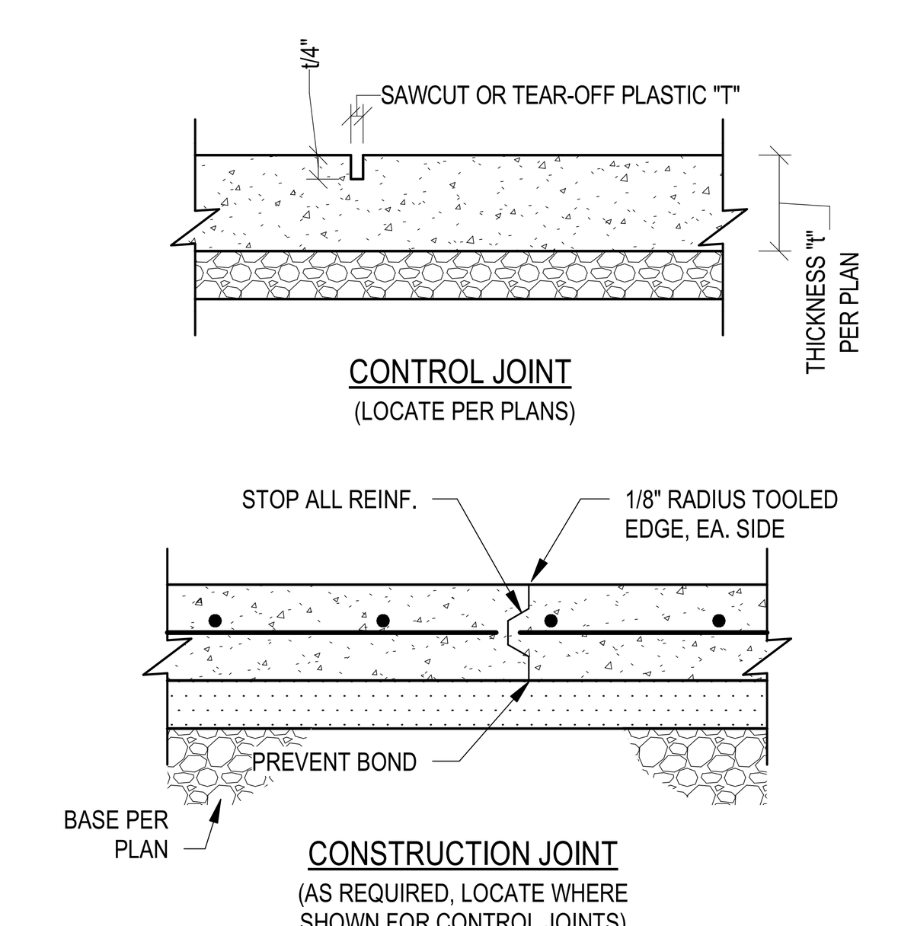
12

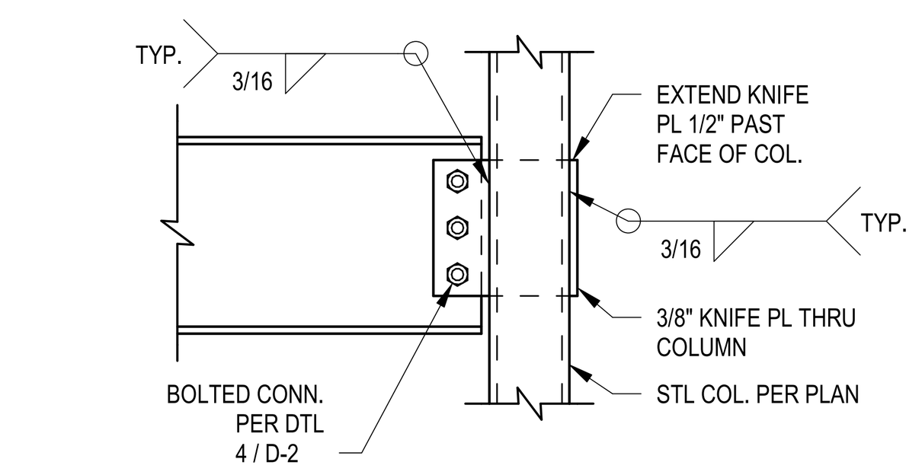


DETAIL
N.T.S.
6



DETAIL
N.T.S.
4

VECTOR ENGINEERS
CONSULTANT:
OWNER:
ELEMENT HOTEL
PROJECT:
PROPRIETARY DO NOT DISTRIBUTE
ISSUE DATE | DESCRIPTION
REV. DATE | DESCRIPTION
For Review
02/24/2025 10:13:22 AM
100% CD
STRUCTURAL DETAILS
SHEET #
SD-1



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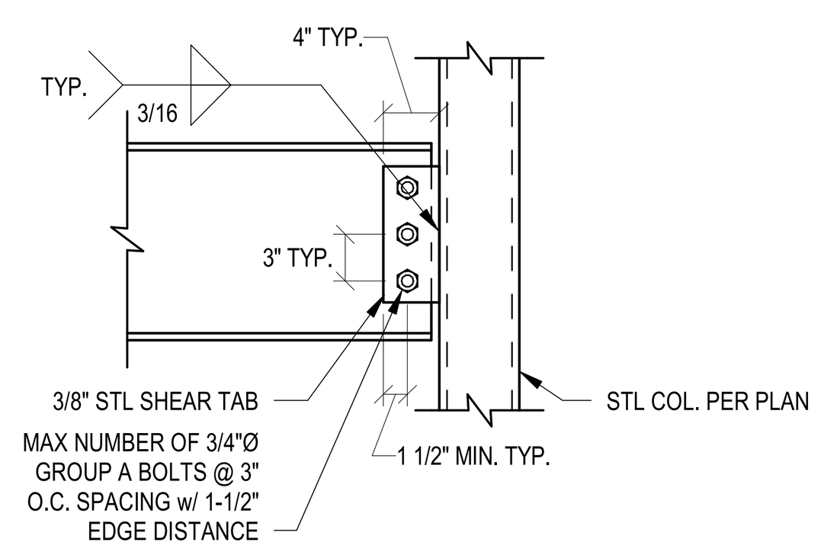

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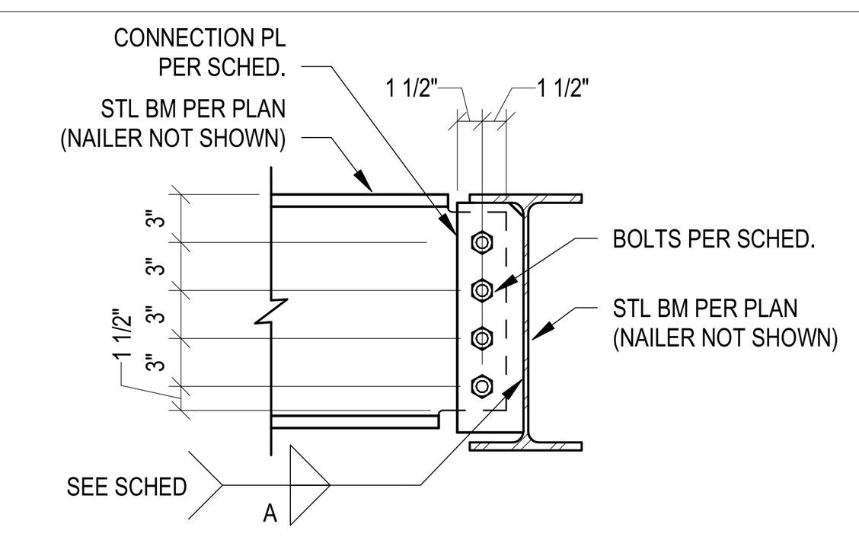
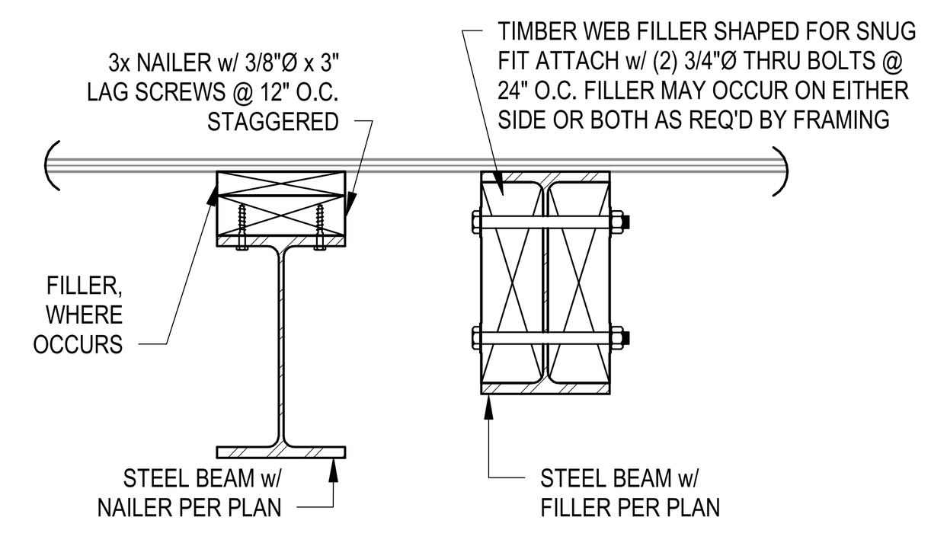


BEAM CONNECTION SCHEDULE

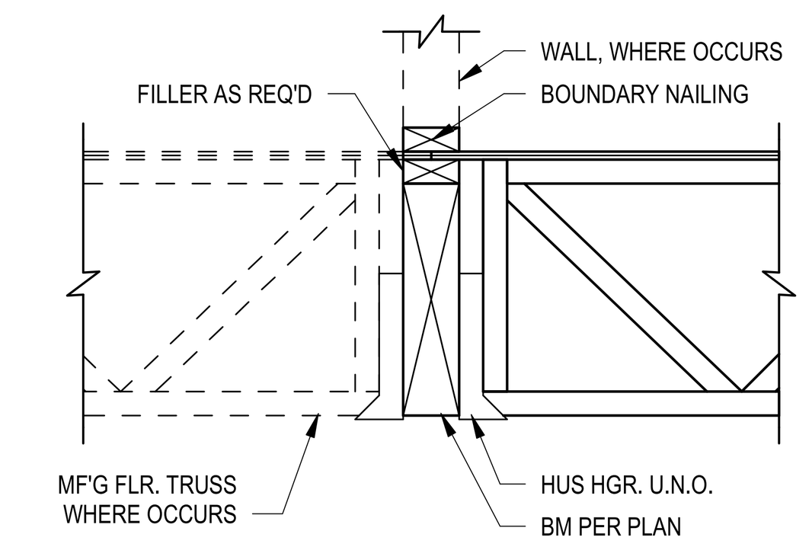
SHEAR > CONNECTION			
MEMBER (DEPTH)	# OF BOLTS CONN. 3/4" DIA. A325N	PL t	WELD SIZE A
8"	2	5/16"	1/4"
8"-10"	2	5/16"	1/4"
12"-14"	3	5/16"	1/4"
15"-16"	4	3/8"	5/16"
18"-21"	5	3/8"	5/16"
24"	6	3/8"	5/16"

DETAIL 16
N.T.S.



NOTE:
1. WEB FILLER SHALL BE NOTCHED AROUND NAILER LAG SCREW HEADS WHERE NAILER AND FILLER OCCUR ON THE SAME STEEL BEAM.
2. FILLER THAT OCCURS ABOVE NAILER SHALL BE NAILED TO NAILER (AND LOWER LAYERS WHERE OCCURS) WITH (2) 16d NAILS AT 8" O.C.

DETAIL 11
N.T.S.



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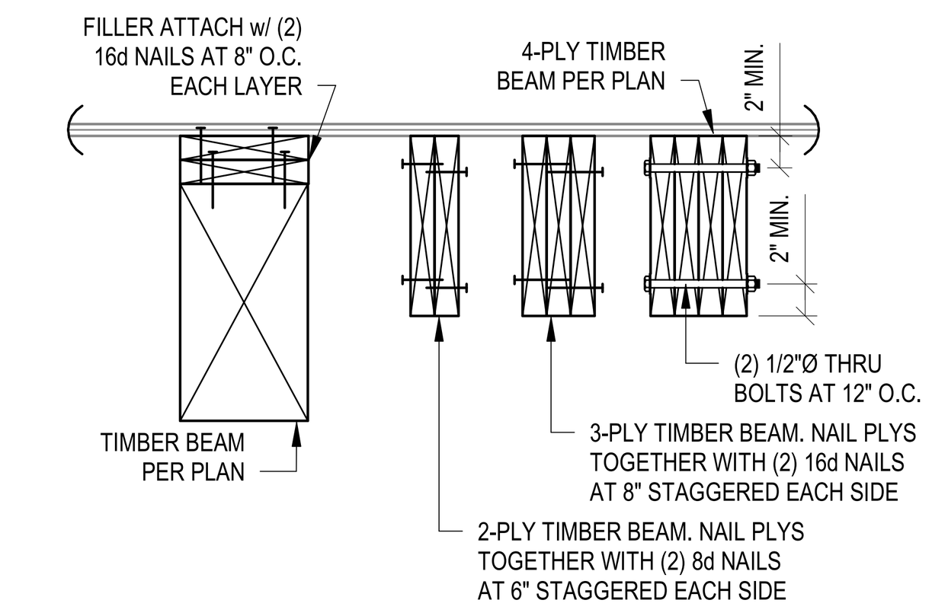

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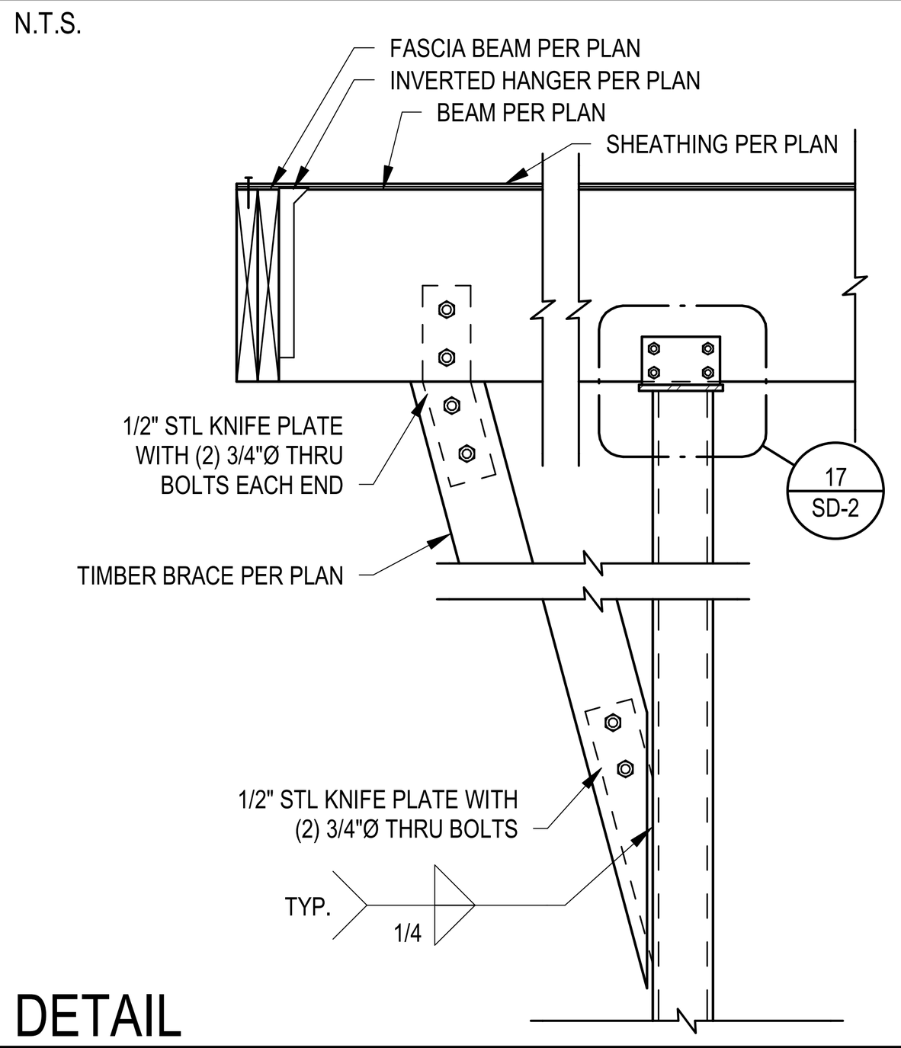

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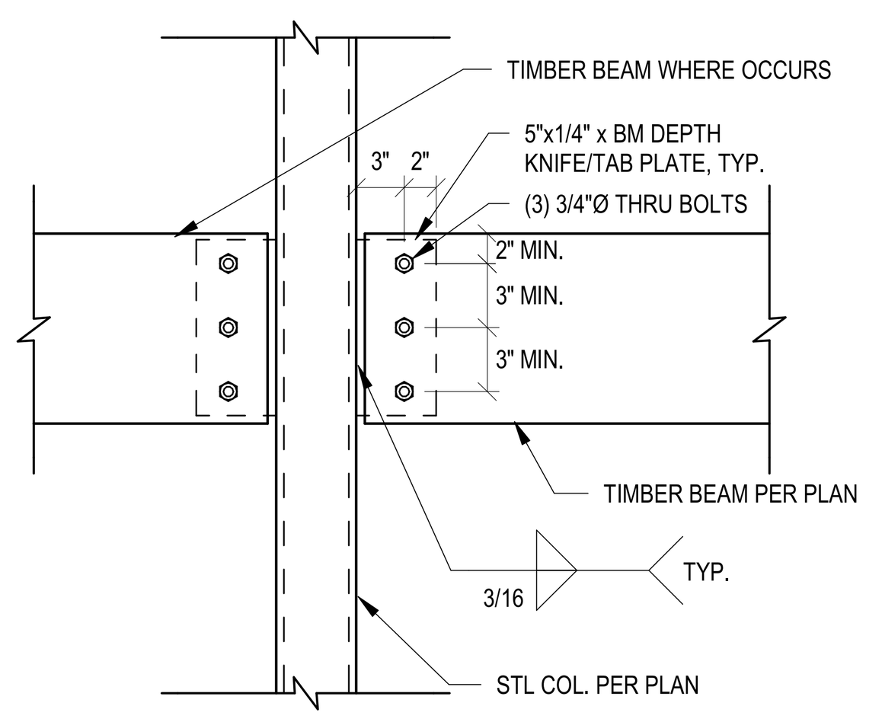

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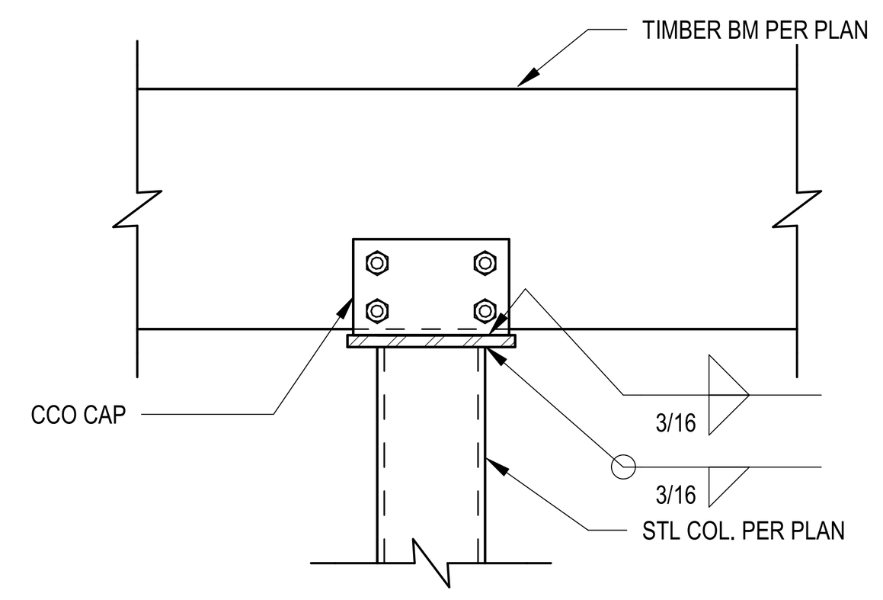

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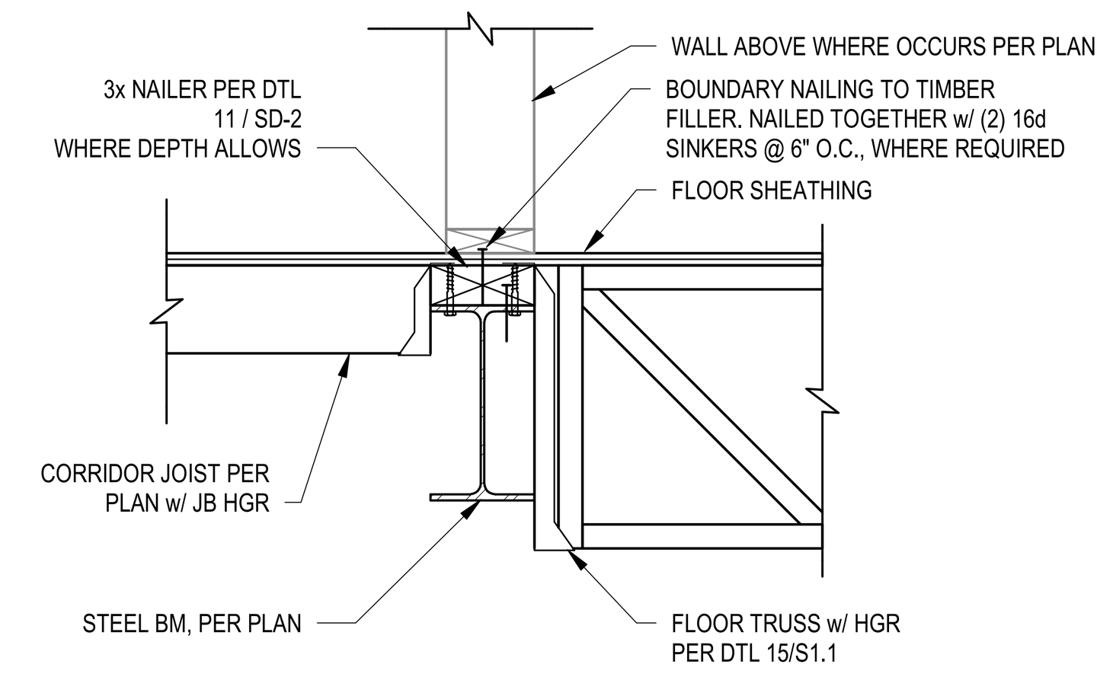

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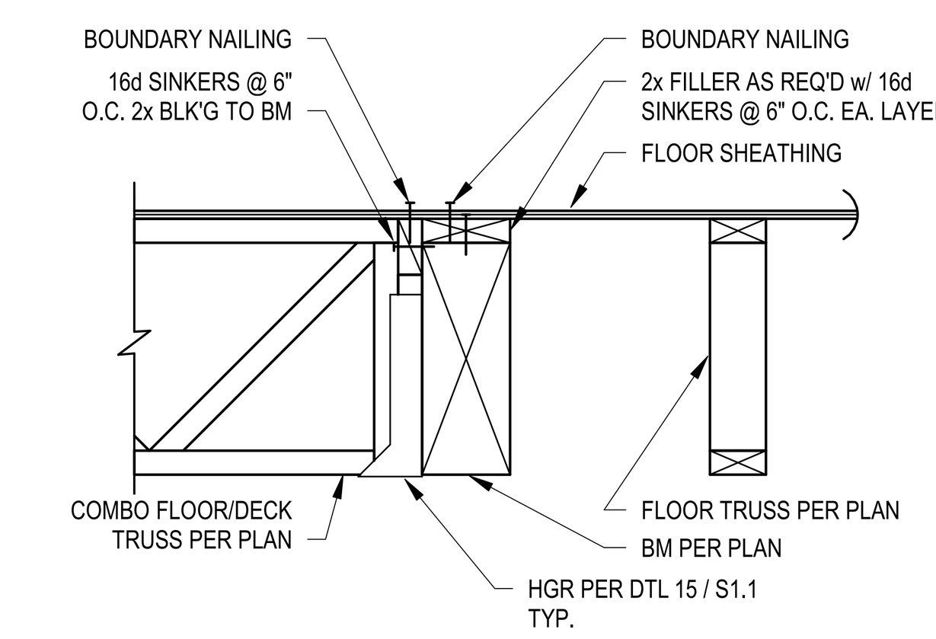

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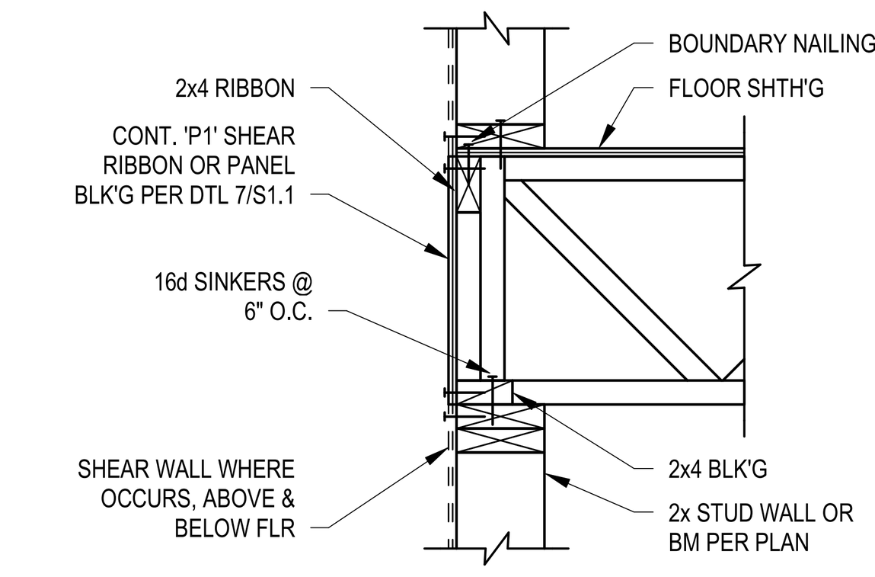

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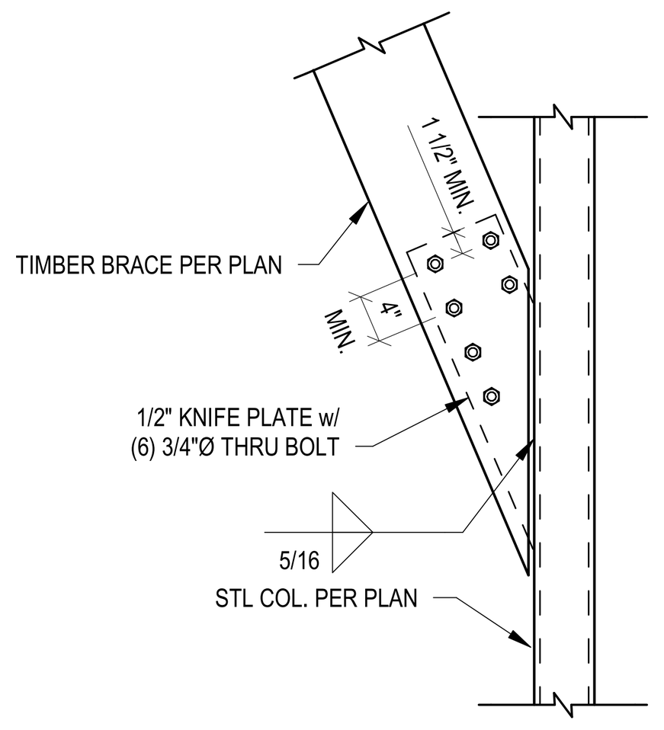

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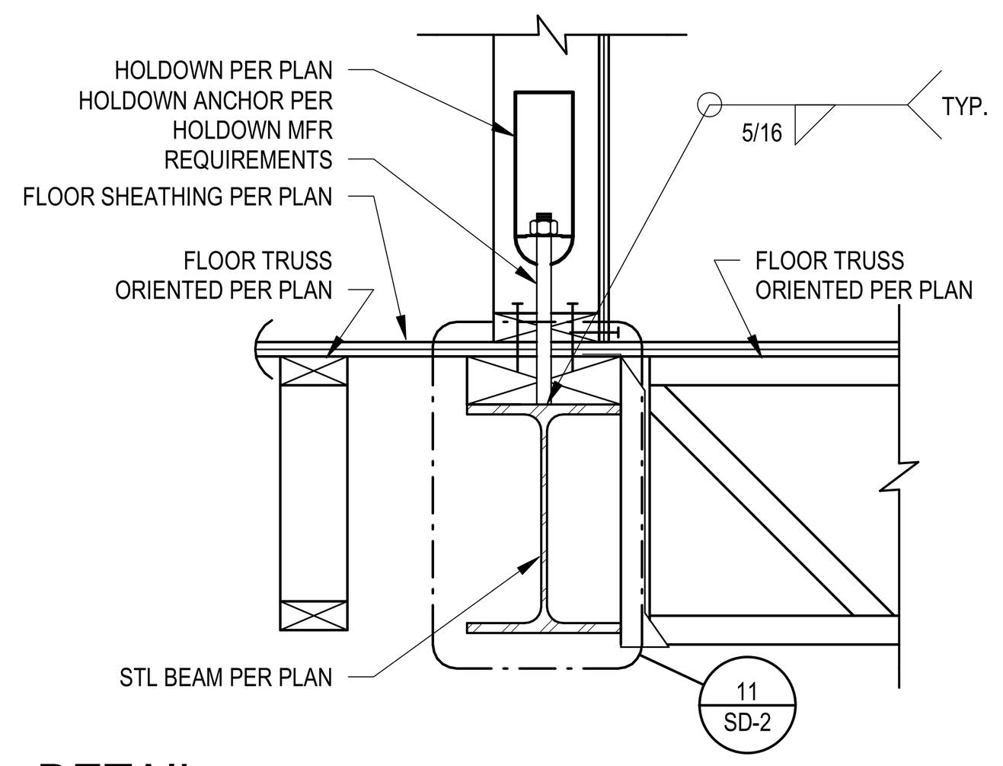

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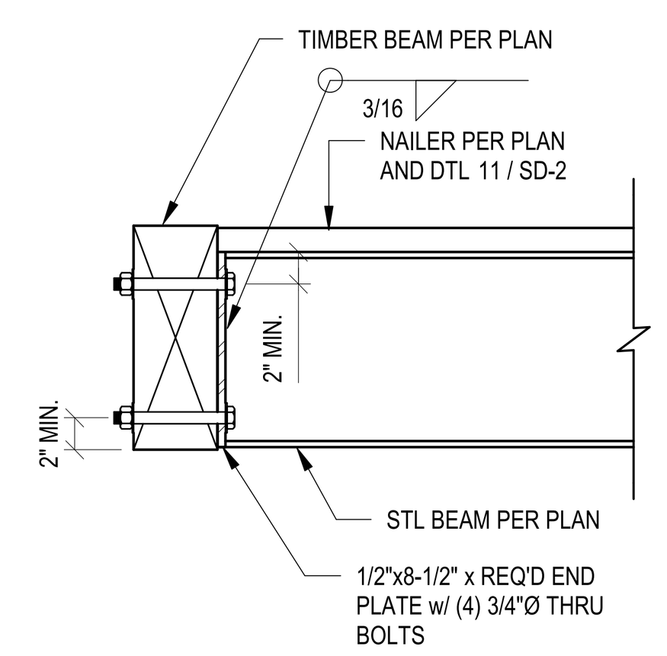

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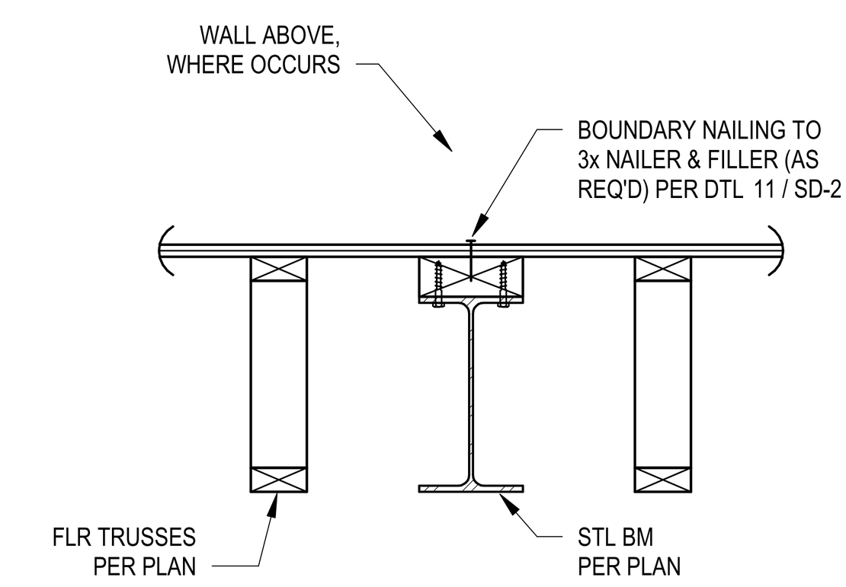

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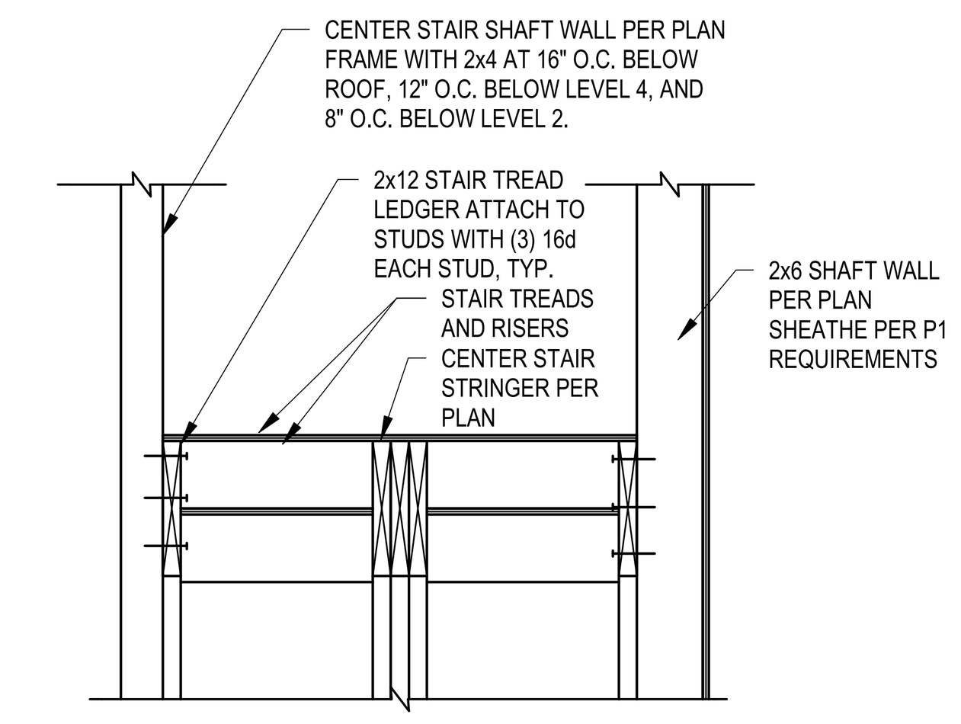

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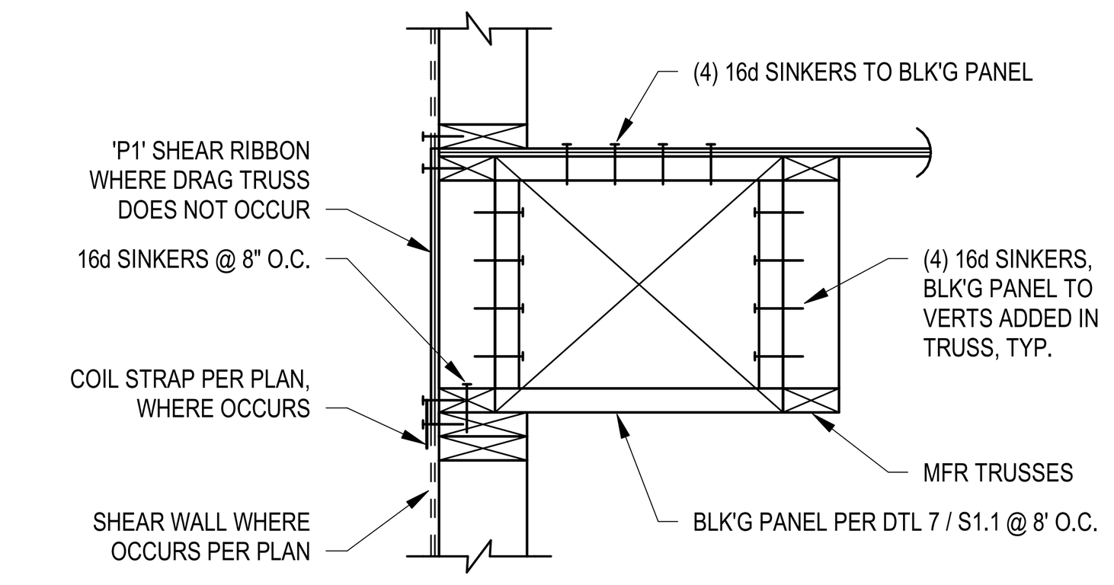

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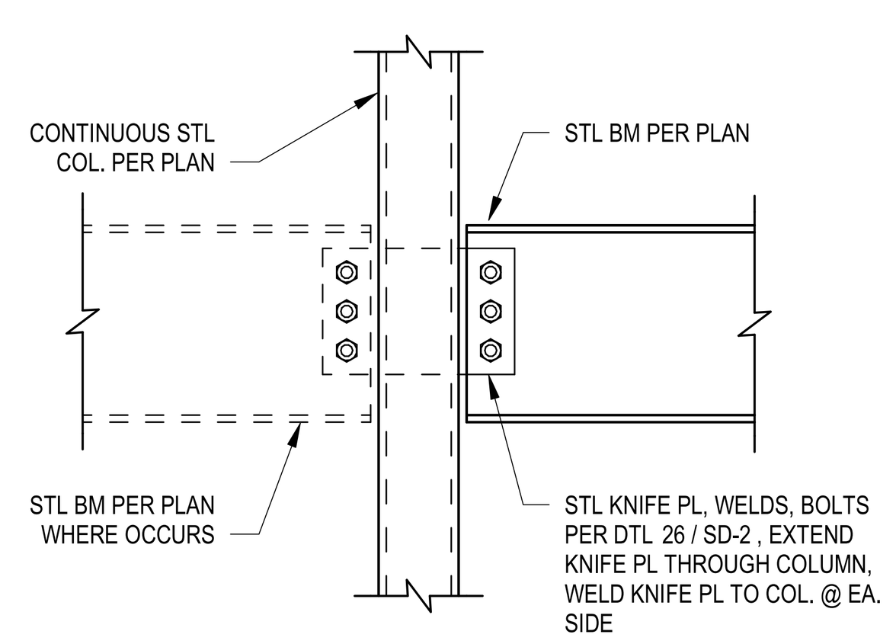

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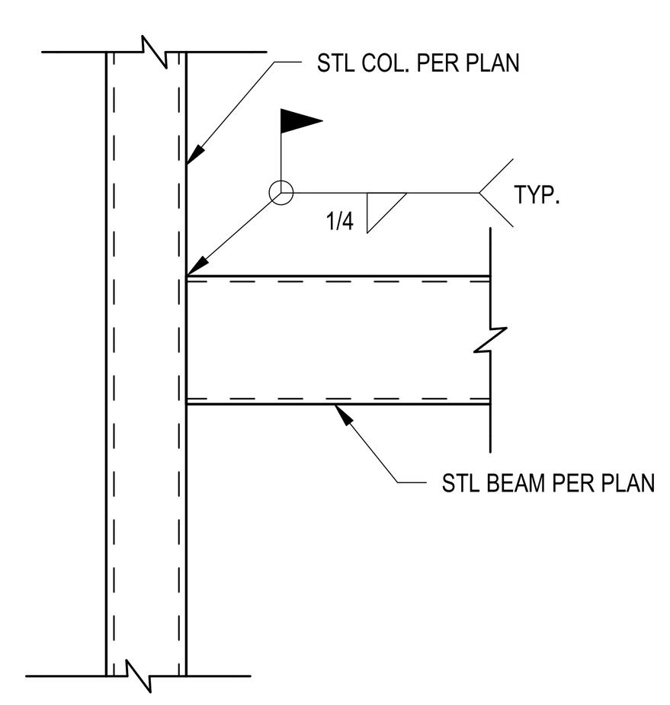

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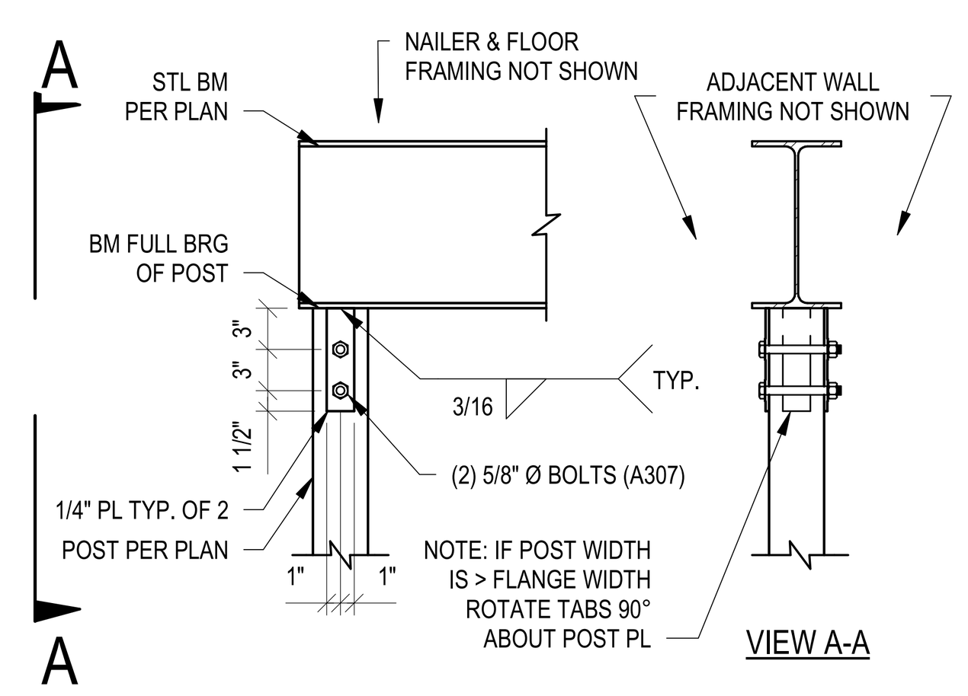

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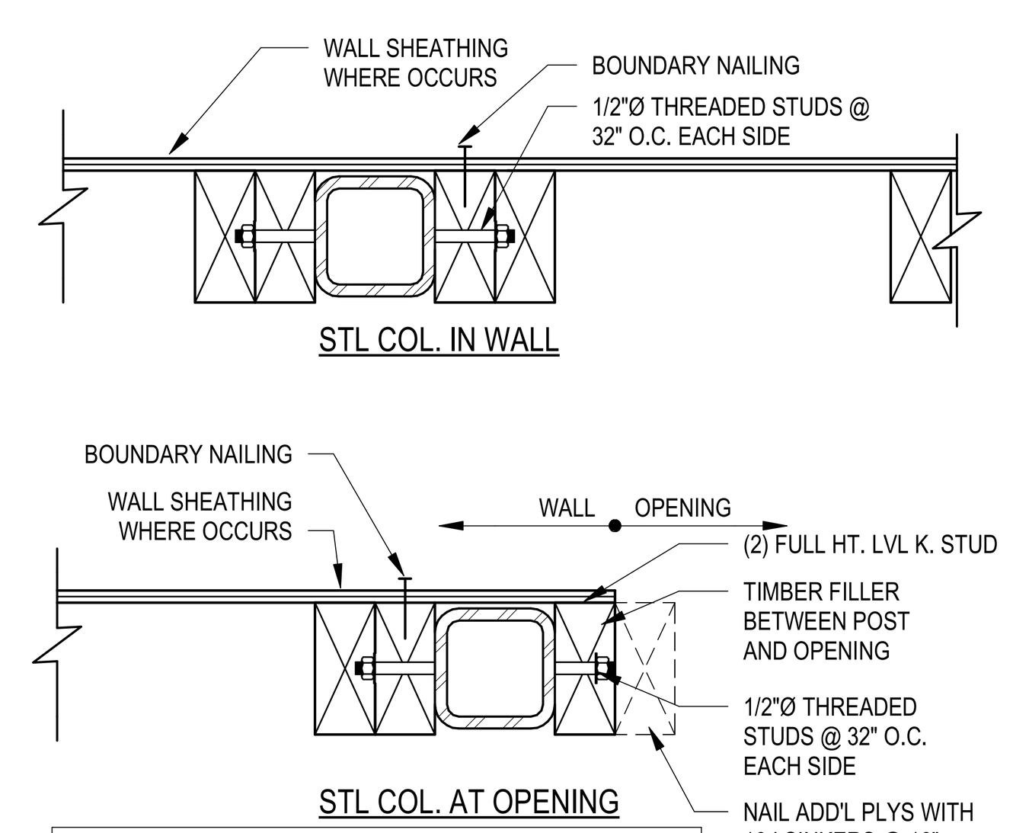
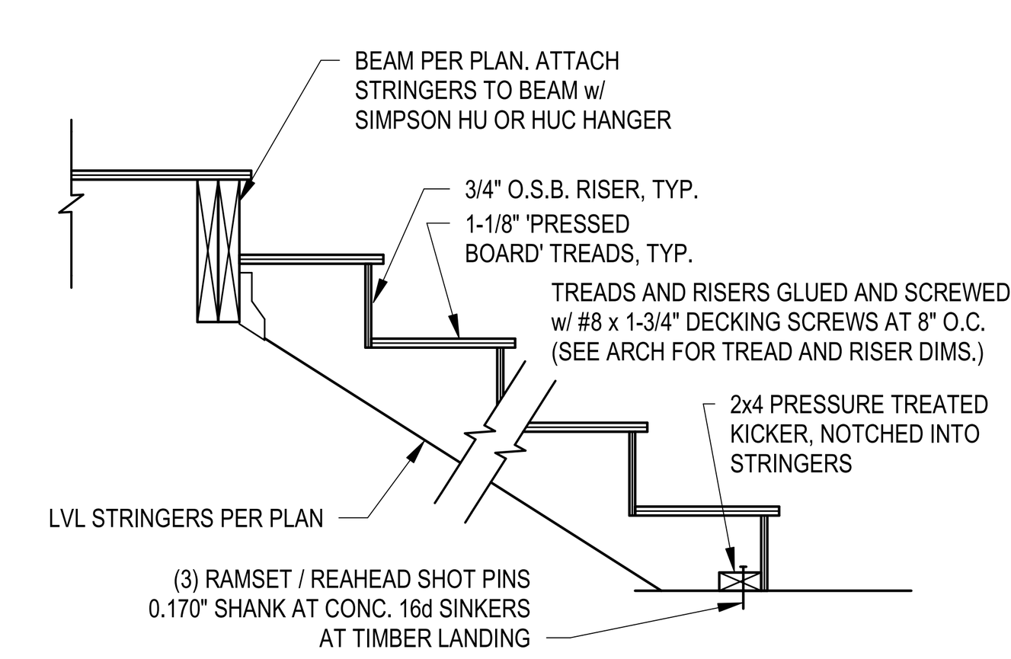


NOTE:
SIMILAR CONDITION w/ THREADED STUDS ROTATE 90 DEG. FOR COLUMNS AT CORNER OF WALLS.

DETAIL 14
N.T.S.



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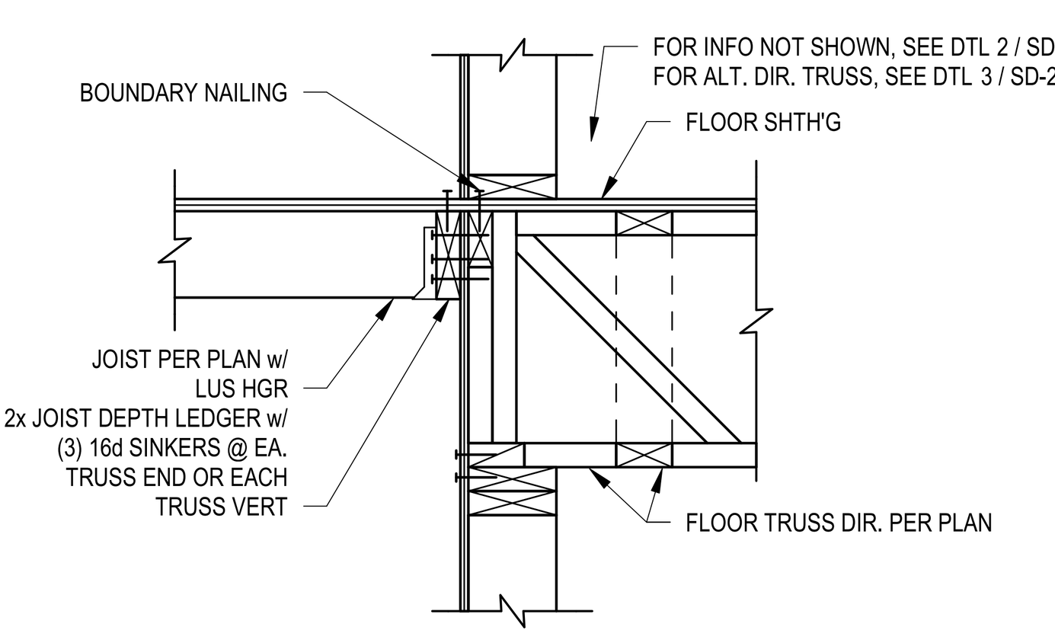

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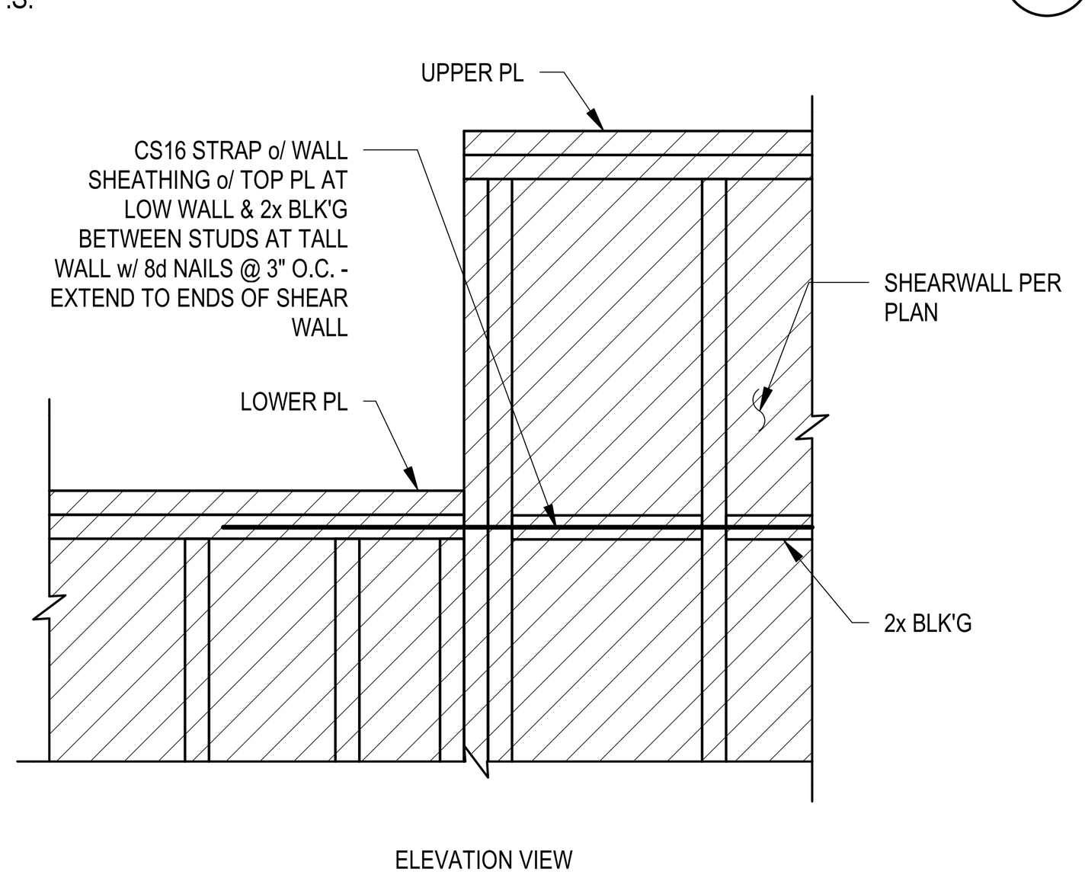

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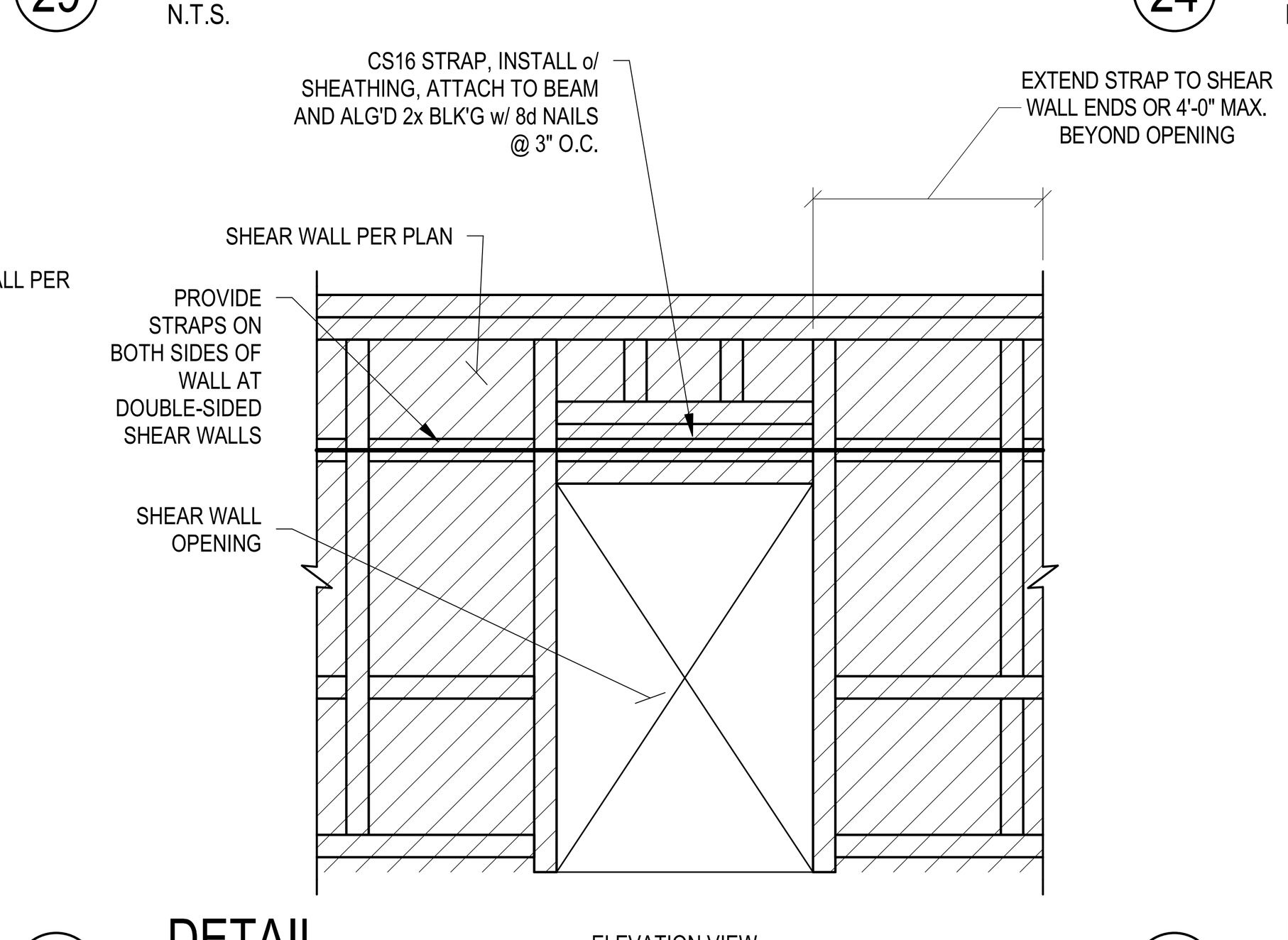

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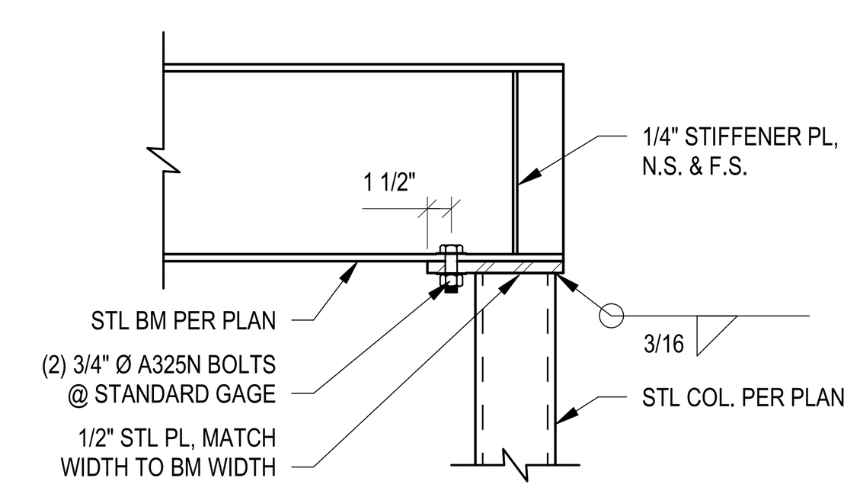

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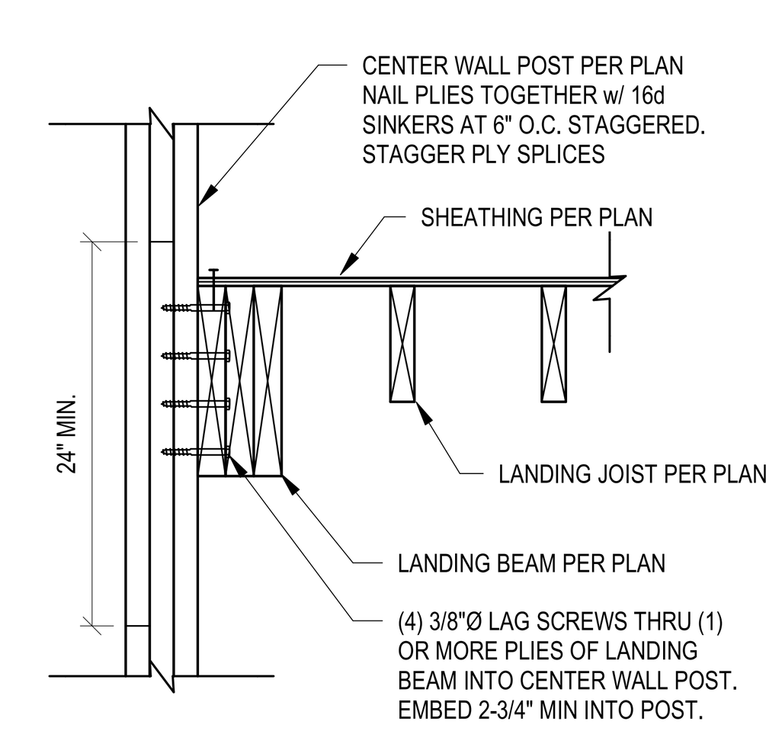

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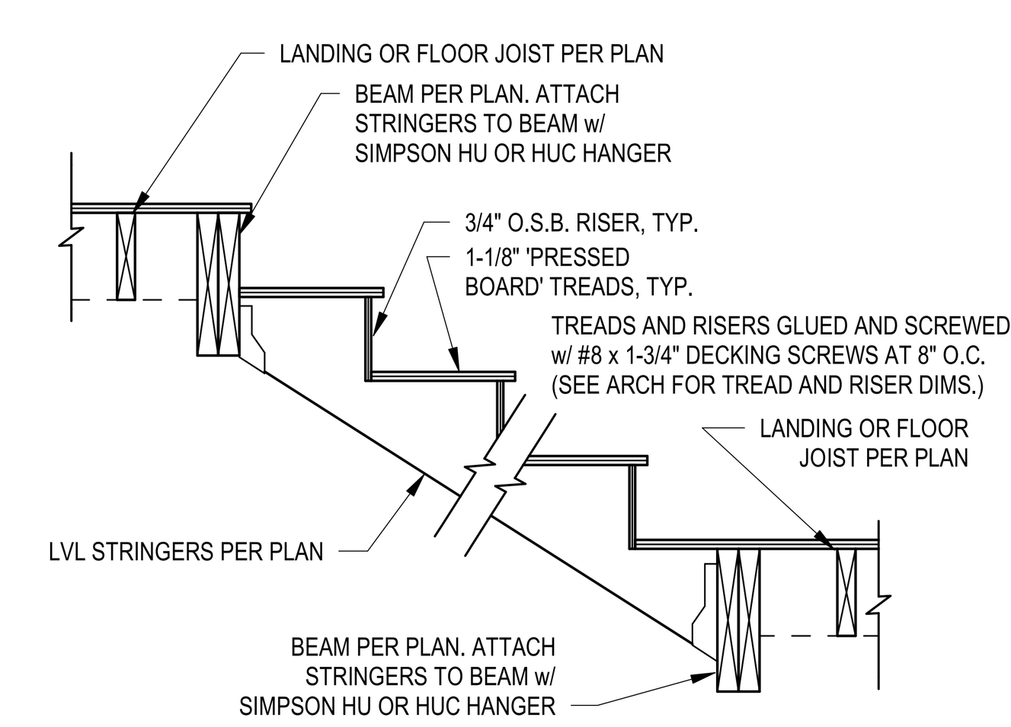
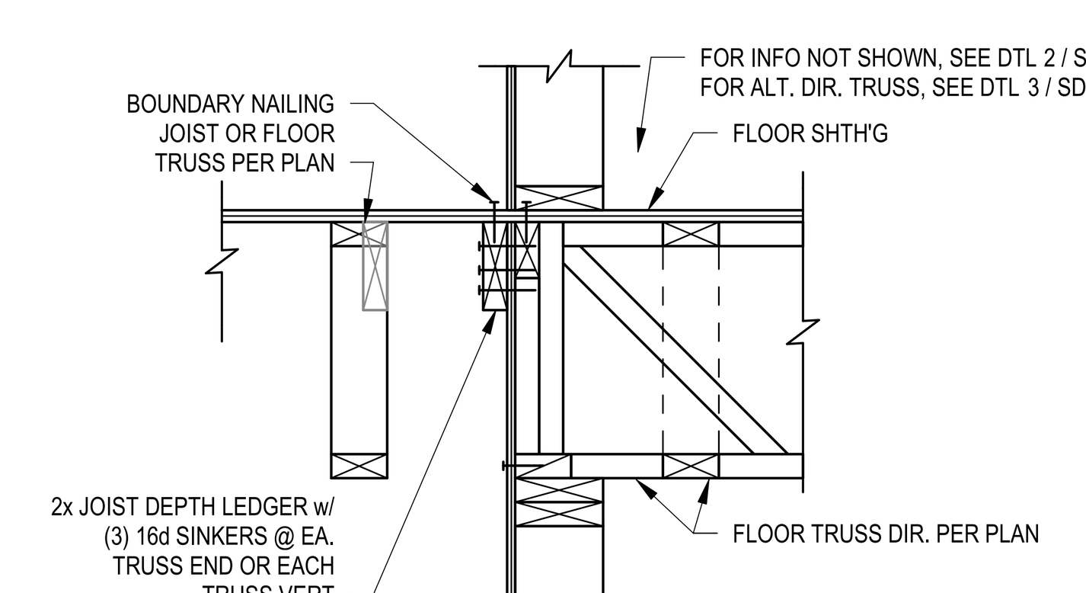


DETAIL 10
3/4" = 1'-0"



DETAIL 5
N.T.S.

VECTOR ENGINEERS
BOISE, IDAHO
WWW.VECTORSE.COM
ID FIRM LICENSE NUMBER
CONSULTANT:

OWNER:

ELEMENT HOTEL
PROJECT:

PROPRIETARY DO NOT DISTRIBUTE

#	ISSUE DATE	DESCRIPTION
#	REV. DATE	DESCRIPTION

For Review
02/24/2025 10:13:22 AM

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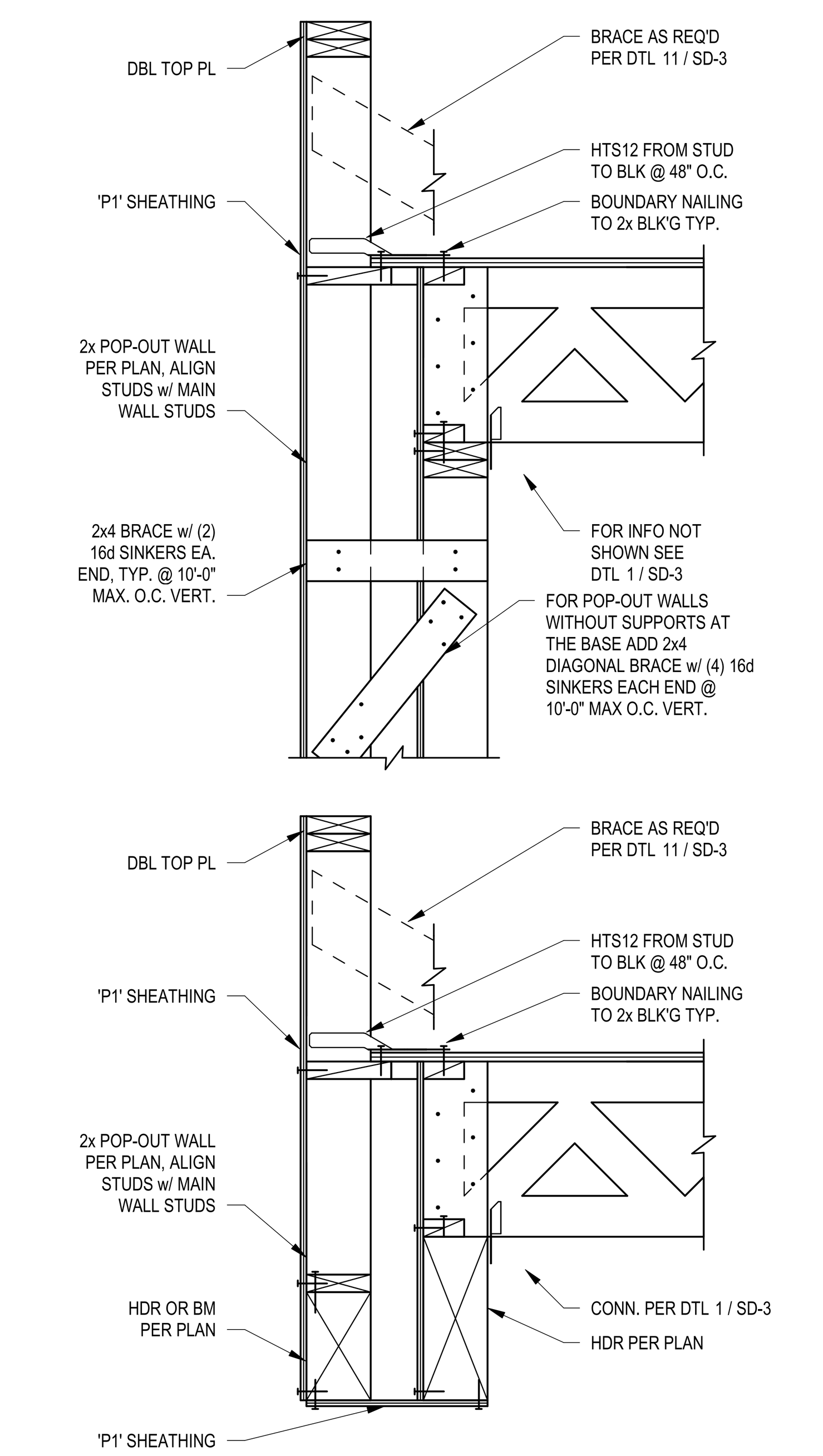
STRUCTURAL DETAILS

SHEET #
SD-2



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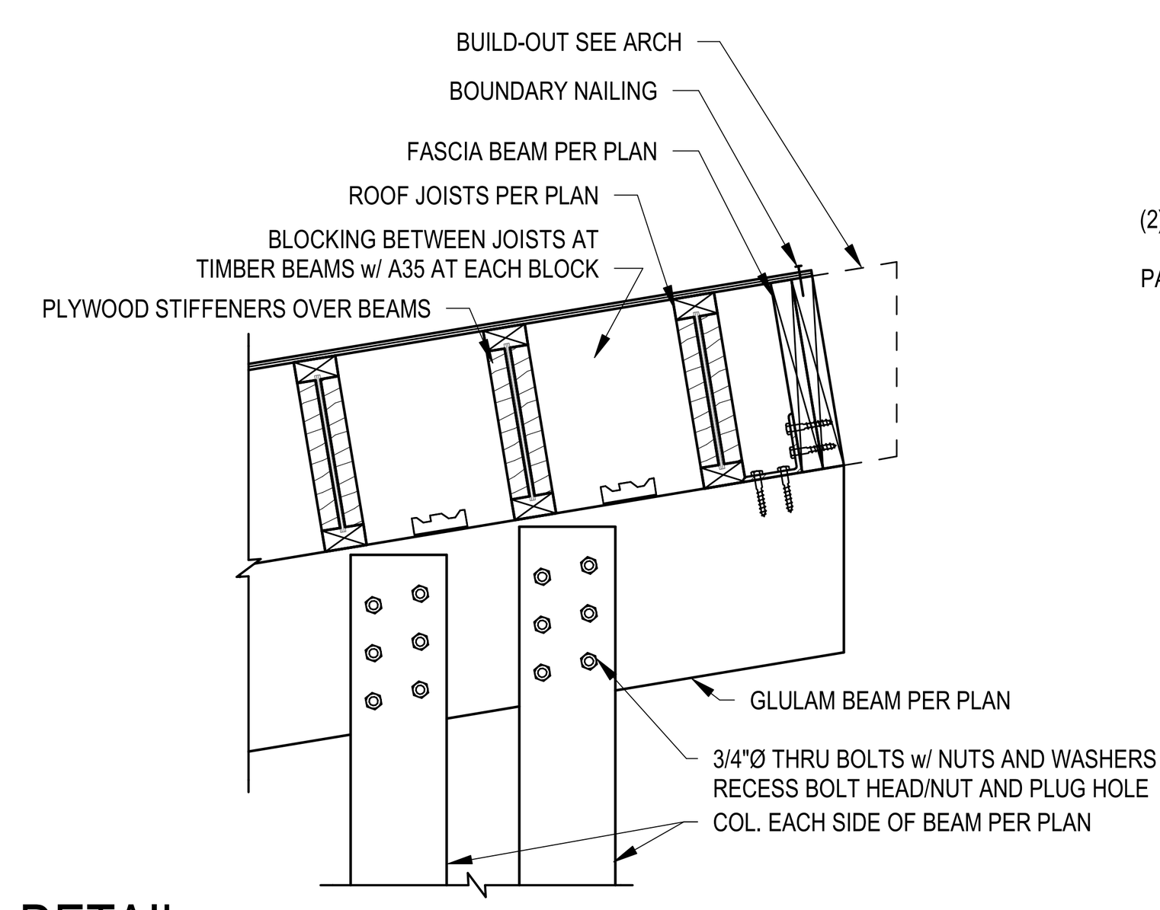

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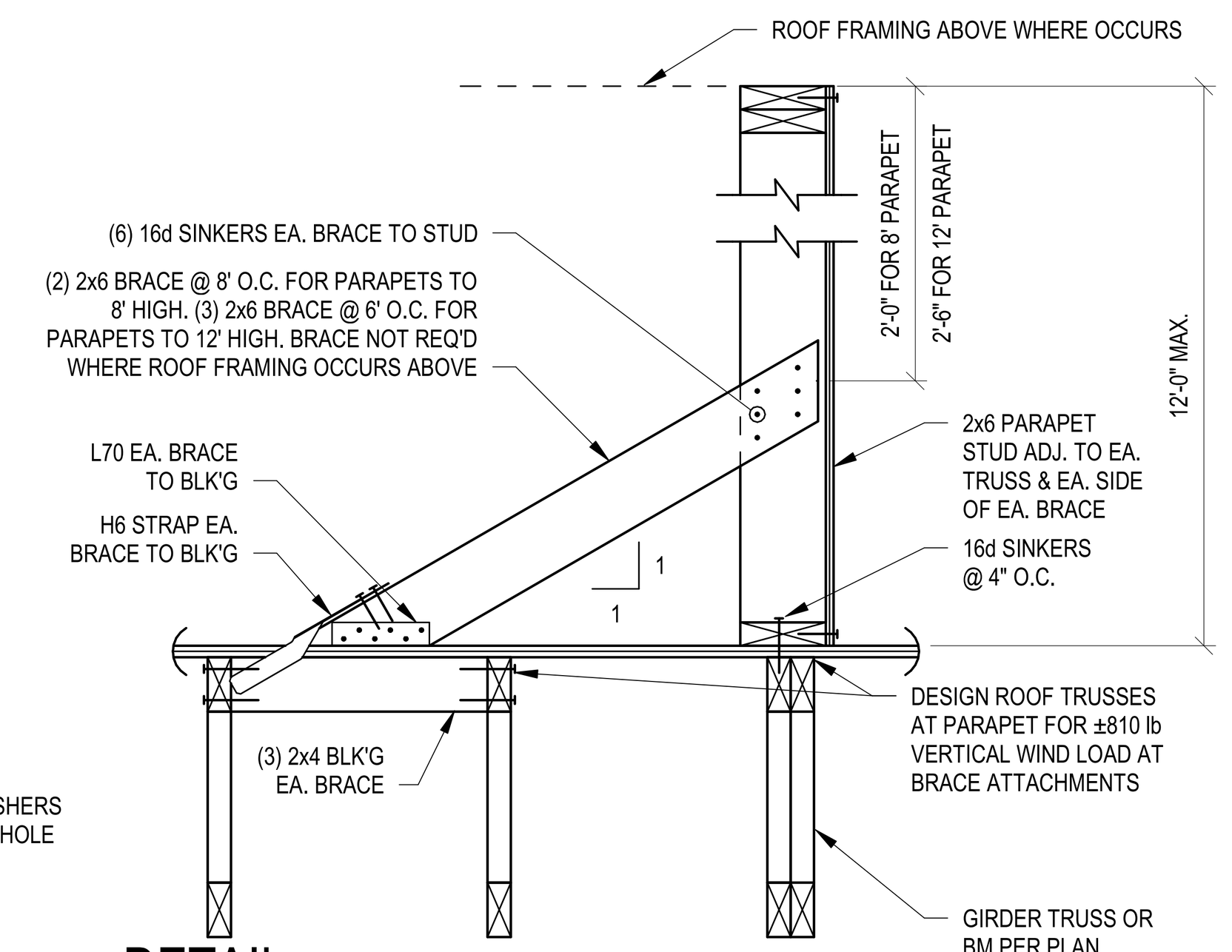

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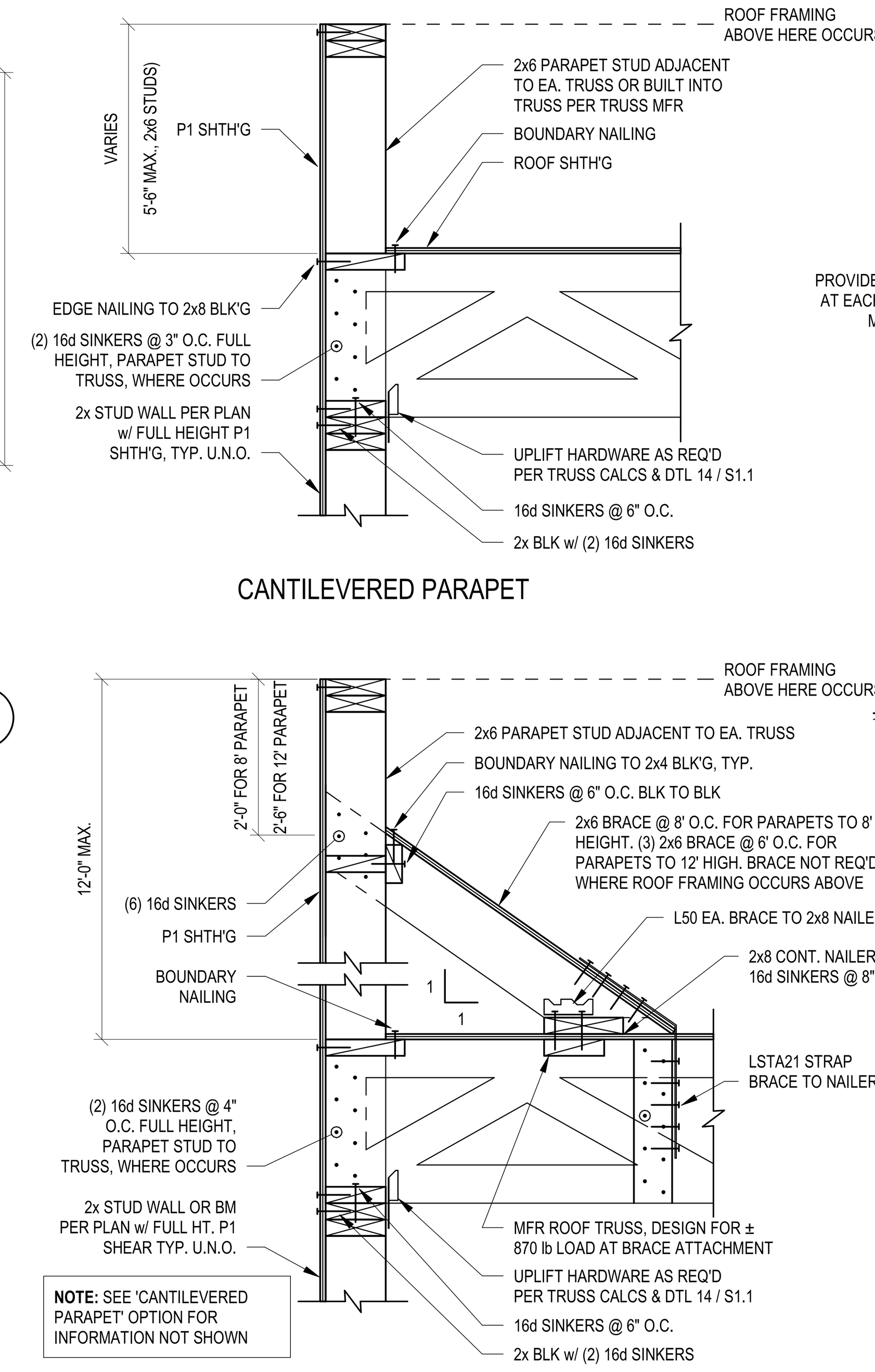

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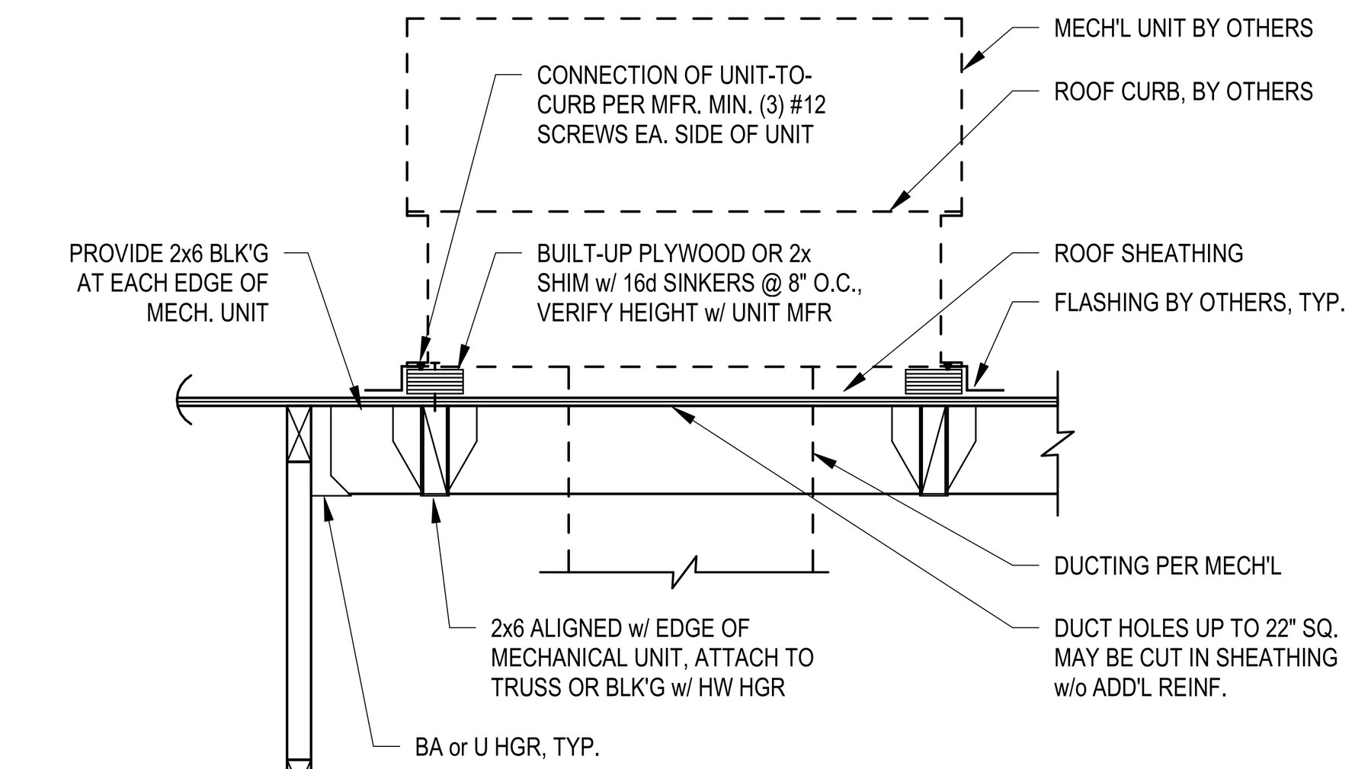

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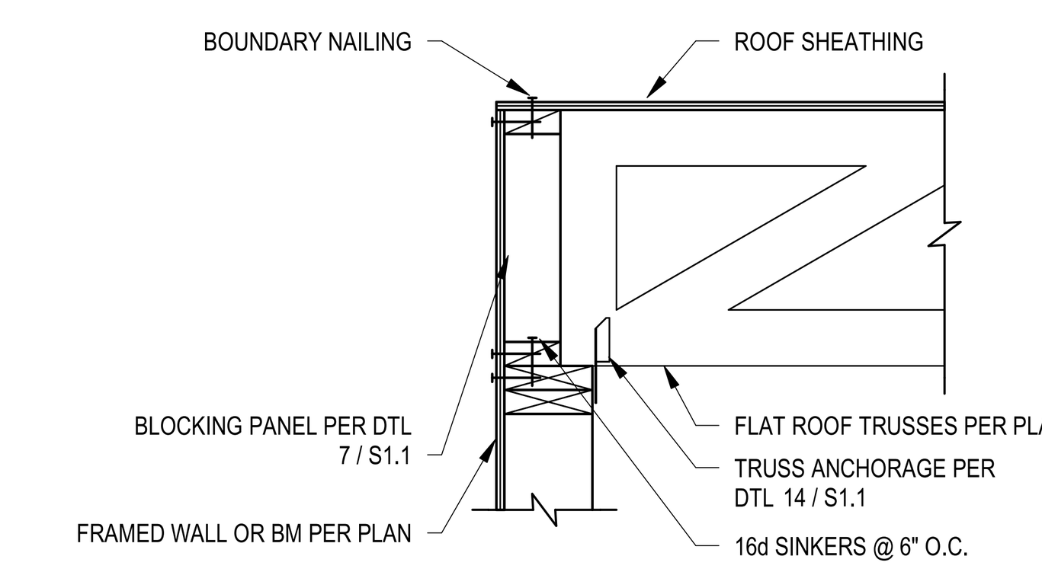

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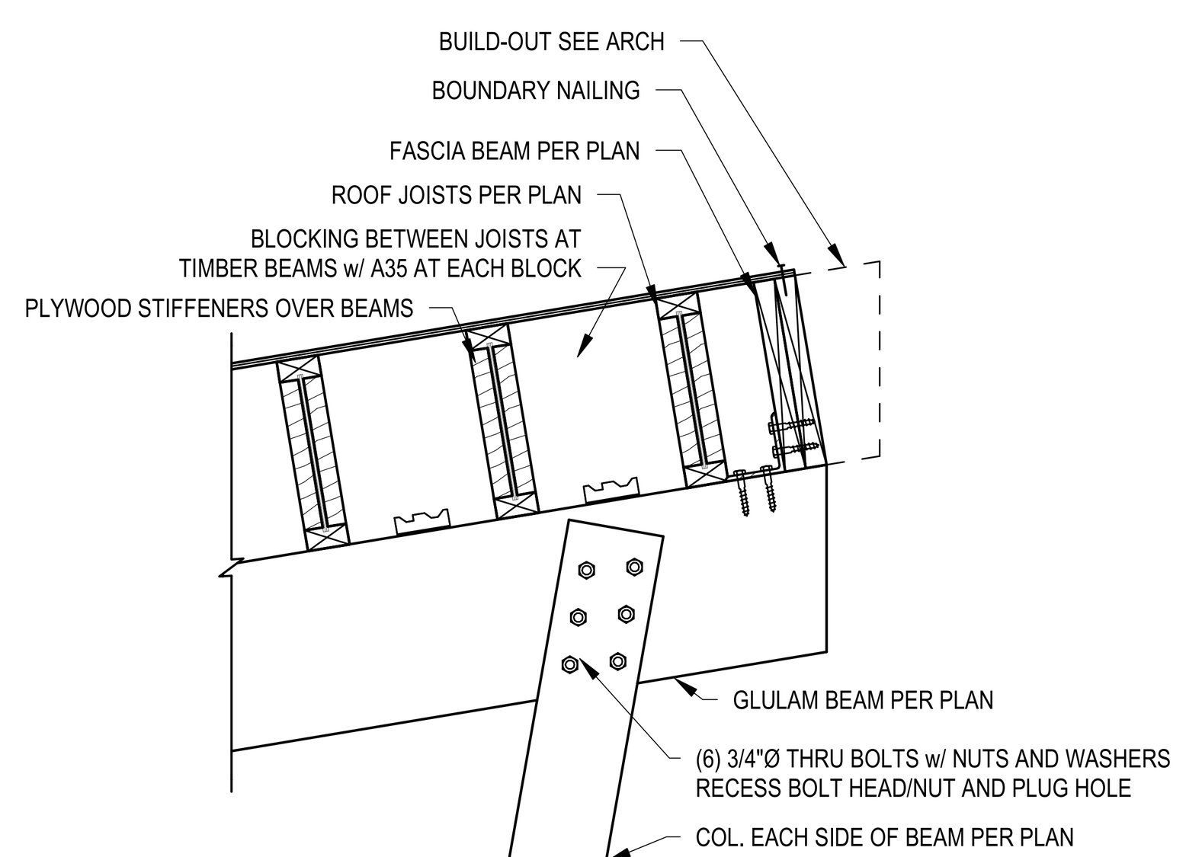

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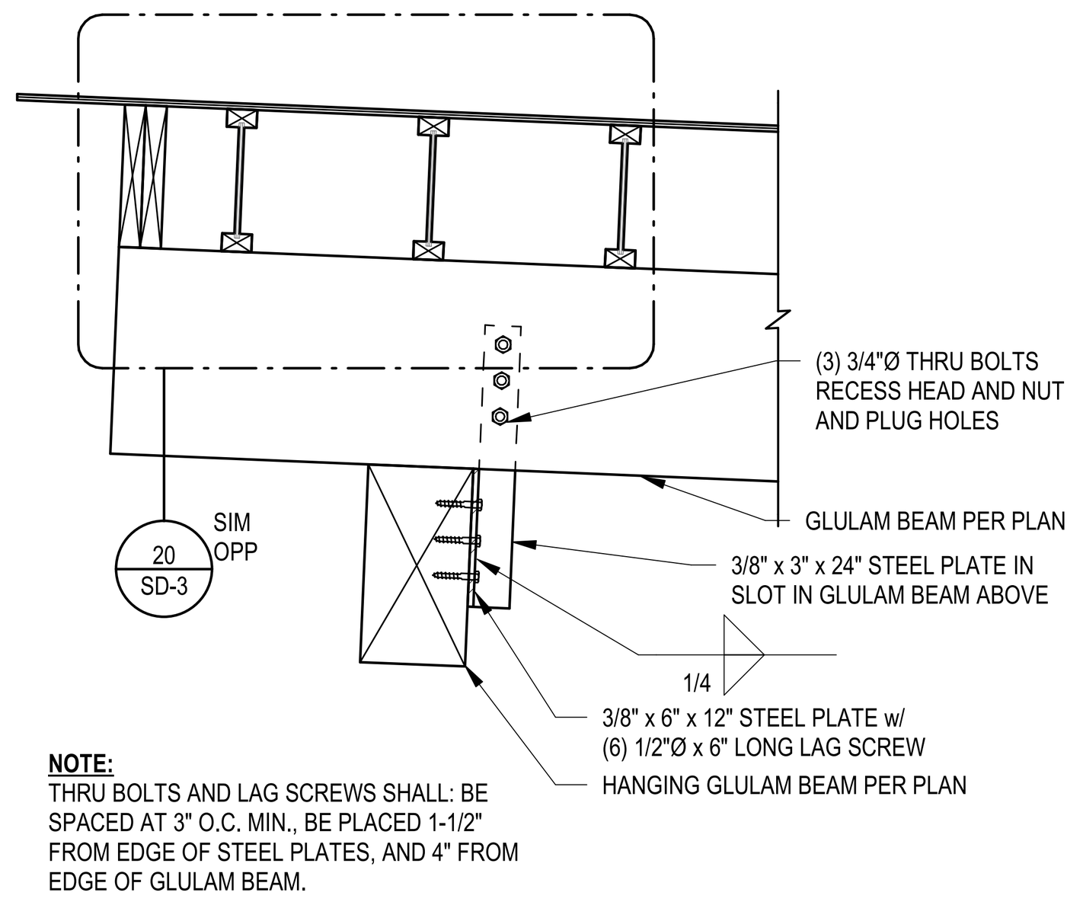

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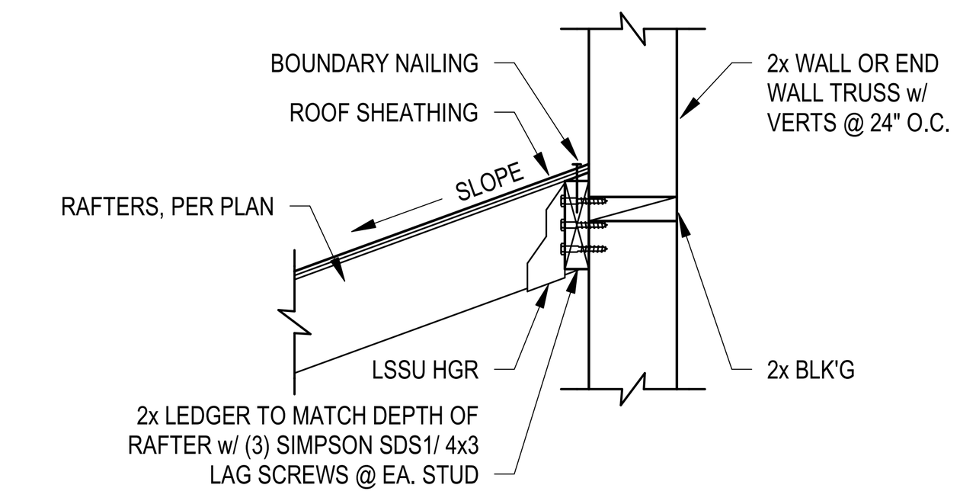

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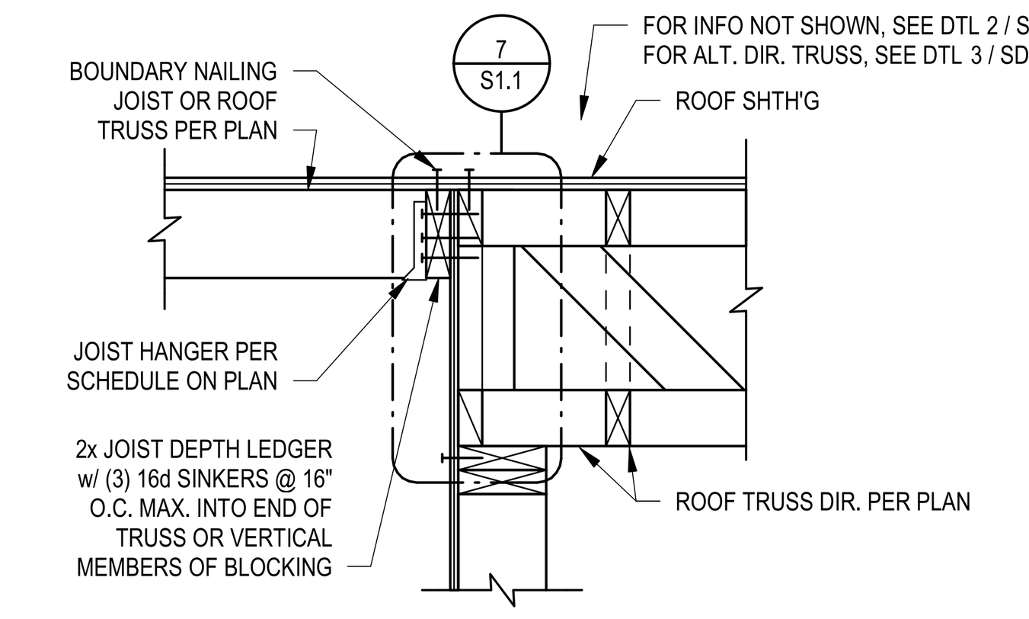

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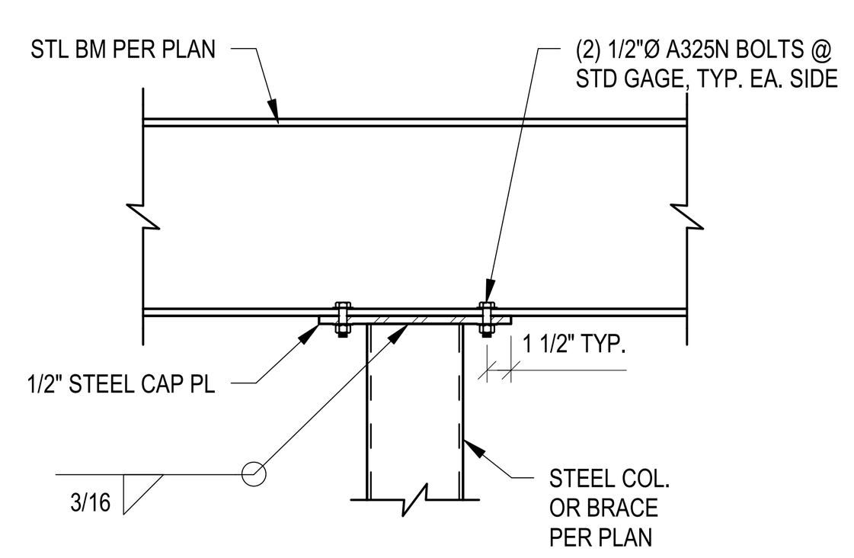

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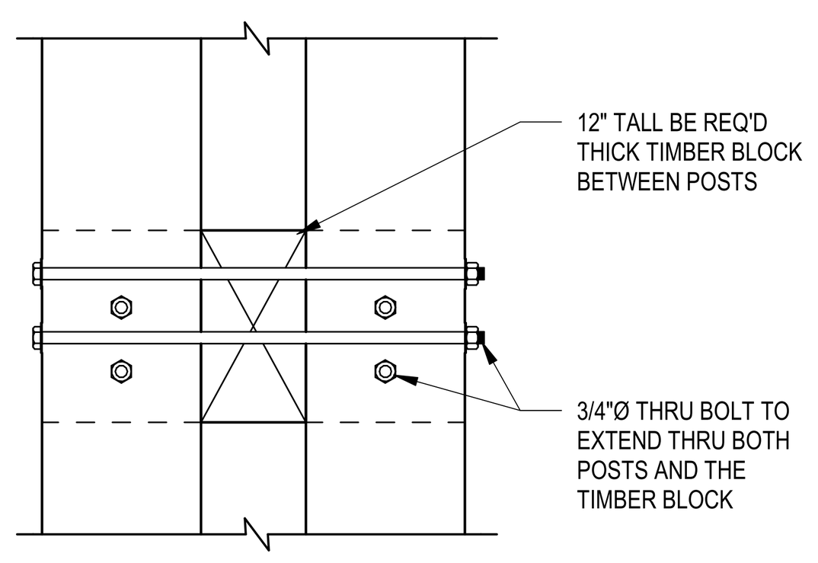

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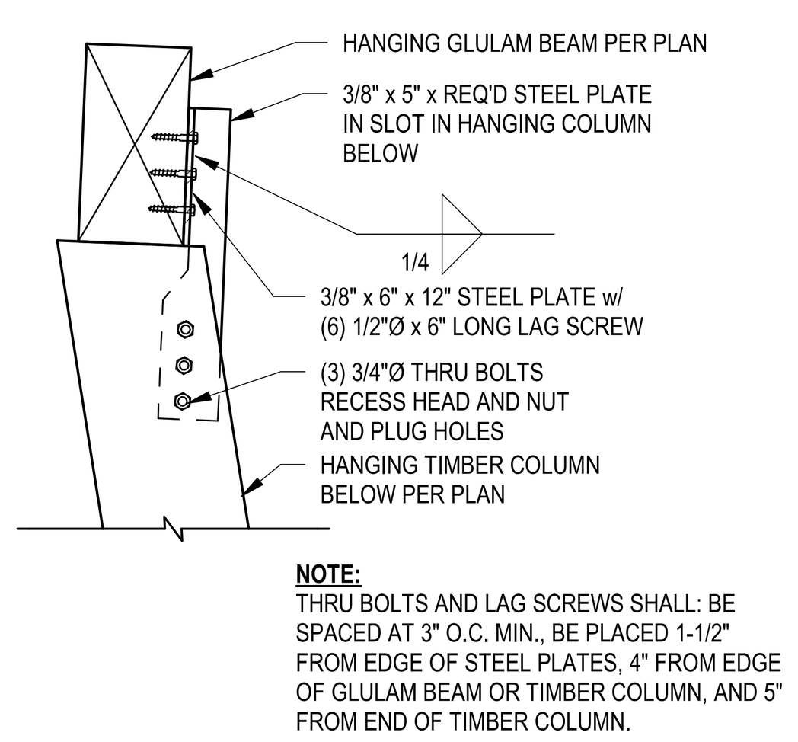

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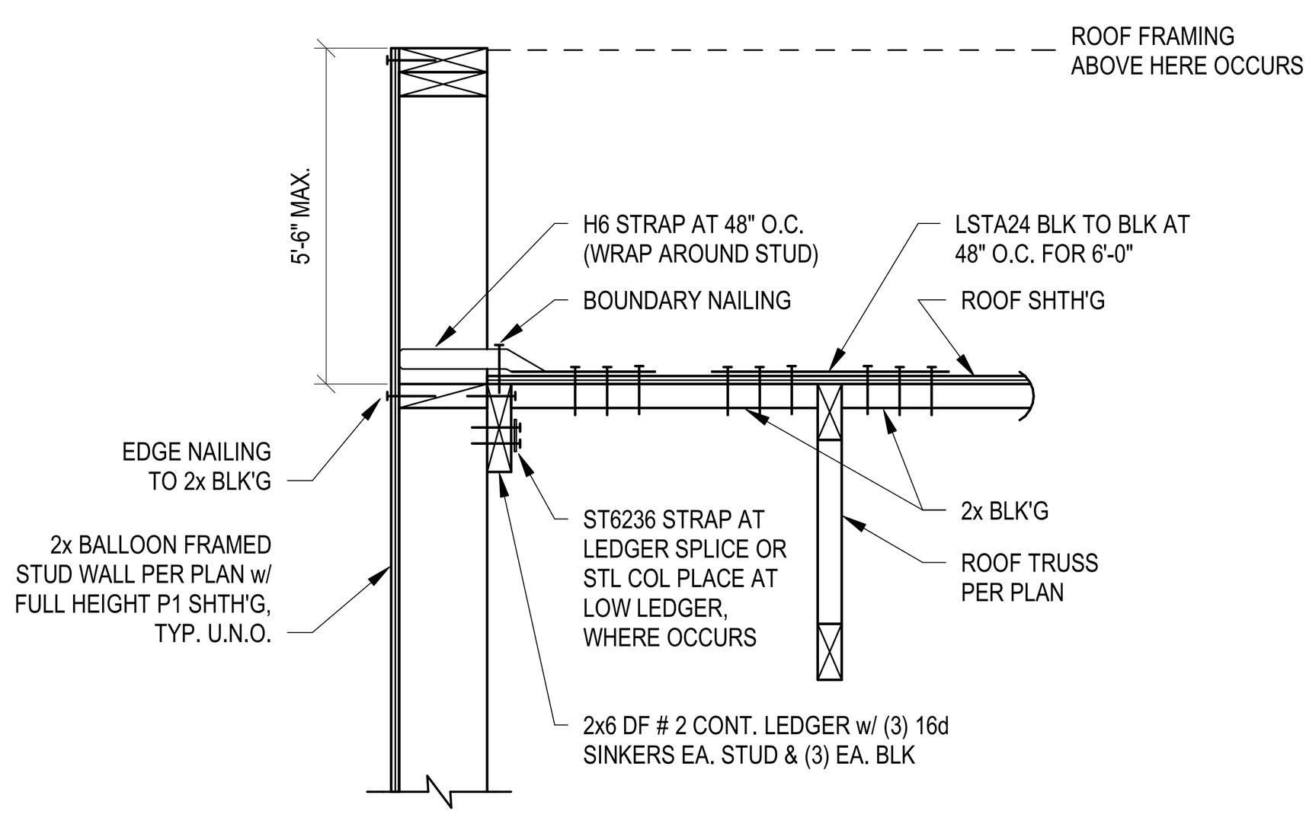

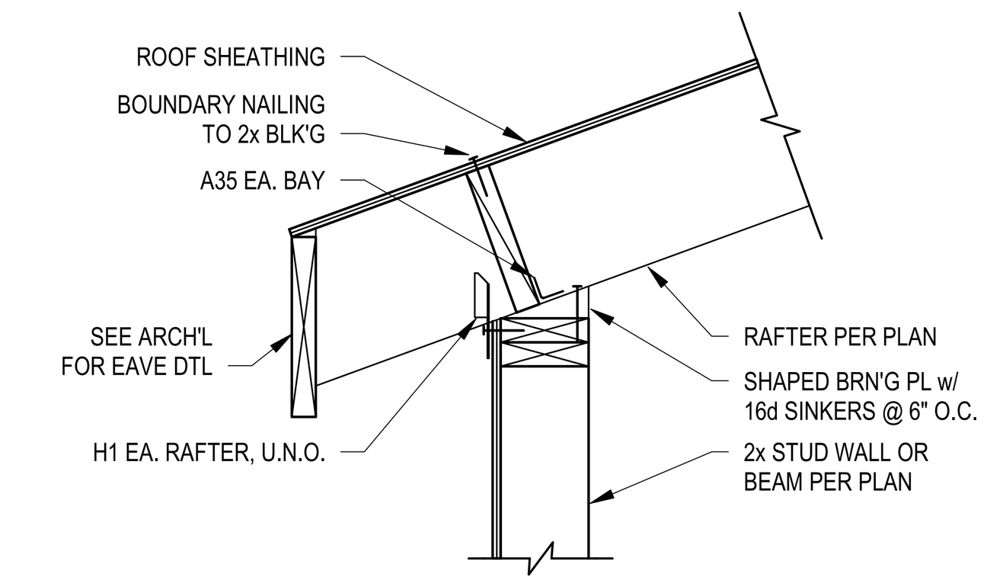

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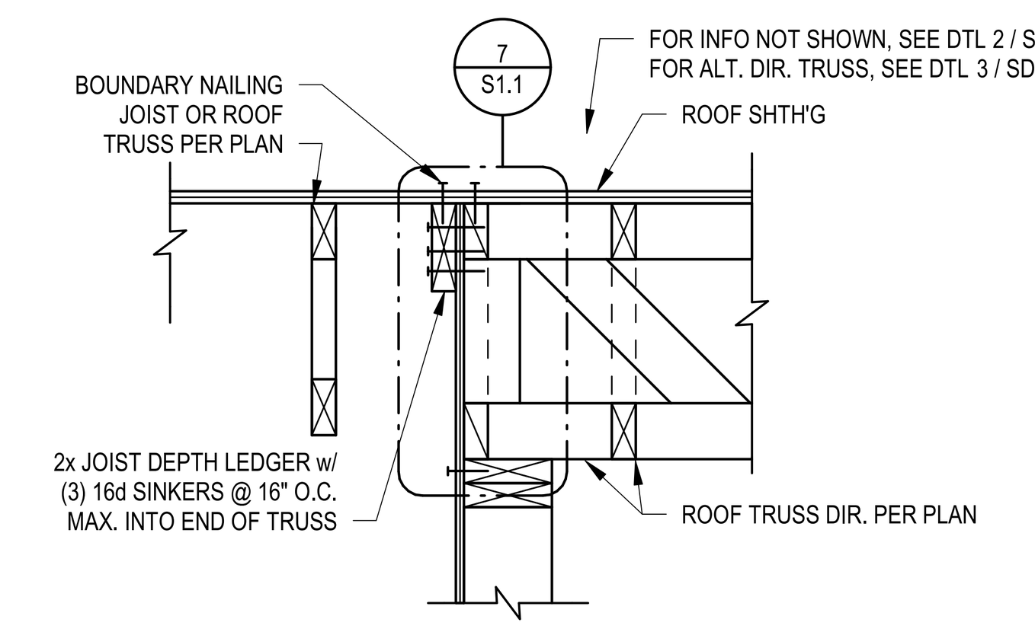

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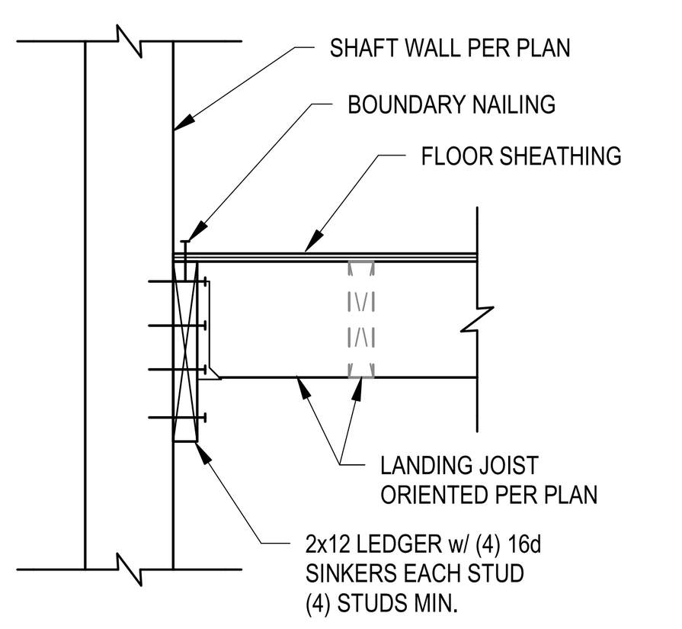

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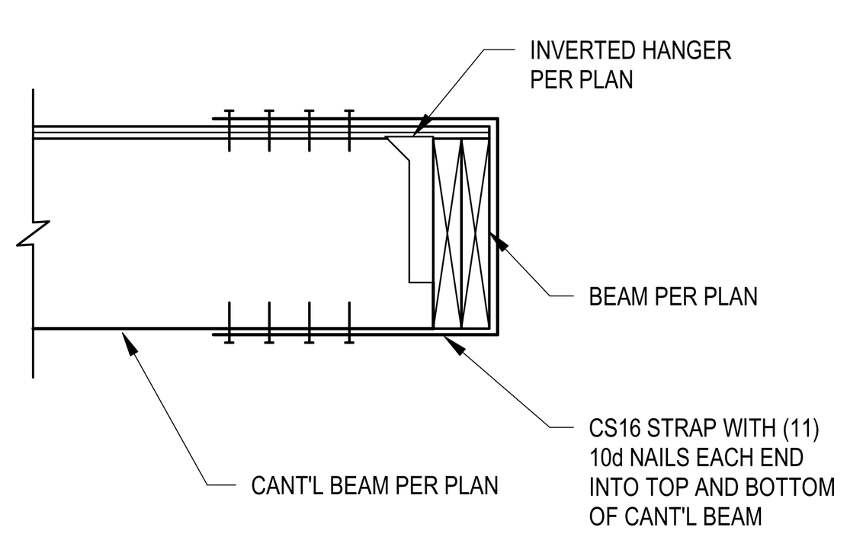

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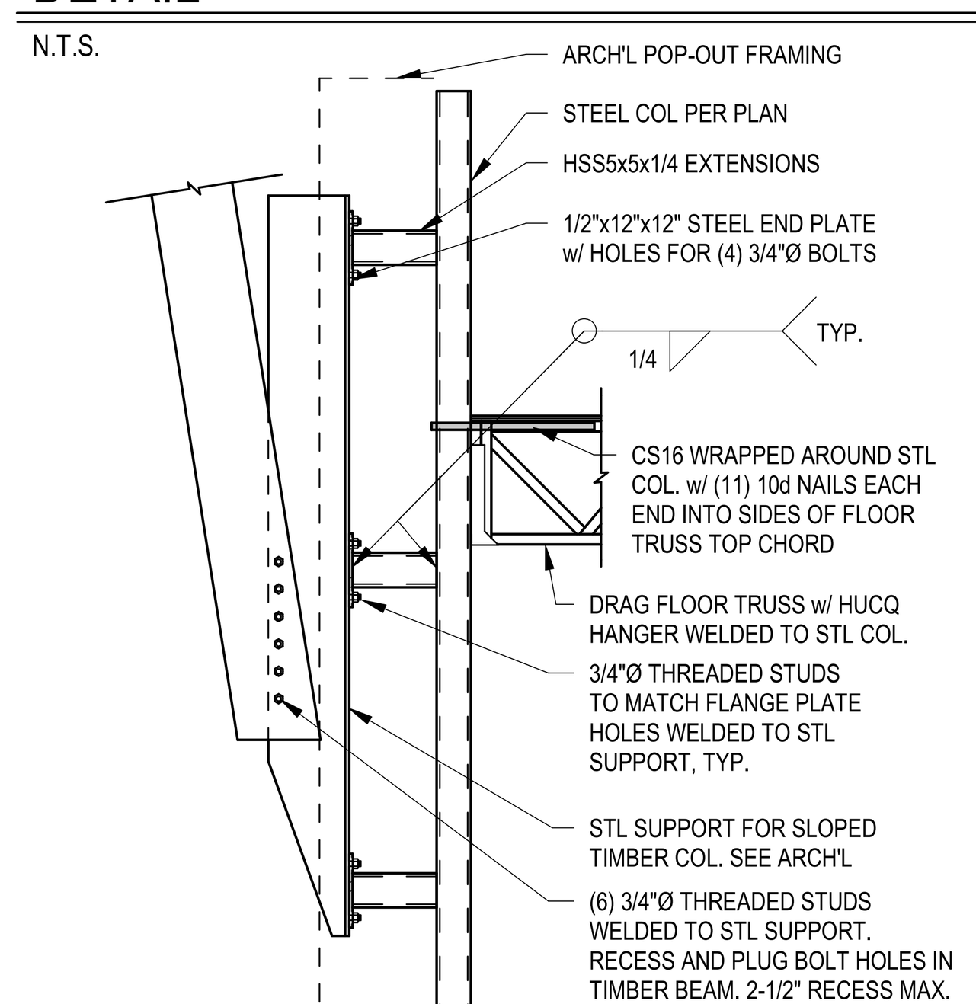

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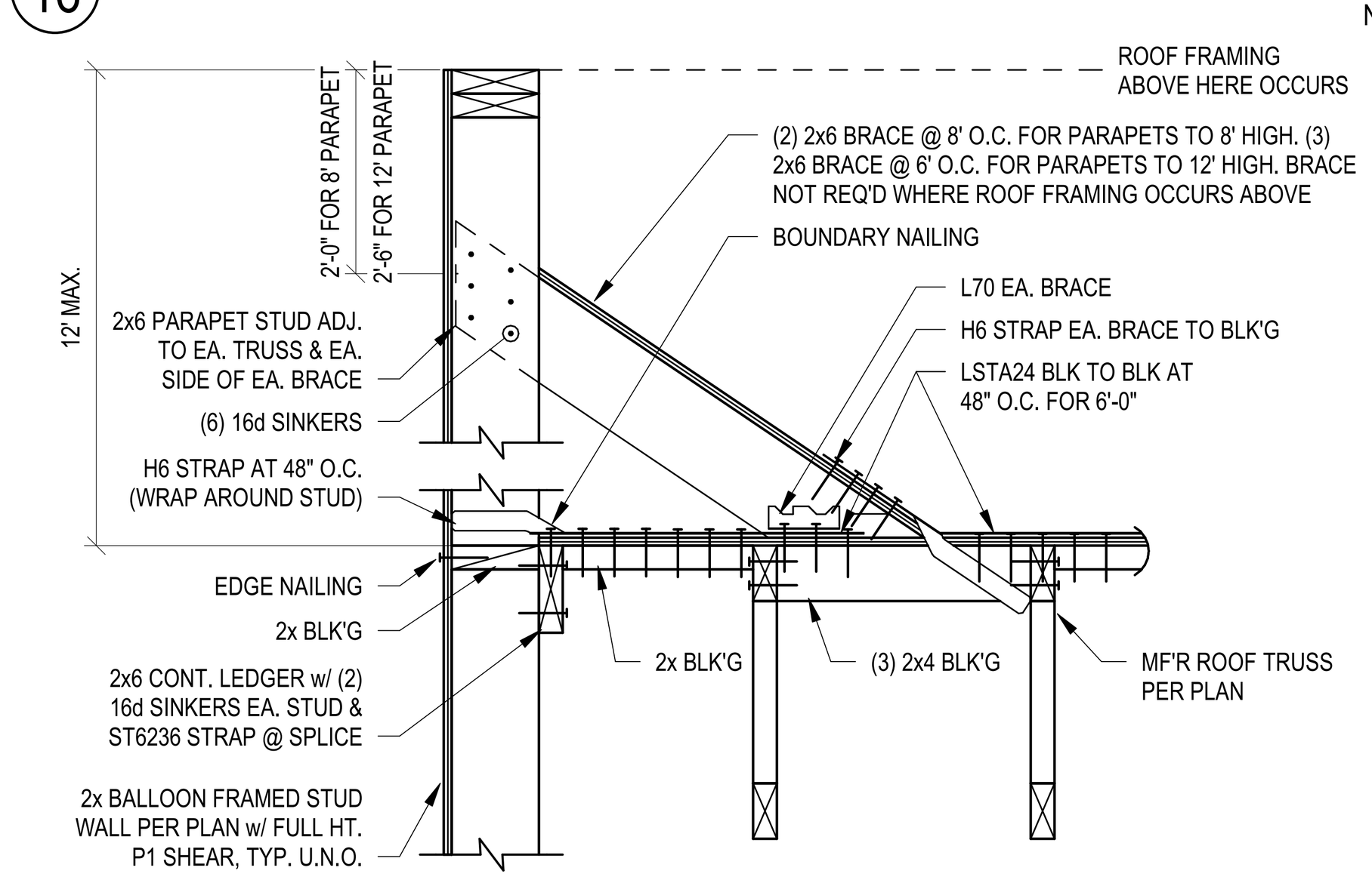

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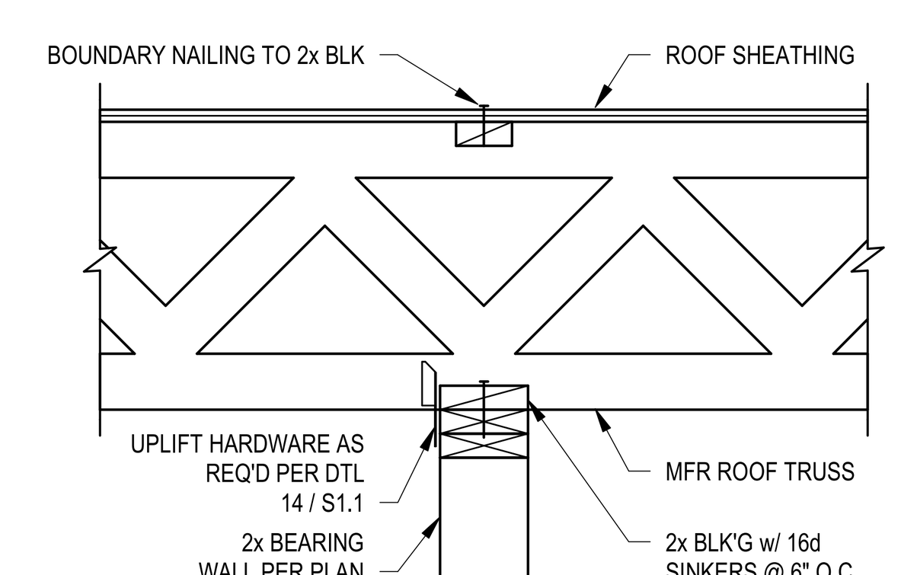

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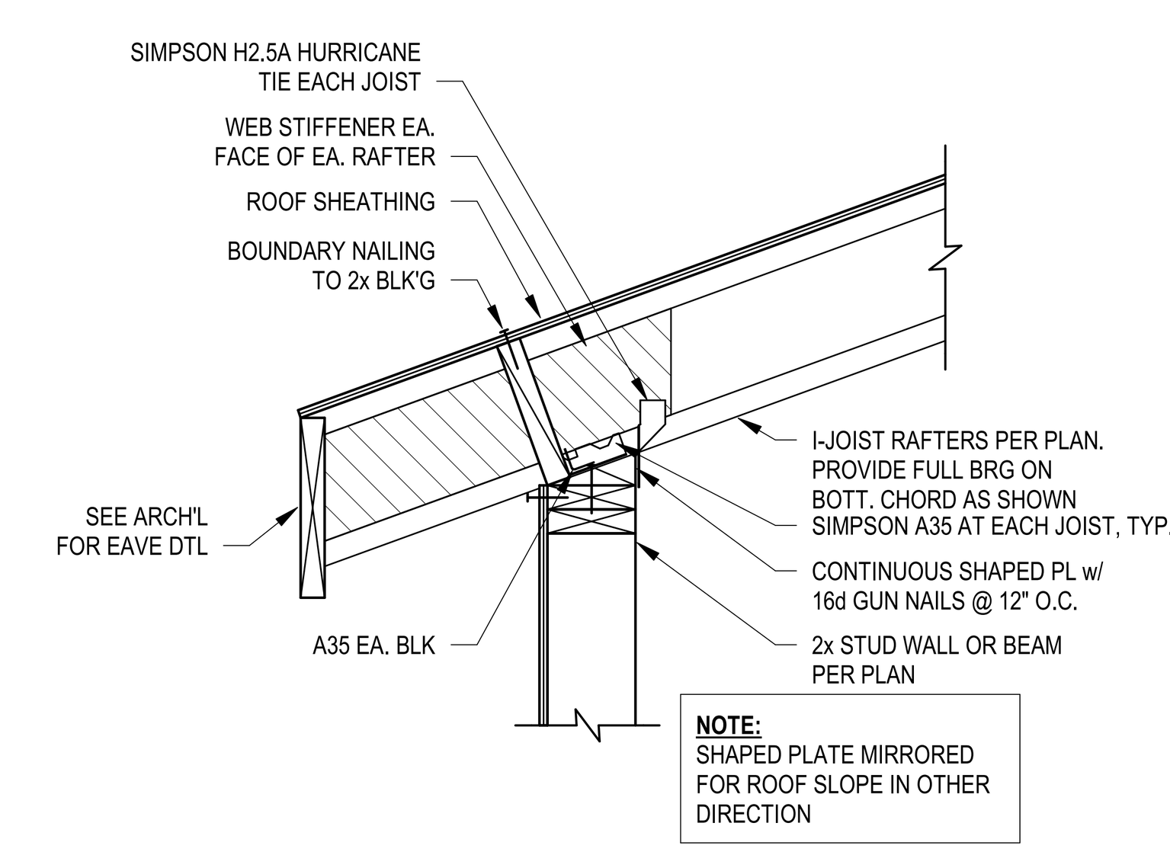

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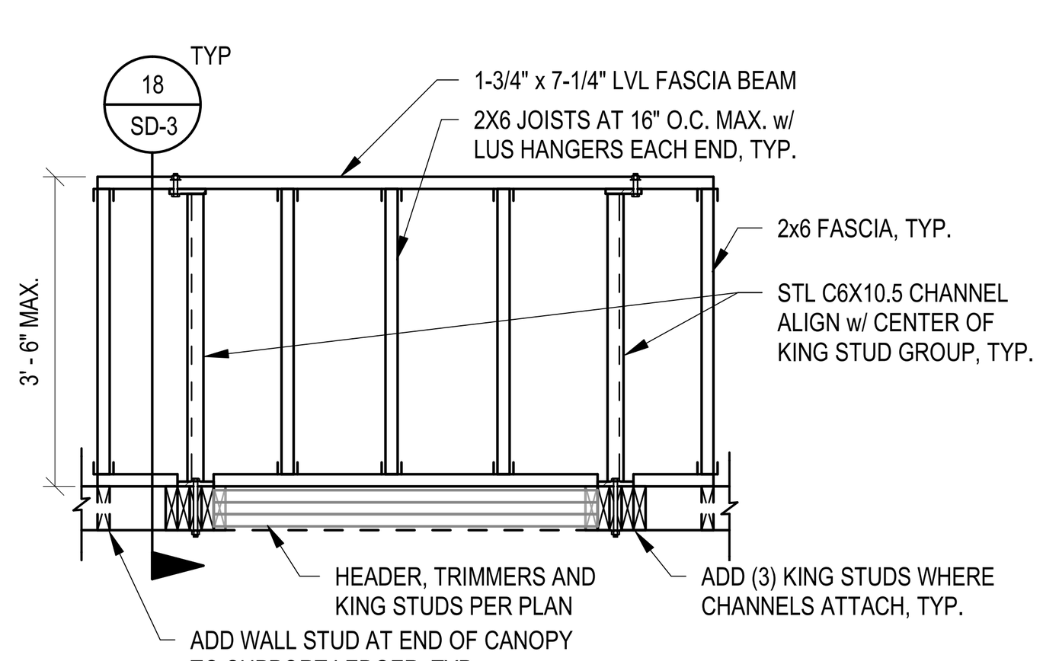

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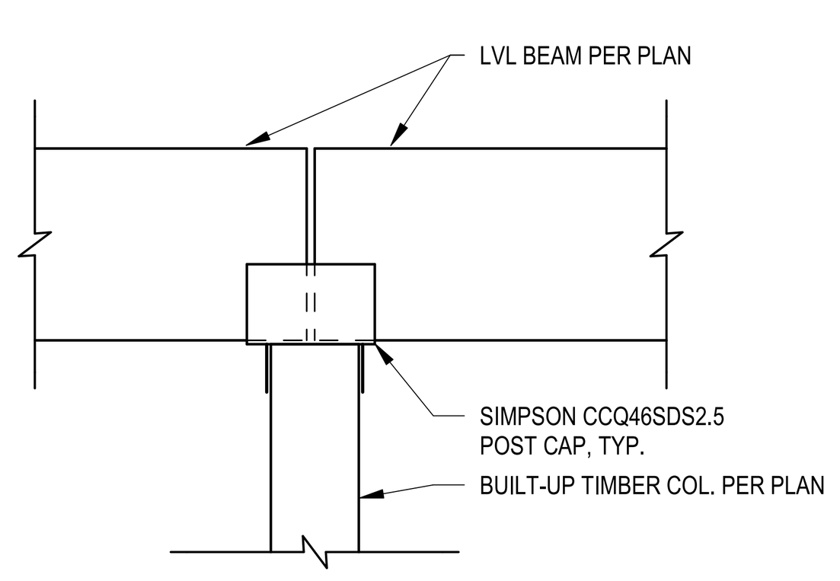

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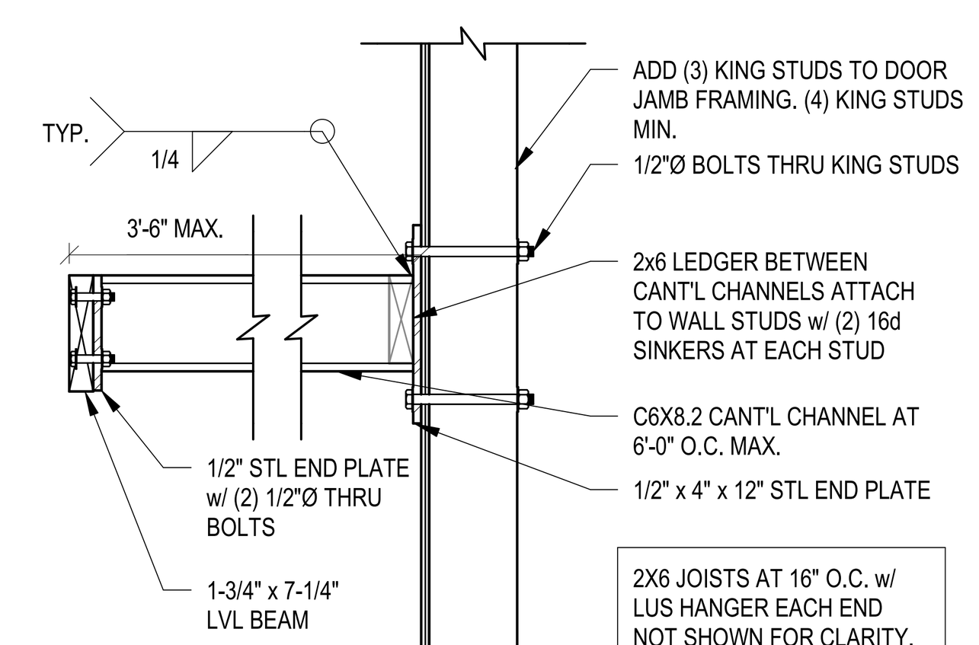

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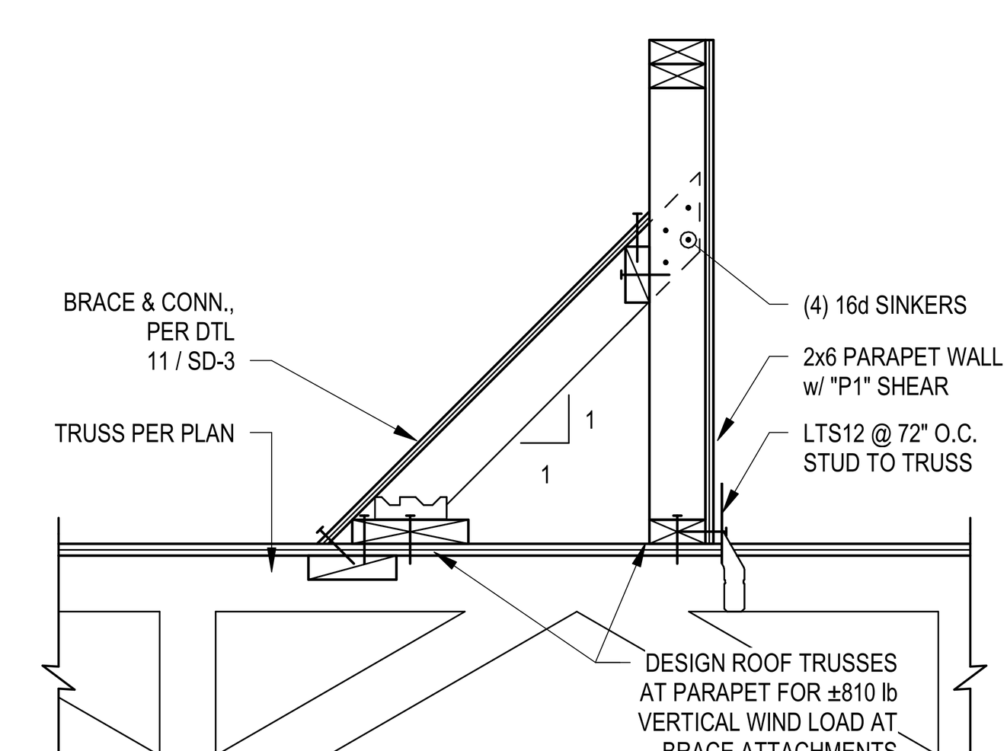

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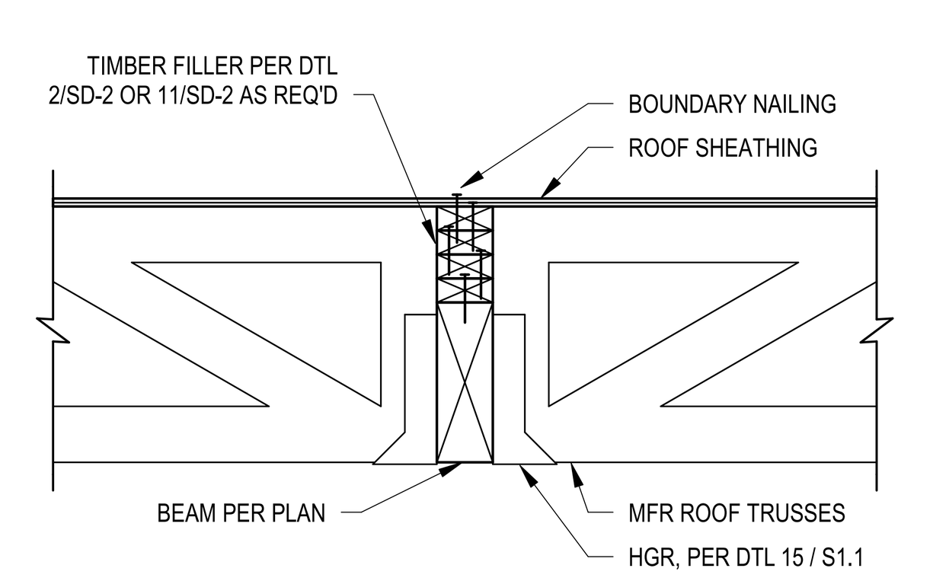

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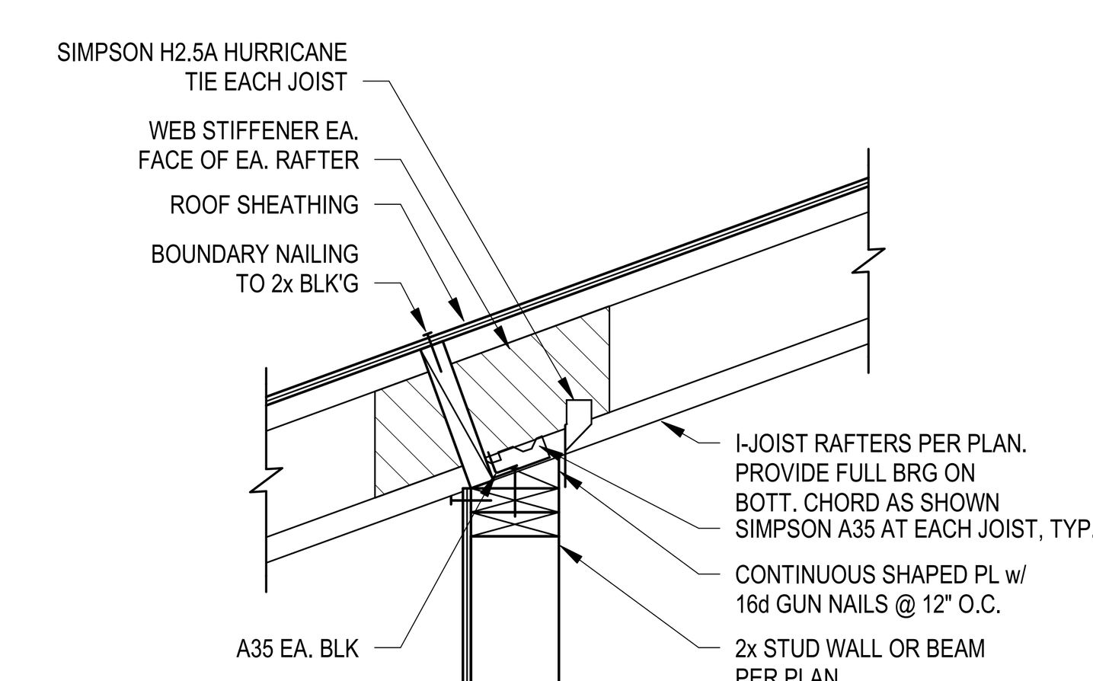

DETAIL 5
N.T.S.

VECTOR ENGINEERS
Boise, Idaho
(208) 996-0303
WWW.VECTORSE.COM
ID FIRM LICENSE NUMBER 10449-0

CONSULTANT:

OWNER:

ELEMENT HOTEL

PROJECT:

ISSUE DATE	DESCRIPTION

#	REV. DATE	DESCRIPTION

For Review
02/24/2025 10:13:22 AM

100% CD

STRUCTURAL DETAILS

PROJECT #: U1477.005.221
DRAWN BY: SDC
CHECKED BY: BSE

SHEET #
SD-3